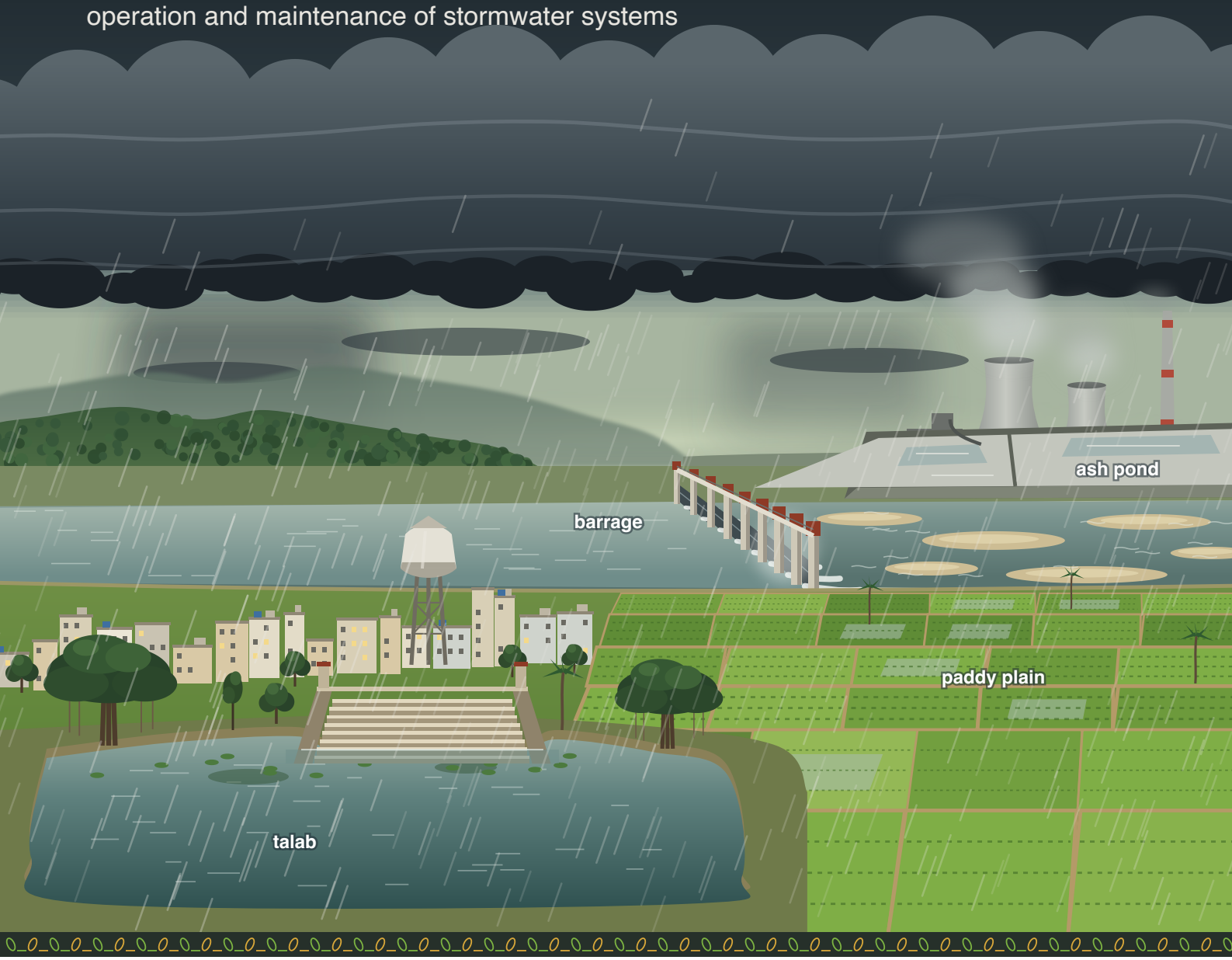
CHHATTISGARH

Stormwater Design Manual

Standards for the planning, design, construction, operation and maintenance of stormwater systems



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Prepared by

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About this draft

Status. This is Draft v1.0 of the *Chhattisgarh Stormwater Design Manual*. It was prepared by Neer as a proposed stormwater design standard for Chhattisgarh.

Not adopted. This Manual has not been adopted, notified or endorsed by the Government of Chhattisgarh or by any of its departments, local bodies or agencies. In particular, it has not been adopted, notified or endorsed by the Urban Administration and Development Department (UADD), the Housing and Environment Department or its Directorate of Town and Country Planning (T&CP), the Water Resources Department (WRD), the Nava Raipur Atal Nagar Vikas Pradhikaran (NRDA), the Raipur Development Authority (RDA), any Municipal Corporation, Nagar Palika Parishad, Nagar Panchayat or other local body, or any other agency. Where it names a body, such as the Revenue and Disaster Management Department, the General Administration Department, the Public Health Engineering Department (PHED), the Forest and Climate Change Department, the Chhattisgarh State Wetland Authority, the Chhattisgarh Environment Conservation Board (CECB), the Chhattisgarh State Disaster Management Authority (CGSDMA), the State Ground Water Management and Regulatory Authority, the Arpa Special Area Development Authority, the Chhattisgarh Housing Board, the Raipur Municipal Corporation (RMC), the Town Services of the Bhilai Steel Plant (SAIL), a coal or power company such as South Eastern Coalfields Limited (SECL), NTPC, the Chhattisgarh State Power Generation Company (CSPGCL) or Bharat Aluminium Company (BALCO), a district administration, or a body of the Union, such as the Central Water Commission (CWC), the Central Ground Water Board (CGWB) or the Directorate General of Mines Safety (DGMS), it describes the role that body would have if the Manual were adopted. It does not say the body has agreed to that role.

No legal force. Until a competent authority adopts it, with or without changes, this Manual has no legal force. The Chhattisgarh Bhumi Vikas Niyam, 1984 as amended, the development plans, including the Naya Raipur Development Plan-2031, and their approval conditions, drainage schemes, the conditions of environmental clearances and mining plans, the Coal Mines Regulations, 2017, orders of courts and tribunals and approval conditions in force continue to apply.

Names. Some places, rivers and gauges have more than one name or spelling in statutes, sources and on IMD and CWC stations:

- Chhattisgarh was part of Madhya Pradesh until November 2000, and older sources, including the CWC flood estimation reports and IMD's older tables, name its districts as districts of "Madhya Pradesh" (one annex of a CWC report spells it "Madhaya Pradesh"). The district map has also changed. Before 2012 Raipur district included today's Baloda Bazar and Gariaband, and before 1998 Mahasamund and Dhamtari; Durg included Balod and Bemetara; before 1998 Bilaspur included Korba and Janjgir-Champa, and Pendra Road has been in Gaurela-Pendra-Marwahi district since 2020; before 2022 Rajnandgaon included Khairagarh-Chhuikhadan-Gandai and Mohla-Manpur-Ambagarh Chowki, and Raigarh included Sarangarh-Bilaigarh; the old Bastar district included today's Kanker, Kondagaon, Narayanpur, Dantewada, Bijapur and Sukma; and Surguja (also Sarguja or Sirguja) included Balrampur and Surajpur, and before 1998 Koriya. Kabirdham district is also called Kawardha. IMD and CWC sources, and the rainfall data in this Manual, use the districts of their time, and a clause that names a district must be checked against the current list;
- the **Seonath** is also written **Sheonath** and **Shivnath**, the form of many State and court documents; **Katghora** is written Kathghora or Kathgora, **Seorinarayan** Sheorinarayan, **Wadrafnagar** Wardafnagar, **Jashpur Nagar** Jashpurnagar, **Manendragarh** Mahendragarh in IMD's tables, and the **Maniari** Maniyari. The Gangrel dam on the Mahanadi forms the **Ravishankar Sagar** (R.S. Sagar), the dam on the Hasdeo is the **Minimata Bango** (Hasdeo Bango) dam and the barrage below it at Korba the **Darri** (Hasdeo) barrage, and the Seonath barrage at **Mongra** is written Mogra in WRD's gauge report;
- **Nava Raipur Atal Nagar** is the new capital, called **Naya Raipur** in its development plan of 2008 and in most sources, Nava Raipur in others and Atal Nagar since 2018. Its authority, called **NRDA** in this Manual, now names itself the Nava Raipur Atal Nagar Vikas Pradhikaran (NRANVP). It is a special area development

authority under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973: its notification No. 4066 of 23 May 2022 delegates to its Chief Executive Officer the powers of a special area development authority under ss.69 to 69-ग of that Act, with some exceptions. No separate Act for NRDA was found;

- where a shared clause names the State's main city or its municipal corporation, this Manual names **Raipur**, the capital, and the **Raipur Municipal Corporation (RMC)**. **RDA** is the Raipur Development Authority, which carries out development schemes in and around the city; the law under which it was constituted was not verified for this Manual;
- **Raipur** has two IMD observatories about 15 km apart: **Raipur (Obsy)** (42875) in the city, with records from 1901, and **Mana (A)** (11203) at the airport, also called Raipur (Mana), from 1979. The 370.3 mm of 4 August 1910 is a Raipur (Obsy) record; on 30 June 2007 Raipur (Obsy) recorded 297.0 mm and Mana (A) 273.0 mm. Durg also has two gauges, Durg and Durg (Obsy), whose records differ (337.0 mm on 15 June 2004 and 260.4 mm on 13 August 2006). This Manual names the station each figure comes from;
- **Jagdalpur** has two systems of river marks, which must not be mixed. CWC's flood forecasting site on the Indravati at Jagdalpur has a warning level of 539.49 m (539.50 m in one advisory) and a danger level of 540.80 m, and its highest flood level, 544.68 m of 9 July 1973, was exceeded on 26 September 2026, when the river was reported at 544.891 m, as transcribed from CWC's data. The press reports the river against a local gauge (12.56 m, "4.26 metres above the danger mark", on the same day). This Manual uses CWC's levels for design and treats the local gauge as an alert level only. No source read states the datum of either set, and levels are quoted as reported [TBC];
- dam releases, like most river flows in Chhattisgarh's sources, are given in **cusecs** (cubic feet a second; 1 cusec = 0.0283 m³/s). This Manual quotes them as given and adds the metric value where it matters for design;
- the **Chhattisgarh plain** is the basin of the upper Mahanadi and the Seonath around Raipur, Durg–Bhilai, Bilaspur and Rajnandgaon; the **Bastar plateau** is the south, the Bastar division on the Indravati and the Sabari; and the **northern hills** are Surguja, Koriya and Jashpur, on the Rihand, the Kanhar and the headwaters of the Hasdeo and the Brahmani. The **Maikal** range bounds the plain on the west. The names describe regions, not administrative units, except where a district bears the same name;
- the plain's farmers name four classes of soil, from the ridges to the valley floors: **bhata** (भाठा), thin gravelly soil, often less than 30 cm deep over murram or laterite; **matasi** (मटासी), a sandy or silty loam of the upper slopes; **dorsa** (डोरसा), a clay loam of the lower slopes; and **kanhar** (कन्हार), a deep black clay of the lowest land that cracks when dry. The names come from agronomy papers; no dictionary source was found for *matasi*, *dorsa* or *kanhar* in this sense, and *bhata* is given only as stony ground. Local names do not map one-to-one onto soil taxonomy; and
- in the coalfields of Korba, Raigarh and Surguja a **mine void** is the pit left by opencast mining, a **pit lake** a void that has filled with water, and an **overburden dump** (OB dump) the heap of rock and soil removed to reach the coal. An **ash pond** is the pond in which a power station stores its fly ash as slurry, held by an **ash dyke**. **SECL** is the subsidiary of Coal India that holds most of the State's coal mines; **DGMS** regulates mine safety, including the danger of inundation, under the Coal Mines Regulations, 2017.

This Manual uses the first form and keeps the others where a title, a source or a station uses them. Some names of bodies and laws need a word of explanation:

- the **Chhattisgarh Bhumi Vikas Niyam, 1984** (CGBVN 1984) are the land development rules of 1984 that Chhattisgarh inherited from Madhya Pradesh in 2000 and has amended many times since, by notifications of the Housing and Environment Department under s.85(1) read with s.24(3) of the **Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973**. Madhya Pradesh replaced its own 1984 rules with the Madhya Pradesh Bhumi Vikas Rules, 2012, which do not apply in Chhattisgarh. No consolidated text of the Chhattisgarh rules was found; where this Manual relies on the wording of the 1984 rules as Madhya Pradesh published them, it says so;
- a **Municipal Corporation** (Nagar Palik Nigam) is constituted under the Chhattisgarh Municipal Corporation Act, 1956, and a **Nagar Palika Parishad** (Municipal Council) or **Nagar Panchayat** under the Chhattisgarh Municipalities Act, 1961; both are Madhya Pradesh Acts as adapted by Chhattisgarh, and their Chhattisgarh texts were not read for this Manual. All three are urban local bodies (ULBs) under **UADD**. The

State has 14 Municipal Corporations, nine of which, Raipur, Bhilai, Durg, Bilaspur, Korba, Rajnandgaon, Raigarh, Jagdalpur and Ambikapur, were chosen for AMRUT; the other five are taken to be Bhilai-Charoda, Risali, Birgaon, Dhamtari and Chirmiri [TBC];

- **Bhilai** has two drainage regimes: the township of the Bhilai Steel Plant, whose drains are kept by the plant's Town Services, and the areas of the Bhilai and Risali Municipal Corporations. In July 2025 the Commissioner of the Bhilai Municipal Corporation was reported to have warned the plant over uncleaned township drains, whose water hyacinth had blocked culverts downstream;
- the **Housing and Environment Department** issues the Bhumi Vikas Niyam, approves development plans under s.19 of the 1973 Adhiniyam and oversees the special area development authorities; **T&CP**, its Directorate, prepares development plans and grants development permission, and the ULB grants building permission. Works of Union and State departments, and of local authorities, need the concurrence of T&CP under ss.27–28 of the Adhiniyam. Bilaspur's **Arpa Special Area Development Authority** had a development plan for 2033, approved on 10 February 2014, but after orders of the High Court of 22 January 2019 the State decided that its permission was no longer needed to sell, build on or develop land in its area;
- **WRD** (Jal Sansadhan Vibhag) holds the State's rivers, dams, barrages and reservoirs, including Gangrel, Bango and Kopra Jalashay, and is named in this Manual as the State groundwater agency. The Chhattisgarh Ground Water (Management and Regulation) Act, 2022 provides for a State Ground Water Management and Regulatory Authority, but no notification bringing the Act into force was found;
- **CECB**, under the Housing and Environment Department, reports to the National Green Tribunal (NGT) on sewage and treatment plants. The **Chhattisgarh State Wetland Authority**, whose nodal department is Forest and Climate Change, was reported to have been reconstituted in December 2021 under the Wetlands (Conservation and Management) Rules, 2017; no wetland notified under those Rules was found. **CGSDMA** was reported to have been constituted in 2007; no State disaster management plan or urban flood plan of it was found;
- **nistar** (निस्तार) is the community's right of use over village land and tanks. Tanks over which a village exercised rights of irrigation or nistar vested in the State on 6 April 1959 (Chhattisgarh Land Revenue Code, 1959, s.251), and unauthorised occupation of them can be proceeded against under s.248; and
- a **tehsil** is the revenue subdivision of a district, under a Tahsildar, who hears cases of encroachment on public land and tanks.

Some abbreviations have more than one meaning:

- AMC is the antecedent moisture condition in Chapters 2 and 3; this Manual does not use it for any municipal body;
- RMC is the Raipur Municipal Corporation, not ready-mixed concrete; RDA is the Raipur Development Authority;
- NRDA is used for the authority of Nava Raipur Atal Nagar under both its old and new names;
- HFL is the highest flood level recorded at a gauge, as CWC uses it. It is not a design level, and sources sometimes give different values for the same gauge. The local gauge at Jagdalpur is not a CWC level. WL and DL are CWC's warning and danger levels;
- FRL (full reservoir level) is the term of WRD's gauge reports for its dams and barrages; FTL (full tank level) is used for talabs and small tanks; MWL is the maximum water level of a reservoir;
- NGT CZ is the Central Zone bench of the National Green Tribunal, at Bhopal, which hears cases from Chhattisgarh;
- OB is overburden in the coalfields; and
- SWD is a storm water drain.

The sense is clear from the context. Hindi and Chhattisgarhi terms used in the text, such as *talab* (तालाब, a pond or tank that holds rainwater, as in Raipur's Budha Talab), *dabri* (डबरी, a small dug pond, of a farm or village), *kund* (कुंड, a small walled tank), *bandh* (बाँध, an embankment or dam, and the tank it forms, as in Telibandha), *nala* (नाला,

a natural drain or large storm drain; in Chhattisgarhi also *narwa*, नरवा), *nali* (नाली, a small street drain), *anicut* (a weir across a river), *nistar* and the soil names *bhata*, *matasi*, *dorsa* and *kanhar*, are explained where they first appear.

Use with care. Clauses and values may change after review. Designers remain responsible for their designs and for checking the data they use. Neer accepts no liability for decisions made on the basis of this draft.

Disclosure. Neer publishes the India Flood Atlas, a national map of modelled 1% AEP flood depth. Clause 6.11.3 names it as one example of a regional map that an Approving Authority may adopt for floor levels. The Manual does not require the Atlas; any map adopted under Clause 7.3.4 may be used.

Comments. Comments and corrections are welcome at info@neer.ai.

Preface

Chhattisgarh's cities flood for three reasons. Raipur showed the first on 4 August 1910, when its observatory recorded 370.3 mm in a day, more than the 100-year day on the national maps, and on a city whose drains have still no published design storm; it showed the second in early July 2026, when five ponds in one colony overflowed into homes and sewage entered houses, as reported; and Korba and Jagdalpur show the third, where rivers raised by dam releases, ash dykes and mine voids, and in Bastar an Indravati at a new record level, decide what happens at the outfalls.

1. **Bay depressions that bring days above the national maps, on a flat paddy plain whose clay seals once wet, with no design storm.** The State's heaviest rain comes from low-pressure systems and depressions that form over the Bay of Bengal and cross it westward; IMD counts 100 storms and depressions that affected the State from 1891 to 2014. Their days exceed the 100-year values that the CWC flood estimation reports map for the State: Raipur (Obsy) 370.3 mm on 4 August 1910, against about 320–360 mm on the map; a raingauge at Rajnandgaon 392.0 mm on 21 September 1965, against about 320 mm; Korba 358.0 mm on 12 August 2004 and Durg 337.0 mm on 15 June 2004, against about 280–320 mm. In September 2026 a statewide flood followed such a system: eight people were reported dead, about 16,000 rescued and more than 22,000 affected in 571 villages, all 14 gates of Gangrel were opened, and the Kharun overflowed at Mahadev Ghat in Raipur, as reported. In July 2026 Bilaspur was reported to have received 28 cm in a day; another report put it at 415.6 mm, and IMD has confirmed neither. The rain falls on a plain of gentle slopes (0–5% at Naya Raipur), laid out in bunded paddy fields and dotted with talabs, where the lowland *kanhar* clay, 40–50% clay by the CGWB's account, cracks when dry but has low permeability once wet. The drains built against such storms were sized for no stated storm. Raipur's plan for AMRUT in 2015 put the city's storm drain coverage at 49%, about 480 km of pucca drains, recorded 11 road stretches that flood two or three times a year and found that "Capacity of drain is not sufficient In rainy season most of the drains are overflowed", and proposed a drainage project of ₹1,191 crore; it states no return period, design rainfall or runoff coefficient. The only explicit design storm found in the State is in the Naya Raipur Development Plan-2031 of 2008: "5 years Maximum rainfall intensity should be taken for design of system", for the new capital only.
2. **Cities of talabs that have lost most of them, and nalas that carry sewage to the rivers.** Raipur is reported to have had about 130 ponds, of which 37 to 40 remain, about 69 ha in all; the counts conflict. Its nalas carry the city's sewage as well as its rain: Raipur's domestic sewage is linked to the Kharun "through eight big canals", as reported, and the city's new treatment plants at Chandanidih, Kara and Nimora draw it off through diversion weirs built on the nalas. In January 2021 CECB told the NGT that only Bilaspur had a sewerage system, and in January 2025 the High Court was told that about 40% of Bilaspur's sewage would still reach the Arpa untreated, as reported. In January 2026 the NGT ordered Durg's treatment plants on the Shankar and Pulgaon nalas, which carry most of the city's sewage to the Seonath, finished within six months, as reported. In Raipur the floods of July 2026 filled the Fafadih and Gudhiyari underpasses and pushed sewage into homes, as reported, and the residents blamed drains narrowed by encroachment and inadequate desilting.
3. **A north and east reshaped by coal and power and governed by dam releases, and a south where the Indravati set a new record.** The Hasdeo at Korba runs below the Minimata Bango dam and the Darri barrage, whose spillways were designed for 24,000 m³/s and 19,820 m³/s; in August 2024 Bango released about 30,000 cusecs (850 m³/s) through six gates and Darri about 20,000 cusecs, and houses in Korba were flooded, as reported, and in 2026 the Hasdeo set new highest levels at Katghora (313.58 m, 20 July) and Bangodam (303.5 m, 17 August), as transcribed from CWC's data. Around Korba and Raigarh the coal mines draw water from the ground, about 7,500–13,700 m³ a day across six of Korba's mines, with drawdown cones about 1.2 km wide in one study, and power stations store their ash behind dykes that have breached: NTPC's Dhanras dyke at Korba in July 2024, damaging about 16 ha of paddy, a dyke of CSPGCL's station at Korba West in April 2026, when one person died, and an ash dam of BALCO, whose slurry reached a nala and the Hasdeo, as reported. On the Mahanadi the outfalls of Rajim, Janjgir-Champa and Raigarh depend on releases from Gangrel, whose spillway was designed for 17,230 m³/s, and in the east on the backwater of the Hirakud reservoir in Odisha, which partly submerges 33 villages in Chhattisgarh, as reported; in August 2020 the Mahanadi rose above its danger level in Janjgir-Champa,

the Hirakud gates were opened at the State's request and the Kharun set its highest level at Pathardihi, 281.68 m on 29 August, as reported. In the south the Indravati at Jagdalpur rose on 26 September 2026 to 544.891 m, 4.1 m above CWC's danger level and above its record of 1973, as transcribed, after the Bastar floods of August 2025 had killed eight people, as reported.

Behind all three lies lost storage. Village tanks vest in the State under the Land Revenue Code (s.251), and in 2024, on the orders of the NGT, the Tahsildar at Baloda proceeded under s.248 against those who had encroached on its ponds. Around the talabs the only buffer enforced is the Wetlands Rules' bar on construction within 50 m of the highest flood level, which the State Wetland Authority has applied to wetlands of more than 2.25 ha as "deemed" wetlands, citing judgments of the Supreme Court: in 2025 it halted works at Karbala Talab in Raipur, a probe was ordered at Narhaiya Talab, and the Authority directed NRDA to withdraw tenders for plots and residences beside the Jhanjh wetland in Nava Raipur, as reported. A State directive of May 2023 to survey every water body in Raipur district was reported not to have been carried out, and no wetland in the State has been notified under the Rules. Nava Raipur was planned around a network of lakes and reservoirs, 55 of them as reported, with parks designed to carry storm water to them. Kopra Jalashay near Bilaspur, a WRD reservoir of 210 ha built in 1993, is the State's first Ramsar site, with a designation date of 8 August 2025 on its Ramsar information sheet (it was announced in December 2025). Raipur is still planned under its Development Plan (Revised) 2021, approved in 2008; a draft for 2031 that emphasises water bodies was discussed in 2022, and its approval was not found.

Design for Chhattisgarh therefore has to cover a monsoon that comes first to Bastar and lingers into October, on a plain of paddy clays, limestone and talabs, a plateau of granite and laterite and northern hills of coal, in cities that drain to rivers held up by dams and barrages and, in places, to mine voids. Under IMD's new normals the monsoon sets in at Jagdalpur about 11–13 June, at Raipur and Pendra Road about 16 June and at Bilaspur about 17 June, and withdraws between 7 October at Pendra Road and 12–14 October at Jagdalpur; IMD's Raipur centre uses 10 June and 15 October. About 90% of the year's rain falls from June to September, and July and August bring nearly 58%; annual normals run from 1,011.2 mm in Kabirdham and 1,061.5 mm in Durg to 1,129.3 mm in Raipur, 1,575.7 mm in Bastar and 1,839.3 mm in Dantewada. Of the 100 storms and depressions that affected the State from 1891 to 2014, 27 came in July, 19 in August and 21 in October, and large days fall outside the season: Pendra Road recorded 228.1 mm on 8 October 1994, Ambikapur 206.0 mm on 1 October 1961 and Bailadila 207.0 mm on 11 May 1990. Underground, the State draws less than half of its extractable groundwater (47.32% in 2024), but five blocks of the plain around Raipur, Balod and Bemetara are critical, among them Dharsiwa in Raipur district at 93.51%; after the monsoon the water stood within 5 m of the surface in about 76% of the observation wells in 2023; the Chandi and Charmuria limestones under Raipur, Durg and Rajnandgaon are described by CGWB as "Karstified and Cavernous"; and arsenic at Ambagarh Chowki and fluoride in eight districts limit what may be recharged.

National guidance sizes public drains but does not control the runoff that new building adds. This Manual makes stormwater a condition of development approval. Every development above a modest size must:

- hold frequent rainfall on site;
- release the large storms no faster than the undeveloped land did, or slower where catchments already flood; and
- show that its outfall still works when the receiving nala, river, talab or reservoir is high, when a dam or barrage upstream is releasing, or when the gate on its outfall is shut.

The Manual also sets how public drains, roads, nalas, talabs, reservoirs and other water bodies, gated outfalls and basin-wide master plans are designed, and how runoff is handled where it meets a mine void, an overburden dump or an ash pond, so that private sites and public works aim at the same standard.

The Manual draws on leading stormwater standards from India and abroad. Each is adapted to Chhattisgarh's south-west monsoon from mid-June to mid-October, with the Bay depressions that set most of its records; the Mahanadi and its tributaries, the Seonath, the Kharun, the Arpa, the Hasdeo, the Mand and the Kelo, under the dams and barrages of Gangrel, Bango, Darri, Mongra and the Arpa, and the Hirakud backwater downstream; the Indravati and the Sabari in Bastar, and the Rihand and the Kanhar in the north; the bhata, matasi, dorsa and kanhar soils of the plain, the cavernous

limestone under its cities, the granite, gneiss and laterite of Bastar and the coal measures and their mine-disturbed ground in the north and east; the talabs, dabris and bandhs and the lakes of Nava Raipur; and cities whose new colonies, underpasses and roads lie across the natural lines of drainage.

1. **National base.** The CPHEEO *Manual on Storm Water Drainage Systems* (2019). This Manual builds on it and sets higher standards where it allows a State to do so. IRC road-drainage codes and the NDMA urban flood guidelines are also used.
2. **Overall structure.** Kansas City Metro APWA Section 5600 (Version 2025.01), the most recent comprehensive regional standard. It supplies the preserve, retain, detain, convey and maintain sequence and the required retention volume.
3. **Design rainfall.** The CWC *Flood Estimation Reports* for the subzones that divide the State by basin, as the interim source: subzone 3(d) Mahanadi, for the Mahanadi basin, 56% of the State, with Raipur, Durg–Bhilai, Bilaspur, Korba and Raigarh; 3(g) Indravati, for Jagdalpur and the Bastar plateau; 3(f) Lower Godavari, for Sukma and Konta; 1(d) Sone, for Ambikapur and the north; and a small fringe of 3(c) Upper Narmada, with no city. The ratios of those reports give 1-hour storms larger than any hourly fall recorded in the State, so this Manual sets no floor on short storms; it does check the design day against the largest reliable days recorded nearby where those exceed the maps. IMD's national set of rainfall intensity curves (Das et al., 2022) has no station in Chhattisgarh; the chart of rainfall intensity in the Naya Raipur Development Plan-2031 gives no equation or record length and is used only as a cross-check in Nava Raipur; a set of constants for Raipur from 1979 was seen only in an extract and is not used; and frequency analysis of gridded rainfall by the National Institute of Hydrology (NIH), Roorkee, is used for comparison only.
4. **Monsoon practice.** The PUB Singapore *Code of Practice on Surface Water Drainage* (7th ed., 2025), for the fixed-coefficient route for small sites and drainage reserves. Malaysia's MSMA 2 (2012), for monsoon design storms and simplified on-site detention.
5. **Runoff reduction and treatment.** The Georgia Stormwater Management Manual (2016), for runoff reduction and downstream analysis. CIRIA C753 *The SuDS Manual* (2015), for the treatment train. The Mile High Flood District criteria manual, for outlet and storage design.
6. **Local rules.** The Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973; the Chhattisgarh Bhumi Vikas Niyam, 1984 as amended, applied by T&CP, NRDA and every urban local body; the Chhattisgarh Municipal Corporation Act, 1956 and the Chhattisgarh Municipalities Act, 1961; the Chhattisgarh Land Revenue Code, 1959; the Chhattisgarh Ground Water (Management and Regulation) Act, 2022; the Raipur Development Plan (Revised) 2021 and the Naya Raipur Development Plan-2031; the order of the General Administration Department of 31 May 2016 on rainwater harvesting in government buildings; the Wetlands (Conservation and Management) Rules, 2017; the Coal Mines Regulations, 2017 and the conditions of environmental clearances for mines and power stations; and the orders of the High Court of Chhattisgarh and the NGT on water bodies, rivers and sewage. This Manual follows them or, where disclosed, makes them stricter. No base or consolidated text of the Bhumi Vikas Niyam was found. Amendments of 17 December 2008, 17 February 2009, 6 October 2018 and 29 April 2020 and a draft of 31 March 2017 were read; others were read only in machine transcription, drafts of 12 September and 1 December 2025 and a reclassification of land uses in October 2026 were seen only in summaries, and none of them adds a rule on detention, drainage design or flood-prone land. The Municipal Acts were not read in their Chhattisgarh form, and the Land Revenue Code was read in a third-party copy. Every rule cited must be checked against the current text.

Why this Manual is needed

India has no uniform stormwater design standard. The CPHEEO *Manual on Storm Water Drainage Systems* (2019) is the country's first manual dedicated to storm water drainage, but it is advisory. The Ministry of Housing and Urban Affairs lists it among advisory guidelines and treats urban flooding as a matter for State Governments and urban local bodies. None of the national rules or State building rules reviewed for this Manual makes runoff control a condition of building approval, and neither does the Model Building Bye-Laws 2016.

The national documents leave key values open, or give different answers.

- **Return periods.** CPHEEO recommends 5 years for drains in Class I cities and 2 years elsewhere, and lets a State committee adopt higher values. A 2023 CPHEEO presentation shows a different table. IRC:SP:50-2013 suggests 1–2 years for urban road drains; IRC:SP:42-2014 suggests 25 years. An engineer designing a drain in Chhattisgarh can cite 1, 2, 5 or 25 years, depending on the code chosen.
- **Detention.** CPHEEO describes limiting peak outflow to pre-development rates as general practice. It uses no "shall" and names no storm frequencies.
- **Climate.** The NDMA Urban Flooding Guidelines (2010) call for detention and retention facilities and for rainfall curves adjusted for climate change, but give no sizing rule or return period.
- **Design rainfall.** No national document gives design rainfall that a designer can apply directly to a given town. Each project chooses its own source. IMD's 2022 national set of rainfall intensity curves has no panel for any station in Chhattisgarh. The CWC reports that cover the State's cities date from 1987 (subzone 1(d)) to 1997 (3(d)); they take the 1-hour rain as 0.39 (3(d)), 0.37 (1(d)) and 0.32 (3(g) and 3(f)) of the 24-hour rain, which gives a 50-year hour of about 80–140 mm in the State's cities, while no hourly fall recorded on the self-recording gauges of those reports exceeds about 80 mm (79 mm at Kanker on 26 June 1974). At the other end their maps are exceeded at gauges: Raipur (Obsy) recorded 370.3 mm on 4 August 1910 where the map gives a 100-year day of about 320–360 mm, and Rajnandgaon 392.0 mm on 21 September 1965 against about 320 mm. The reanalysis grids of NIH-DHAARA give a 50-year hour of 59–85 mm, closer to the records, but err both ways on the day: 349 mm at Nava Raipur and 380 mm at Mahasamund, above the maps' 280–320 and 320–360 mm, and 217 mm at Bilaspur and 249 mm at Ambikapur, against about 200–240 and 280 mm.

Chhattisgarh has no drainage design standard of its own. Research for this Manual found no standard, IDF relationship or circular of UADD, T&CP, WRD, RMC or any other Municipal Corporation that fixes a return period or a design rainfall for city storm drains. Raipur's AMRUT plan of 2015 records the failures of its drains but states no design basis, and the CAG's audit of AMRUT in the State, for the year ended March 2022, covers water supply, sewerage and parks and never mentions drains. The one exception is the new capital: the Naya Raipur Development Plan-2031 takes a 5-year intensity for its drain system, 100 years for any river embankment and road levels "at least 0.5 to 1.0 m above the observed high flood level", and follows IRC:SP:50 for road drainage. It is a plan for one city, not a State standard. This Manual is offered as a technical companion to the Bhumi Vikas Niyam, the Municipal Acts and the development plans: it supplies the design storm, detention, water-level and buffer values that they leave open. It has not been adopted by UADD, the Housing and Environment Department or any other body. In particular:

- **The building rules require rainwater harvesting and a suitable site, but not detention.** Rule 78(4) of the Bhumi Vikas Niyam requires rainwater harvesting by the recipes of Appendix ढ (N), which include a filter bed of fine stone and sand and, since 2009, an underground storage tank, with a penalty of up to ₹1,000 a year for every 100 m² of built-up area where the system is not built or kept. A Rajya Sabha answer of 2019 gave its threshold as plots of more than 500 m²; the amendment of 17 February 2009 substitutes the figures "500" and "300" in the rule without saying what they replace, and the rule itself was not read, so the threshold is not confirmed [TBC]. Government buildings with roofs of more than 1,500 sq ft (new) or 5,000 sq ft (existing) must harvest rainwater under an order of the General Administration Department of 31 May 2016. Rule 50 is the rule on unsuitable sites, which T&CP's report format of 2011 asks its officers to check. No rule requires on-site detention, limits peak runoff or sets "zero runoff", and the green-building draft of 2017, which would have required harvesting on plots from 100 m² and unpaved ground in open spaces, was not found in final form;
- **The waterway line was never confirmed.** In the 1984 rules as Madhya Pradesh published them, rule 50(b) bars building within 9 m of the highest water mark and, along a major watercourse, within 9 m of the average high flood mark or 15 m of its defined boundary, whichever is more. Rule 50 is still the Chhattisgarh rule on unsuitable sites, but whether Chhattisgarh kept those distances was not confirmed [TBC]. The inspection form of 2018 asks the distance from the plot to the nearest major nala "in compliance with" the Bhumi Vikas Niyam, and leaves the rule number blank. T&CP's checklist of 2011 asks, for land next to a nala, canal or river, for a no-objection of WRD or the Revenue Department and the "high flood point" from WRD; integrated townships may not be sited on the bank of a perennial river (2008); and the State's Act of 2002 on the regularisation of unauthorised development bars regularising a building on the bed or edge of a reservoir or on a natural drain;

- **No floodplain is zoned.** Chhattisgarh has no floodplain zoning law, no notified flood zone and no demarcation by WRD of the floodplain of any river. The only buffer enforced is the 50 m from the highest flood level of the Wetlands Rules, applied by the State Wetland Authority to talabs and wetlands of more than 2.25 ha; it does not reach the nalas, the rivers or the smaller talabs inside the cities. Apart from Naya Raipur's 100 years for embankments, no design return period was found for any river or nala corridor in the State. This Manual offers a technical method for drawing a design flood line and corridors (Chapter 7); it does not replace any line that the State notifies or a court draws, and where such a line binds and is wider, it governs;
- **Basements and plinths are inspected, but not for floods.** The amendment of 2018 sets basements back 3 m or 6 m from the plot boundary (rule 73(3)(क)) and adds a plinth certificate (rule 96), whose form checks location, orientation and setbacks but no level. No minimum plinth height, flood bar on basements or level for the crest of a basement ramp was found in any amendment read; and
- **No document sets the river or reservoir level for which an outfall is designed.** Of the CWC sites in the State, warning and danger levels were found only for the Indravati at Jagdalpur; on the Kharun at Pathardihi, Raipur's river, only a highest flood level of 281.68 m (2020) was found; no source read states the datum of these levels; and the levels of the Mahanadi barrages and the reach of the Hirakud backwater inside the State were not found. Danger levels and highest flood levels are flood-warning levels and records, not design levels.

So each agency and each local body interprets the central manual its own way. RMC and the other Municipal Corporations, the Nagar Palika Parishads and Nagar Panchayats, RDA, NRDA, T&CP, the Town Services of the Bhilai Steel Plant, WRD, PHED, Smart City companies, the coal and power companies, road agencies, their consultants and funding agencies fill the gaps differently. Two drains on the same road, built by different agencies, can be sized for different storms from different rainfall sources. No rule divides natural nalas from municipal drains: in Raipur a nala is a storm drain, a sewer and a feeder to a treatment plant at once, and in Bhilai the plant's township and the Municipal Corporations each keep their own drains, which empty into the same nalas. And a reviewer has no common basis against which to check a design.

What this Manual does about it. It turns those open choices into one State standard:

- one source and method for design rainfall across the State, in three rainfall zones and six hydrologic sub-areas, with a checked station analysis to govern where one exists, no floor on short storms where the records show the CWC ratios overstate them, and a check of the design day against the largest reliable days recorded nearby, such as Raipur's 370.3 mm and Rajnandgaon's 392.0 mm (Chapter 2);
- named return periods for every drainage element (Chapters 2 and 6);
- a required pre- versus post-development rule for private sites (Chapter 4);
- a required tailwater check for outfalls to nalas, rivers, talabs and reservoirs, gated, flap-gated or free, against a design river or reservoir level, using CWC's levels rather than local gauges and allowing for releases from Gangrel, Bango and the barrages and for the Hirakud backwater (Chapter 5);
- a design flood line, and corridors and setbacks along rivers and nalas and around talabs and reservoirs, which no rule now sizes, proposed by this Manual for use with the inherited waterway line of rule 50, once confirmed, and the 50 m wetland buffer wherever those are wider (Chapter 7);
- recharge first where the water table is drawn down, as in Dharsiwa (Raipur) and Durg–Bhilai, with pre-treatment, but no infiltration devices on kanhar clay, a check for cavities in the limestone under Raipur, Durg and Rajnandgaon, none on mine-disturbed ground or ash ponds, checks for fluoride and arsenic, and treatment where drains carry sewage (Chapter 8);
- rules for runoff that meets a mine void, an overburden dump or an ash pond: a void is not a design outfall unless the mine's lessee and DGMS consent and its level and spill are surveyed, dumps and ash ponds are treated as land that sheds water and sediment fast, and the flow from a breached ash dyke is a hazard to be mapped, not a design inflow (Chapters 3, 5 and 8); and
- basin-wide master plans, so that the agencies that share a basin plan together (Chapter 11).

It uses the route the CPHEEO manual itself provides: a State may adopt standards higher than the national values (CPHEEO 2019, Table 4.1, Note 2). A Municipal Corporation, a development authority, NRDA or UADD can still adopt a stricter standard (Chapter 1).

Sources: CPHEEO (2019) Part A §4.4–4.5 and Table 4.1; CPHEEO presentation to CSE (August 2023); IRC:SP:50-2013 §6; IRC:SP:42-2014 §6; NDMA (2010) Urban Flooding Guidelines, para 6.6.2; MoHUA Model Building Bye-Laws (2016); Government of Chhattisgarh, Housing and Environment Department, amendments to the Chhattisgarh Bhumi Vikas Niyam, 1984: No. एफ 7-26/32/2008 of 17 December 2008 (Appendix ण), No. 318/1193/32/2006 of 17 February 2009 (Gazette p. 124(10), items 25–28), draft No. F 7-15/2016/32 of 30 March 2017 (rules 79-C to 79-G), No. F 7-27/2017/32 of 6 October 2018 (rules 73(3)(क) and 96, Appendices Q, Q1 and Q2) and No. F 07-24/2019/32 of 29 April 2020, from archived copies of the T&CP website; other amendments of 2010–2023 (machine transcription only) and the drafts RULE/170/2025-HOUSING of 12 September and 1 December 2025 and the land-use amendment of October 2026 (summaries only); Madhya Pradesh Bhumi Vikas Rules, 1984, rules 17, 50, 51 and 78 (third-party copy of the Madhya Pradesh text); Rajya Sabha answer of 8 July 2019 on rainwater harvesting (as summarised); Housing and Environment Department letter No. 2940/1445/32/2009 of 23 November 2009; General Administration Department order No. F 9-22/2016/1/5 of 31 May 2016; T&CP Directorate checklist No. 1569/स्टेनो/नग्रानि./2011, Appendix 01, item 15, and letter No. 1680/TCR-2/नग्रानि./2011 of 23 June 2011; Housing and Environment Department order No. F 7-35/2012/32 of 2 January 2013; the Chhattisgarh Act of 2002 on the regularisation of unauthorised development (CG Rajpatra, 26 April 2002), s.7(1); Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973, ss.19, 24, 27–28, 30, 64–69 and 85; Chhattisgarh Land Revenue Code, 1959, ss.237, 248 and 251 (third-party copy); Chhattisgarh Ground Water (Management and Regulation) Act, 2022; notification No. 706/2286/32/2007 of 5 April 2008 (approval of the Raipur Development Plan (Revised) 2021), first page only; notification No. F 7-3/2014/32 of 10 February 2014 (Arpa Special Area Development Plan 2033) and letter No. F 07-02/2019/32 of 14 February 2019; NRANVP letter No. 4808/5(1)/BPS/NRANVP/2022 of 21 June 2022, forwarding notification No. 4066 of 23 May 2022; NRDA, Naya Raipur Development Plan-2031 (2008), Chapters 1–3, 7–9 and 10–12, §10.3 and Fig. 10.1; Raipur Municipal Corporation, Sector-wise SLIP template: Storm Water Drainage (AMRUT, 4 November 2015); CAG, Audit Report (Performance and Compliance Audit) for the year ended 31 March 2022, Government of Chhattisgarh, Chapter II (AMRUT); CECB report in NGT O.A. 593/2017, with the monthly progress report of January 2021 in O.A. 673/2018; NGT (Central Zone), O.A. 78/2024, order of 14 November 2024 and the Collector's action-taken report; reports of the NGT order of 16 January 2026 on Durg's nalas and of High Court orders on the Arpa (2025) and on the rivers (2026); Ramsar Information Sheet, site 2583, Kopra Jalashay; CWC, Flood Estimation Reports for Mahanadi subzone 3(d) (M-3(d)/R-3/25/1996), Indravati subzone 3(g) (I/21/1992), Lower Godavari subzone 3(f) (LG-3(f)/R-2/24/1995) and Sone subzone 1(d) (S/15/1987), plates read by eye; CWC, National Register of Large Dams (2009); CWC flood advisories (2019, 2020) and highest flood levels as transcribed by SANDRP (2020–2026); WRD, Daily Tank Gauge Report of 10 October 2024; IMD, Climate of Chhattisgarh (Climatological Summaries of States Series No. 22, 2015); IMD, Extremes of Temperature and Rainfall for Indian Stations (up to 2012); IMD, Pai et al. (2020), new normal dates of onset and withdrawal of the south-west monsoon, Table 2; IMD Meteorological Centre Raipur, monsoon reports 2020, 2023 and 2024; Das et al. (2022), MAUSAM 73(4); NIH-DHAARA (accessed 7 November 2026); CGWB, Dynamic Ground Water Resources of Chhattisgarh 2024 and factsheet (December 2024); CGWB district brochures for Raipur, Durg and Rajnandgaon; Mandal et al. (2018) and Ranjan and Singh (2018), Journal of Pharmacognosy and Phytochemistry 7(4); Singh et al. (2023), Environment, Development and Sustainability (as summarised); reports of the floods of 2020, 2024, 2025 and 2026, of the dam releases at Gangrel and Bango, of the ash-dyke breaches at Korba (2024–2026), of the State Wetland Authority's actions at Raipur and Nava Raipur (2025), of Raipur's ponds and of the sewage reaching the Kharun (press and secondary sources).

How to use this Manual

Clauses and commentary. Numbered clauses (for example 4.5.2) are the requirements of this Manual and use "shall". Paragraphs headed **Commentary** explain the reason for a clause and cite its precedent. They are guidance, not requirements. Where a value has no direct precedent elsewhere, the commentary says so and explains why this Manual sets it.

Return periods. Storms are described both by annual exceedance probability (AEP) and by return period. For example, "1% AEP (100-yr)" means a storm with a 1-in-100 chance of being equalled or exceeded in any year.

Illustrative values. Worked examples and figures marked "Illustrative — not design values" show method and scale only. Design values come from the sources set in Chapter 2.

Annexes and forms. Each chapter lists the annexes and forms it relies on. They follow the chapters, grouped by chapter, and hold the forms to submit, worked examples, templates, model text and standard drawings. Annexes that need data not yet collected are marked *To be prepared*. They say what they will contain, and the clause that depends on them states the rule that applies in the meantime.

Figures. All figures are original drawings prepared for this Manual. Where a figure follows a published concept, its caption credits the source.

Where to start.

If you are…	Read first
A developer or architect	Chapter 1 (does it apply?), Chapter 4 (what to provide), Chapter 10 (what to submit)
A design engineer	Chapters 2–6, then 8 and 9
A local body or approving authority	Chapters 1, 10 and 11
A planner or waterbody manager	Chapters 7 and 11

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2	Design Rainfall and Design Storms	Annexes 2
3	Hydrologic Methods	Annexes 3
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Abbreviations

Abbreviation	Meaning
AEP	Annual exceedance probability

Abbreviation	Meaning
AMC	Antecedent moisture condition
AMRUT	Atal Mission for Rejuvenation and Urban Transformation
APWA	American Public Works Association
ARF	Areal reduction factor
AWS	Automatic weather station
BALCO	Bharat Aluminium Company Limited, Korba
BOD	Biochemical oxygen demand
BSMP	Basin Stormwater Master Plan
BSP	Bhilai Steel Plant, of the Steel Authority of India (SAIL)
CAG	Comptroller and Auditor General of India
CECB	Chhattisgarh Environment Conservation Board
CGBVN 1984	The Chhattisgarh Bhumi Vikas Niyam, 1984, as amended
CGSDMA	Chhattisgarh State Disaster Management Authority
CGWA	Central Ground Water Authority
CGWB	Central Ground Water Board
CN	Curve number (SCS/NRCS method)
CPCB	Central Pollution Control Board
CPHEEO	Central Public Health and Environmental Engineering Organisation
CSE	Centre for Science and Environment
CSPGCL	Chhattisgarh State Power Generation Company Limited
CWC	Central Water Commission
DGMS	Directorate General of Mines Safety
DPR	Detailed project report
DWF	Dry-weather flow
EC	Environmental clearance (of a mine, power station or project)
EIA	Environmental impact assessment
ESC	Erosion and sediment control
FAR	Floor area ratio
FER	Flood Estimation Report (CWC)
FRL / FTL / MWL	Full reservoir level / full tank level / maximum water level
GAD	General Administration Department, Chhattisgarh
HAT / MHWS	Highest astronomical tide / mean high water springs (coastal terms; not applicable in Chhattisgarh)
HFL	High flood level (the highest level recorded at a gauge)
HGL	Hydraulic grade line

Abbreviation	Meaning
HSG	Hydrologic soil group
IDF	Intensity–duration–frequency
IGKV	Indira Gandhi Krishi Vishwavidyalaya, Raipur
IMD	India Meteorological Department
IRC	Indian Roads Congress
MLD	Million litres a day
MoEF&CC	Ministry of Environment, Forest and Climate Change
MoHUA	Ministry of Housing and Urban Affairs
NDMA	National Disaster Management Authority
NGT	National Green Tribunal (NGT CZ: its Central Zone bench, Bhopal)
NIH	National Institute of Hydrology, Roorkee
NOC	No-objection certificate
NRANVP	Nava Raipur Atal Nagar Vikas Pradhikaran (the present name of NRDA)
NRDA	Naya Raipur Development Authority, now NRANVP
NTPC	NTPC Limited (formerly National Thermal Power Corporation)
OB	Overburden (the rock and soil removed to reach coal)
PHED	Public Health Engineering Department, Chhattisgarh
PSD	Permissible site discharge
RDA	Raipur Development Authority
RMC	Raipur Municipal Corporation
RRV	Required retention volume
RWH	Rainwater harvesting
SADA	Special area development authority (under the 1973 Adhiniyam)
SANDRP	South Asia Network on Dams, Rivers and People
SECL	South Eastern Coalfields Limited, a subsidiary of Coal India
SLA	Sea-level allowance (coastal term; not applicable in Chhattisgarh)
SLIP	Service level improvement plan (AMRUT)
SoI	Survey of India
SRRG	Self-recording rain gauge
STP	Sewage treatment plant
SUDA	State Urban Development Agency, Chhattisgarh
SUH	Synthetic unit hydrograph (CWC)
SW monsoon / JJAS	South-west monsoon, June–September
SWD	Storm water drain

Abbreviation	Meaning
T&CP	Directorate of Town and Country Planning, Chhattisgarh
UADD	Urban Administration and Development Department, Chhattisgarh
ULB	Urban local body (a Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat)
UTM	Universal Transverse Mercator
WL / DL	Warning level / danger level (CWC)
WQv	Water quality volume
WRD	Water Resources Department, Chhattisgarh
WSUD	Water-sensitive urban design

CHAPTER 1

Policy, Applicability and Adoption

1.1 Purpose and objectives

1.1.1 This Manual sets the minimum standards for the planning, design, construction, operation and maintenance of stormwater systems in Chhattisgarh. It applies to private development and to public works, as set out in Clause 1.5.

1.1.2 The Manual has the following objectives:

- (a) to make stormwater management a **condition of development approval**, so that new development and redevelopment do not increase flood peaks or runoff volumes downstream;
- (b) to retain frequent rainfall on site, so that groundwater recharge is sustained and pollutant loads are reduced;
- (c) to size storage for the long-duration south-west monsoon storms of July–September, brought by depressions and lows from the Bay of Bengal, which fall on kanhar and dorsa paddy land already saturated and puddled, on talabs and reservoirs at their full levels and often on rivers raised by releases from Gangrel, Minimata Bango and the barrages upstream, as well as for short-duration intensities;
- (d) to design outfalls for the stage of the rivers into which the towns drain, the Kharun at Raipur, the Seonath at Durg, the Arpa at Bilaspur, the Hasdeo at Korba, the Kelo and the Mahanadi at Raigarh and the Indravati at Jagdalpur, raised by the releases of Gangrel, Minimata Bango, the Hasdeo barrage at Darri and the barrages above the towns; the backwater of the Hirakud reservoir on the low outfalls of Raigarh and Sarangarh; and the levels of the talabs, reservoirs, nalas and mine voids into which local drains discharge, including the case where the outfall is closed;
- (e) to protect tanks (talabs), marshes, rivers, supply channels and surplus courses as part of the drainage system;
- (f) to give every element of the drainage network a safe overland path for the 1% AEP (100-yr) storm;
- (g) to make design inputs, approvals and facility records public, so that designs can be checked and repeated; and
- (h) to make sure that facilities, once built, are maintained and remain effective.

1.1.3 This Manual does not replace the rainwater harvesting (RWH) mandate, the land development rules made by the State Government under section 85(1) read with section 24(3) of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973 (the Chhattisgarh Bhumi Vikas Niyam 1984), or the statutes on waterbodies. It adds runoff control requirements to them (Clause 1.8).

> **Commentary.** Chhattisgarh's floods come from three sources, often together. The first is the heavy storm of a Bay of Bengal depression. Raipur recorded 370.3 mm on 4 August 1910, its highest day and the highest of any IMD observatory in the State, while Kurud recorded 381.0 mm and Arang 318.0 mm on the same day; Raipur recorded 297.0 mm on 30 June 2007, when more than 6,000 people were moved from its low-lying areas (IMD; as reported); and records of 337.0 mm at Durg (15 June 2004), 358.0 mm at Korba (12 August 2004) and 365.0 mm at Jashpurnagar (30 June 2001) have been set since the CWC isopluvial plates were drawn (IMD). In July 2026 Bilaspur was flooded by a fall reported as 28 cm in 24 hours, which another report gives as 415.6 mm; the conflict is not resolved (as reported). The second is the rivers and the dams and barrages on them. On 28–29 August 2020 the Kharun at Pathardihi, below Raipur, rose to 281.68 m, 2.47 m above the 279.21 m of 1994 quoted as its previous highest level, and the Mahanadi at Kalma and Surajgarh and the Mand at Kurubhata set new highest

levels (CWC data as transcribed by SANDRP), while 11,942 houses were reported damaged in the State (as reported). In August 2025 floods in the Bastar division killed eight people (as reported). On 24–27 September 2026 all 14 gates of Gangrel were open, the Kharun overflowed at Mahadev Ghat in Raipur, the Indravati at Jagdalpur rose to 544.891 m, above its highest level of 1973, and eight people died and thousands were rescued across the State (as reported; the reported numbers differ). The Hasdeo at Korba rises with the releases of Minimata Bango and Darri, which flooded parts of the town in August 2024, and the low outfalls of Raigarh and Sarangarh lie within the backwater of the Hirakud reservoir (as reported). The third is the city drainage that fails in a fall that is not extreme. RMC recorded in 2015 that its drains covered 49% of the city, that "most of the drains are overflowed" in the rains and that 11 road stretches flooded two or three times a year (RMC, AMRUT plan), and in July 2026 colonies, underpasses and the railway station of Raipur were waterlogged in a storm for which no rainfall was reported (as reported).

Missing design makes all three worse. The only design storm found for a town's drains in the State is the 5-year intensity of the Naya Raipur Development Plan-2031; RMC's plan for ₹1,191 crore of storm-drain works states no return period, and the CAG's audit of AMRUT in the State, which funded water supply, sewerage and parks in nine Municipal Corporations, does not mention drains at all (CAG, year ended March 2022). The CGBVN 1984 control rainwater harvesting systems, not runoff. Objectives (b) to (e) address the storage, outfall and building-level failures, which a drain-sizing manual alone cannot fix.

No national instrument requires on-site detention. CPHEEO (2019) is advisory and describes peak control only as typical practice. NDMA (2010) says detention and retention "will" be included but gives no sizing rule. MBBL 2016 and the Chhattisgarh Bhumi Vikas Niyam 1984 mandate RWH only. Objective (a) is therefore the main new step this Manual takes.

1.2 Stormwater management hierarchy

1.2.1 Every development to which Chapter 4 applies shall consider stormwater measures in the following order. A designer shall not use a lower measure in place of a higher one that is practicable on the site.

1. **Preserve** — keep or restore natural drainage features, tanks, wetlands, permeable soils, trees and overland flow paths (Chapter 7).
2. **Retain** — hold the runoff from the retention depth of Annex 2E on site, by infiltration, reuse or evapotranspiration (Chapters 4 and 8).
3. **Detain** — store larger storms temporarily and release them at or below the rates in Chapter 4, checked against downstream and tailwater conditions (Chapters 4 and 5).
4. **Convey** — carry the remaining flow safely in the minor system, and the 1% AEP flow in the major system (Chapters 6 and 9).

1.2.2 **Maintenance** underlies all four measures. A measure that cannot be maintained by its owner under Chapter 10 shall not be credited.

1.2.3 Credits from higher measures reduce the requirements of lower ones. Preserved areas and retained volume reduce detention storage as set out in Chapter 4.

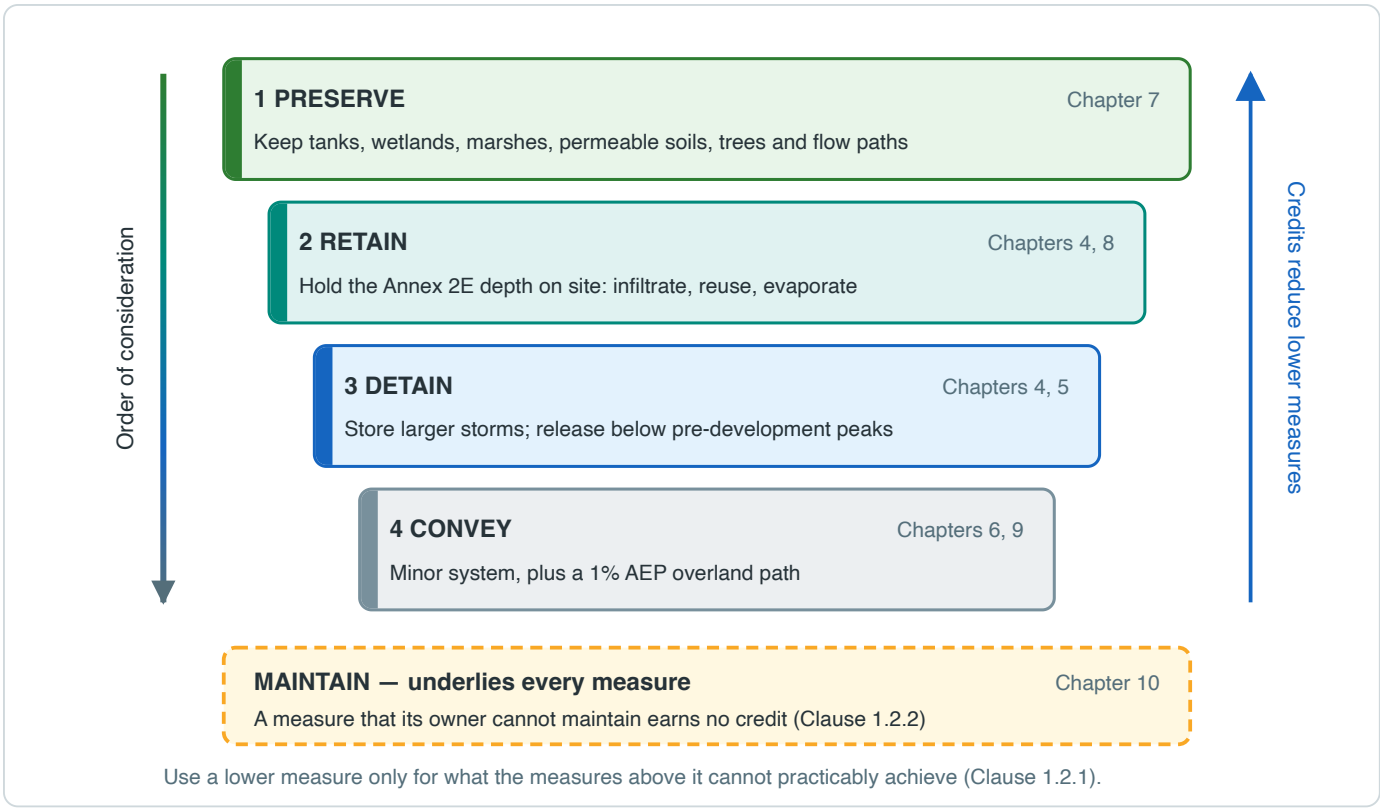

Figure 1.1 — Stormwater management hierarchy: preserve, retain, detain, convey, with maintenance underlying each measure. Original drawing for this Manual, adapted from Mid-America Regional Council / Kansas City Metro Chapter APWA, "Layers of Stormwater Management", APWA 5600 Manual Factsheet, 2025 (concept).

Commentary. The order follows the "layers" of Kansas City Metro APWA Section 5600, Version 2025.01: preserve or restore, retain, detain, convey, maintain (MARC factsheet, 2025). The same logic is behind Georgia's runoff-reduction standard and CSE's "attenuate and infiltrate first" principle (CSE, 2023). For Chhattisgarh, the top layer has a particular meaning. Much of the State's flood storage lies in its talabs, dabris (small dug ponds) and bandhas, in the bunded paddy fields of the plain, and in the reservoirs and barrage ponds on its rivers. Raipur was a city of tanks: it is reported to have had about 130 ponds, of which some 37–40 remain, over about 69 ha, most of them silted and polluted (as reported). Nava Raipur Atal Nagar was planned on a network of some 55 lakes and reservoirs, among them Sendh, Jhanjh and Khandawa, with its four major streams to be preserved and its parks acting as "storm water carriers" to the lakes (NRDA, Naya Raipur Development Plan-2031; as reported). Kopra Jalashay near Bilaspur, a 210 ha reservoir built in 1993, became the State's first Ramsar site, designated on 8 August 2025 and announced in December 2025 (Ramsar Information Sheet; as reported). The law protects some of this storage. Village tanks over which the community exercised rights of irrigation or nistar vest in the State (Chhattisgarh Land Revenue Code, 1959, s.251), and encroachment on ponds recorded in the revenue record is proceeded against as unauthorised occupation (s.248), as at Baloda after an order of the NGT (O.A. 78/2024); the Chhattisgarh Act of 2002 on the regularisation of unauthorised development bars the regularisation of construction on the bed or edge of a reservoir or on a natural drain (s.7(1)(चार)); the inherited text of the CGBVN 1984 bars a building within 9 m of the highest water mark of a water body and within 9 m of the average high flood mark or 15 m of the boundary of a major watercourse (Rule 50(b)), a distance whose Chhattisgarh text is to be confirmed [TBC]; and the State Wetland Authority has applied the bar of the Wetlands Rules 2017 on works within 50 m of the highest flood level to Raipur's talabs and to the Jhanjh wetland (as reported). The Municipal Corporation Act 1956, in its Madhya Pradesh text, vests public tanks, streams and drains in the Corporation and forbids any other use of a vested tank (s.82); its Chhattisgarh text was not read [TBC]. No floodplain zoning law protects the rivers. Losing that storage cannot be offset economically by pipes and pumps.

1.3 Legal basis and status

1.3.1 **Notification.** This Manual shall be given statutory effect by the Government of Chhattisgarh through:

- (a) an amendment to the Chhattisgarh Bhumi Vikas Niyam 1984 that makes compliance with this Manual a condition of planning permission, building permit, layout approval and completion certificate, for development to which Clause 1.5 applies; and
- (b) bye-laws or standing orders of each urban local body (ULB), adopting the Manual for works executed or approved by that ULB, including the ULB's own drains and roads.

1.3.2 **Higher State standards.** The design return periods and performance criteria of this Manual exceed the national advisory minimums of the CPHEEO Manual on Storm Water Drainage Systems (2019). The State shall constitute, by notification, a high-powered committee (the **State Stormwater Technical Committee**, Clause 1.9) to adopt these higher standards. This is the route that CPHEEO (2019) provides for higher return periods.

1.3.3 **Precedence.** Within its scope, this Manual governs where it is stricter than CPHEEO (2019), IRC:SP:42-2014, IRC:SP:50-2013 or any departmental standard. Where a statute or statutory rule sets a stricter requirement (for example a waterbody setback), the statute governs.

1.3.4 **Design-practice standard for public works.** Departments and ULBs that procure drainage, road or waterbody works shall require compliance with this Manual in detailed project reports (DPRs) and contracts, from the dates in Clause 1.11.

> Commentary. In Chhattisgarh the building rules are still the Bhumi Vikas Niyam of 1984 that the State inherited from Madhya Pradesh in 2000, retitled the Chhattisgarh Bhumi Vikas Niyam 1984 and amended since by the Housing and Environment Department, under section 85(1) read with section 24(3) of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973; unlike Madhya Pradesh, which replaced its 1984 rules in 2012, Chhattisgarh has not made new rules. Neither the base text nor a consolidated text of the Niyam was found, so the Chhattisgarh text of Rules 2 (definitions), 50, 51, 73 and 78(4) was not read [TBC]; this Manual cites the amendments it read. The amendment of 17 December 2008 (No. एफ 7-26/32/2008) provides for integrated townships, makes a sewage treatment plant and rainwater harvesting compulsory in them and bars them beside a perennial river (Rule 94-क and Appendix ण); that of 17 February 2009 (No. 318/1193/32/2006) substitutes the figures "500" and "300" in Rule 78(4), the rainwater harvesting rule, without saying which figures they replace, adds a penalty of up to ₹1,000 a year for every 100 m² of built-up area where the system is not built and maintained, and adds a storage tank to the systems of Appendix ढ (N); that of 6 October 2018 (No. F 7-27/2017/32) sets basement setbacks of 3 m and 6 m from the plot boundary (Rule 73(3)(क)), introduces a plinth certificate (Rule 96), and adds a site-inspection form that asks whether a major nala adjoins the plot and at what distance, under a rule whose number the Gazette leaves blank; and that of 29 April 2020 (No. F 07-24/2019/32) has no water, drainage, basement or rainwater content. Drafts of September and December 2025 and a reclassification of land uses in October 2026 are reported, none of them on drainage (as reported). Rule 50 is still the Chhattisgarh rule on the unsuitability of a site, as the Directorate of Town and Country Planning's report format of 2011 shows; in the inherited 1984 text it bars a building within 9 m of the highest water mark and, for a major watercourse, within 9 m of the average high flood mark or 15 m of its defined boundary, whichever is more, and on a site that is not properly drained (Rule 50(b) and (f)), but whether the Chhattisgarh text keeps those distances is to be confirmed [TBC]. The rules set no design storm, return period, release rate or detention volume, and no "zero runoff" rule. The route of Clause 1.3.1(a) therefore runs through the State Government, which would amend the Niyam under section 85 of the Adhiniyam. This Manual is a draft; neither the Government nor any authority has adopted it. MoHUA told Parliament in July 2026 that urban flooding "falls under the purview of the State Governments and the ULBs" and listed the CPHEEO manual among "advisory guidelines" (Lok Sabha AU1847). The State therefore needs no central approval to adopt higher standards. CPHEEO's Note 2 to Table 4.1 (§4.4.1.4) allows "a high powered committee constituted by State/UT Government through a notification" to adopt higher return periods. Clause 1.3.2 uses that route so that higher standards are on record for centrally funded schemes.

1.4 Definitions

1.4.1 The following terms apply throughout this Manual. Chapter-specific terms are defined in each chapter (for example Clause 2.2).

Term	Meaning in this Manual
Approving Authority (the Authority)	The body that grants planning permission, building permit or layout approval for a development, or that sanctions a public work: the Directorate of Town and Country Planning, or an officer authorised by it, which grants permission for the development of land in a planning area under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973; the Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat that grants building permission under the CGBVN 1984; NRDA, the Special Area Development Authority of Nava Raipur Atal Nagar, in its special area; or RDA or another authority constituted under the 1973 Adhiniyam in its scheme area, where it grants such permission (the division of these powers, and the bodies that grant permission outside planning areas, are to be confirmed [TBC]), or a department for its own works.
Applicant	The owner, developer or department that seeks approval, and its successors in title.
Development	Any building, layout, sub-division, road, paving or land-use change that requires approval under the Chhattisgarh Bhumi Vikas Niyam 1984 or that is executed as a public work.
Redevelopment	Development on a site where impervious cover already exists.
Land disturbance	Clearing, grading, excavation, filling or paving of land, measured in plan area over the whole of a common plan of development (Clause 1.5.5).
Impervious area	Roofs, paving, compacted gravel, decks and other surfaces that prevent infiltration. Permeable paving designed to Chapter 8 is counted as pervious only for the area it treats.
New or replaced impervious area	Impervious area created, or removed and rebuilt down to the subgrade, by the development. Resurfacing that leaves the base in place is not "replaced".
Retention	Permanent removal of runoff from the drainage system on site, by infiltration, reuse or evapotranspiration (Chapter 4).
Detention	Temporary storage of runoff with a controlled outlet, emptied after the storm (Chapter 4).
Rainwater harvesting (RWH)	Collection of rainfall for supply or recharge under the RWH mandate. RWH storage is not detention unless Chapter 4 allows it.
Minor system	Drains, pipes and inlets designed for the minor-system AEP of Chapter 6.
Major system	The overland and floodway path that carries the 1% AEP (100-yr) flow (Chapter 6).
Pre-development condition	Greenfield: open land in good condition for the mapped hydrologic soil group. Existing impervious cover is not counted (Chapter 3).
Flood-sensitive catchment	The catchments of the nalas and drains of Raipur and Birgaon that discharge to the Kharun, with the talabs on them and the low-lying colonies that drain to them; the land of Bilaspur that drains to the Arpa below the Arpa Bhaisajhar barrage; the land of Durg, Bhilai, Bhilai-Charoda and Risali that drains to the Seonath (Shivnath) through the Shankar and Pulgaon nalas and the other nalas of the twin cities; the land of Korba that drains to the Hasdeo below the Minimata Bango dam and the Hasdeo barrage at Darri; the land of Raigarh and Sarangarh that drains to the Kelo, the Mand or the Mahanadi within reach of the backwater of the Hirakud reservoir; and the land of Jagdalpur that drains to the Indravati, and any catchment the Authority notifies (Chapter 4; Annex 1B).
Waterbody	A river, stream, canal, tank (talab), lake, pond, marsh, supply channel or surplus course, whether perennial or seasonal (Chapter 7).
Regional facility	A detention, retention or tank storage facility serving more than one development, built or restored under a basin plan (Chapter 4).
Tier A / Tier B authority	An Approving Authority classed under Clause 1.6.

Term	Meaning in this Manual
Policy supplement	The set of adoptable alternative clauses in Clause 1.7 and Annex 1C.
Drainage report	The preliminary or final report required by Chapter 10.
Registered professional	A person registered under the Chhattisgarh Bhumi Vikas Niyam 1984 to prepare and sign the relevant plans, with any further qualification required by Clause 10.6.
Compliance calculator	The State-issued tool for computing and reporting compliance (Clause 10.5).
Maintenance declaration	The registered undertaking by an owner to maintain a stormwater facility (Clause 10.10).
Variance	Approval to depart from a numeric requirement, with equivalent performance shown (Clause 1.10).
Waiver	Approval that a requirement does not apply to a site, on the grounds in Clause 1.10.
Notified	Published by the Government or the State Stormwater Technical Committee in the Chhattisgarh Gazette (Rajpatra).

> Commentary. "Replaced" impervious area is defined to the subgrade so that redevelopment of a fully paved site is captured, while routine resurfacing is not. This follows the approach of the APWA 5600 (2025.01) redevelopment triggers (MARC Supplement, 2025).

1.5 Applicability

1.5.1 **Who is covered.** This Manual applies to:

- (a) all development requiring planning permission, building permit or layout approval under the Chhattisgarh Bhumi Vikas Niyam 1984;
- (b) all drainage, road, bridge, culvert, waterbody and flood-mitigation works executed by a ULB, a State department, a board, or a State undertaking; and
- (c) works executed by Central agencies or their contractors within Chhattisgarh, to the extent that the Authority's approval or no-objection is required.

1.5.2 **Retention and detention threshold.** Chapters 4 and 5 (retention, detention, downstream and tailwater analysis) apply to any development that, over the common plan of development, involves:

- (a) land disturbance of **0.1 ha or more**; **or**
- (b) new or replaced impervious area of **500 m² or more**.

1.5.3 **RWH.** The existing RWH mandate continues to apply under Rule 78(4) of the CGBVN 1984, as amended on 17 February 2009, to buildings on plots above a threshold reported as **500 m²** (the Chhattisgarh text of the rule was not read, and the amendment substitutes the figures "500" and "300" [TBC]), by the systems of Appendix ढ (N), with a penalty of up to ₹1,000 a year for every 100 m² of built-up area where the system is not built and maintained; and, under the General Administration Department order of 31 May 2016, to government buildings with roofs of more than 1,500 sq ft (new) or 5,000 sq ft (existing), as now. This Manual does not change it.

1.5.4 **Provisions that apply at every size.** The following apply to all development under Clause 1.5.1, whatever its size, unless exempted by Clause 1.5.8:

- (a) no sewage connection to a storm drain (Chapter 8);

- (b) lowest habitable floor and openings at or above the 1% AEP level plus 0.5 m, where a 1% AEP level is mapped or determinable (Chapter 6);
- (c) waterbody setbacks and the no-fill zone (Chapter 7); and
- (d) no obstruction of a natural drainage path or major-system flow path (Chapter 6).

1.5.5 **Common plan of development.** The thresholds of Clause 1.5.2 apply to the total area of a development, including all phases, plots and sub-divisions under one layout approval or one ownership at the date of application. A development shall not be divided to avoid the thresholds.

1.5.6 **Redevelopment.** For redevelopment, the thresholds apply to new plus replaced impervious area. Retention and detention apply to the whole of the new and replaced impervious area, and the pre-development condition is greenfield (Clause 1.4). Where greenfield control cannot be achieved on a constrained redevelopment site, the relief route of Clause 4.5.6 applies: a minimum on-site cut of 50% from existing peaks, with the shortfall paid as fee-in-lieu into basin storage.

1.5.7 **Linear public works.** For roads, widening and other linear works, the impervious area threshold applies to the added impervious area within each drainage catchment crossed. Retention and detention shall be provided within the right of way, or in a regional facility under Chapter 4, where the right of way cannot hold them.

1.5.8 **Exemptions.** The following are exempt from Chapters 4 and 5:

- (a) a single dwelling on its own plot, where the thresholds of Clause 1.5.2 are not met; and
- (b) linear road resurfacing that adds no impervious area.

Exempt development remains subject to the RWH mandate and to Clause 1.5.4.

1.5.9 **Applicability determination.** The Applicant shall record the applicability decision on **Form 1-1** (Applicability Determination) with the first submittal under Chapter 10. The Authority may confirm or correct it at the pre-application stage.

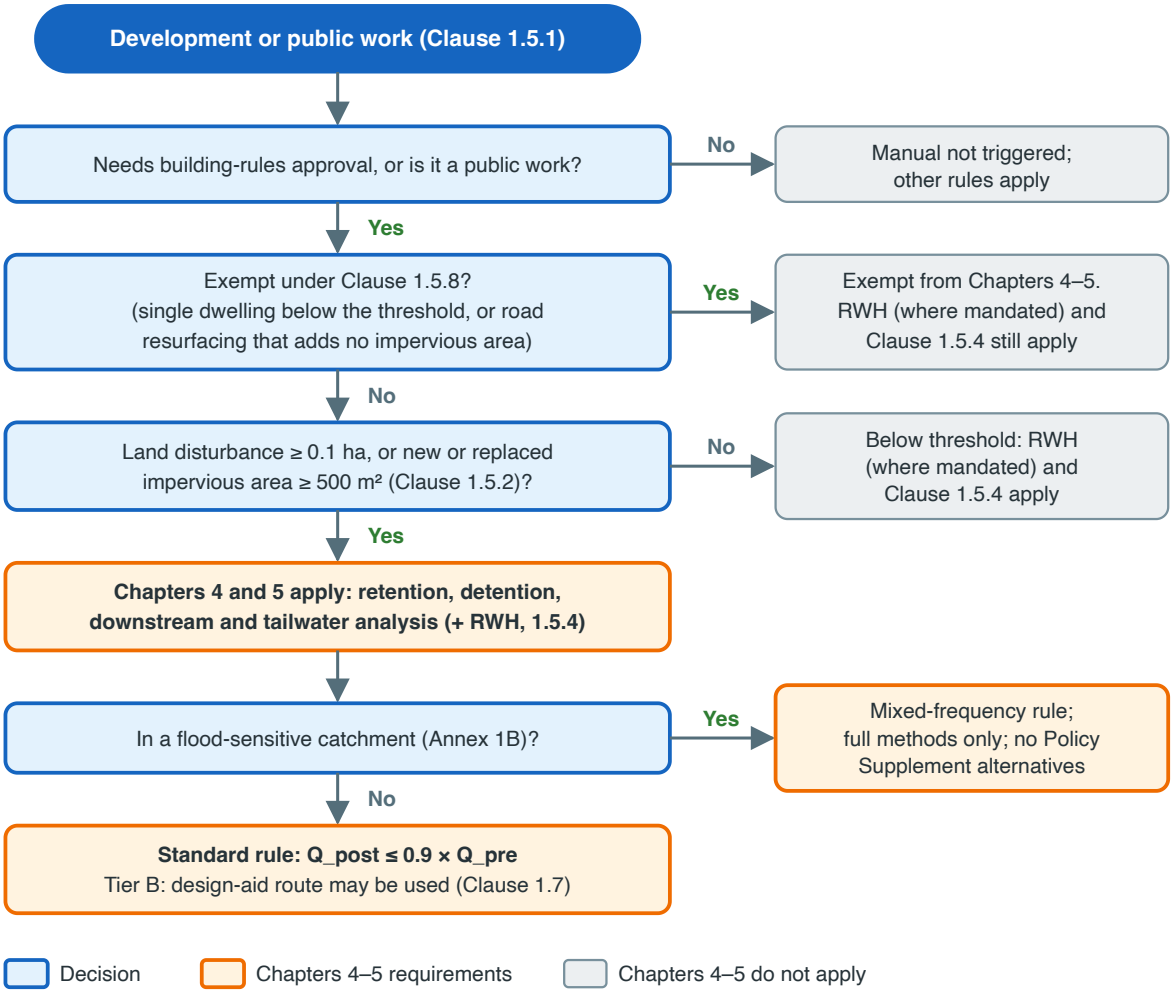

Figure 1.2 — Applicability decision flowchart for Clause 1.5. Original drawing for this Manual.

Commentary. The thresholds have no single direct precedent; they are set by this Manual. Precedents span a wide range:

- Singapore controls peak runoff from developments of 0.2 ha or more (PUB Code of Practice on Surface Water Drainage, 7th ed., cl. 7.1.5).
- Malaysia's 2008 DID checklist used on-site detention for sites up to 0.1 ha and community ponds above that.
- The APWA 5600 (2025.01) Policy Supplement offers alternative triggers of ¼ acre (about 0.1 ha) disturbed, or 1 acre (about 0.4 ha) with 10,000 sq ft (about 930 m²) impervious, and warns agencies not to exceed a 1-acre trigger.
- MBBL 2016 makes recharge mandatory for buildings over 500 m².

The 0.1 ha / 500 m² trigger sits at the strict end of these, because typical Chhattisgarh plots are small and the cumulative effect of many small plots is what fills Raipur's drains. For small buildings in Chhattisgarh, Rule 78(4) of the CGBVN 1984 requires rainwater harvesting on plots above a threshold reported as 500 m² (Rajya Sabha answer of 8 July 2019); the amendment of 17 February 2009 substitutes the figures "500" and "300" in the rule without saying what they replace, so the threshold is to be confirmed from the Chhattisgarh text [TBC]. The systems are fixed recipes in Appendix ढ (N), among them a filter bed of fine stone and fine sand (ढ-2) and an underground storage tank whose water is used in the house and garden (ढ-4); none is sized to a runoff volume, and none limits what leaves the plot. Plots below the threshold have no rule at all. On the kanhar soils of the plain, pits that take water while the clay is cracked in June stand full once the cracks have closed. The plinth certificate

introduced in 2018 (Rule 96 and Appendix Q1) checks the location, setbacks and deviations of a building, but has no item for its plinth level or for drainage, and a draft of 2017 that would have required rainwater harvesting on plots above 100 m² and kept 20% of recreational open space unpaved was not found in final form [TBC]. Clause 1.5.4(b) applies to every building whatever its size for this reason.

Clause 1.5.4(b) has no direct precedent; this Manual sets it because floor levels are the cheapest protection a building can have, and it would be perverse to protect only large developments. Where no 1% AEP level is mapped, Chapter 6 gives the method for small sites.

1.6 Tiered adoption

1.6.1 **Tier A authorities.** The following are Tier A:

- (a) Raipur Municipal Corporation (RMC);
- (b) every Approving Authority within the planning area of Raipur under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973, as that area is notified from time to time, including the Directorate of Town and Country Planning where it grants development permission, RDA in its scheme areas, and the Municipal Corporations, Nagar Palika Parishads and Nagar Panchayats in that area; and NRDA, the Special Area Development Authority of Nava Raipur Atal Nagar, in its special area; and
- (c) every other Municipal Corporation (Nagar Palik Nigam) constituted under the Municipal Corporation Act, 1956 as adapted in Chhattisgarh, as constituted from time to time, among them Bilaspur, Durg, Bhilai, Bhilai-Charoda, Risali, Birgaon, Rajnandgaon, Korba, Raigarh, Jagdalpur, Ambikapur, Dhamtari and Chirmiri, and the Directorate of Town and Country Planning and every development authority constituted under the 1973 Adhiniyam where it grants development permission within the planning area of a Municipal Corporation.

1.6.2 **Tier B authorities.** All other Nagar Palika Parishads and Nagar Panchayats are Tier B.

1.6.3 **What differs between tiers.** All criteria of this Manual apply in both tiers. The tiers differ only in method and review:

Table 1.1 — Differences between Tier A and Tier B authorities

Item	Tier A	Tier B
Performance criteria (Chapters 2–9)	Apply in full	Apply in full
Hydrologic method for storage	Hydrograph method (Chapter 3)	Hydrograph method, or the design-aid route of Clause 1.7 where eligible
Compliance calculator (Clause 10.5)	Required as a summary	Required; may be the sole calculation on the design-aid route
Policy supplement alternatives (Clause 1.7)	Only with State Stormwater Technical Committee concurrence	May be adopted by council resolution
Technical review	By the Authority	By the Authority, with T&CP regional support or an empanelled reviewer (Clause 10.7)
Flood-sensitive catchment rule	Applies	Applies

1.6.4 **Moving between tiers.** A Tier B authority may opt into Tier A by council resolution. The Government may notify a Tier B authority as Tier A when it becomes a Corporation, joins the planning area of Raipur under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973, or the special area of Nava Raipur Atal Nagar, or is found by a post-event review (Clause 10.16) to need the full methods.

1.6.5 **Flood-sensitive catchments override tier.** In a flood-sensitive catchment, whatever the tier, the alternatives of Clause 1.7 are available only as Table 1.2 allows: S-1 (the design-aid route) for sites of 0.5 ha or less (Clause 4.14.1), and S-5 fee-in-lieu where a basin plan has been notified (Clause 1.10.3(d)). S-2, S-3 and S-4 are not available.

> **Commentary.** Raipur Municipal Corporation, with its engineering wing and a city whose nalas, talabs and some 480 km of pucca drains discharge to the Kharun, and a small Nagar Panchayat with one assistant engineer cannot be asked to review the same hydrograph models. The Kansas City solution was to keep one regional standard and publish adoptable alternatives for the most contested criteria, so that smaller agencies can "opt down" without leaving the standard (MARC Supplement, Nov 2025). The tiers here keep the performance criteria the same in both tiers and vary only the method, so that a building in a Tier B town is protected to the same level as one in Raipur.

1.7 Policy supplement: adoptable alternatives

1.7.1 The Government shall publish, with this Manual, a **Policy Supplement** (Annex 1C) of alternative clauses. An Approving Authority may adopt one or more alternatives under the conditions in Clause 1.7.3.

1.7.2 The alternatives are:

Table 1.2 — Policy supplement: adoptable alternatives S-1 to S-5

No.	Main-text requirement	Adoptable alternative	Eligibility
S-1	Storage sized by hydrograph method (Chapter 3)	**Design-aid route:** retention and detention volumes taken from the unit-storage tables (m³ per ha, by rainfall zone, soil group and impervious fraction) in Annex 4 and the compliance calculator	Sites ≤ 2 ha outside flood-sensitive catchments; ≤ 0.5 ha within them (as Clause 4.14.1)
S-2	Release rate Q_post(T) ≤ 0.9 × Q_pre(T), computed for the site	**Permissible site discharge (PSD):** a tabulated unit-area release rate (m³/s/ha) by zone and AEP, derived from the 0.9 × Q_pre rule	Any site outside flood-sensitive catchments
S-3	Waterbody setback extended to the mapped 1% AEP flood extent plus margin (Chapter 7)	**Fixed-width setback table** by waterbody class, where no 1% AEP extent is mapped. Never less than the Zone 1 minimum of Clause 7.4.7	Waterbodies with no mapped 1% AEP extent
S-4	Downstream analysis by hydraulic model (Chapter 5)	**Screening checklist** of downstream capacity and known flooding, with a model only where the screen fails	Sites < 1 ha, outside flood-sensitive catchments, not discharging to a tidal outfall
S-5	On-site detention	**Fee-in-lieu** toward a regional facility or restored tank storage in a master-planned basin (Chapter 4)	Where a basin plan has been notified

1.7.3 **Conditions on adoption.**

- (a) An alternative shall not lower any criterion below the values of this Manual. S-1 and S-2 are simplified methods for meeting the same criterion. They are not lower criteria.
- (b) A Tier B authority adopts an alternative by council resolution, sent to the State Stormwater Technical Committee within 30 days. A Tier A authority needs the Committee's concurrence before adoption.
- (c) The adopted alternatives shall be published on the Authority's website and in the register of Clause 10.17.
- (d) Retention (Annex 2E depth) cannot be replaced by any alternative, except where fee-in-lieu under S-5 funds equivalent retention in the same basin.
- (e) The tables for S-1, S-2 and S-3 shall be issued by the State Stormwater Technical Committee, and reviewed with Chapter 2 (Clause 2.14).

Commentary. The supplement follows the model of the APWA 5600 (2025.01) Policy Supplement, which gives alternatives for the four most contested criteria (applicability, Rational Method limits, stream setback trigger, and Zone 2 setback width). Two of those map onto S-3 and onto the applicability threshold of Clause 1.5.2. Alternative S-3 copies the APWA answer to the same problem: the 1% AEP extent is not mapped for small drainage areas, so a fixed-width table is offered. S-2 follows the Kansas City 2011 shift from "match existing" to fixed unit-area release rates. Local agencies preferred a set rate because existing conditions were hard to determine consistently (MARC Evolution deck). It also reflects the Sydney Water permissible site discharge (PSD) method. The 2 ha / 0.5 ha limits for S-1 (matching Clause 4.14.1) and the 1 ha limit for S-4 have no direct precedent; they are set by this Manual.

1.8 Relationship with other instruments

1.8.1 This Manual shall be applied together with the instruments below. Where they overlap, Clause 1.3.3 governs.

Table 1.3 — Relationship with other instruments

Instrument	Relationship
Chhattisgarh Bhumi Vikas Niyam, 1984, as amended (the rules of 1984 inherited from Madhya Pradesh in 2000; amendments read: No. एफ 7-26/32/2008 of 17 December 2008, No. 318/1193/32/2006 of 17 February 2009, No. F 7-27/2017/32 of 6 October 2018 and No. F 07-24/2019/32 of 29 April 2020; base and consolidated text not read [TBC])	Continue. Rule 50 is the rule on the unsuitability of a site, which the Directorate of Town and Country Planning applies in its site reports (letter No. 1680/TCR-2/नग्रानि./2011 of 23 June 2011); in the inherited 1984 text it bars a building within 9 m of the highest water mark and, for a major watercourse, within 9 m of the average high flood mark or 15 m of its defined boundary, whichever is more, and on a site not properly drained or incapable of being well drained (Rule 50(b) and (f)), and Rule 51 requires surface water drains in the curtilage where the Authority so requires; whether the Chhattisgarh text keeps these words and distances is to be confirmed [TBC]. The amendment of 2018 adds a site-inspection form that records the distance of a major nala from the plot under a rule whose number the Gazette leaves blank, a completion form that asks whether rainwater harvesting has been installed and the drainage line connected, a plinth certificate (Rule 96) with no level item, and basement setbacks of 3 m and 6 m (Rule 73(3)(क)); integrated townships must have a sewage treatment plant and rainwater harvesting, may not be laid out beside a perennial river, and do not count water sources in their area (Appendix ण, 2008, and its amendments). The Niyam define no water body, watercourse, nala or highest flood level in any amendment read, and set no design storm, release rate or detention volume. The distances of Rule 50 are the only metre values found for water; the design criteria of Chapters 2–6 and the hydraulic corridor and setbacks of Chapter 7, whose Zone 1 is at least 15 m, are this Manual's proposals; they are not statutory values, and no authority has adopted them. Where the Niyam, a development plan or a statute are stricter, they govern (Clause 1.3.3).
Rainwater harvesting: CGBVN 1984, Rule 78(4) and Appendix ढ (N); General Administration Department order No. F 9-22/2016/1/5 of 31 May 2016; Chhattisgarh Ground Water (Management and Regulation) Act, 2022	Continue unchanged. Rule 78(4), as amended on 17 February 2009, makes rainwater harvesting compulsory on plots above a threshold reported as 500 m² (Rajya Sabha answer of 8 July 2019; the amendment substitutes the figures "500" and "300" [TBC]), by the systems of Appendix ढ, with a penalty of up to ₹1,000 a year for every 100 m² of built-up area where the system is not built and maintained. The General Administration Department requires rainwater harvesting on government and public-sector buildings with roofs of more than 1,500 sq ft (new) and 5,000 sq ft (existing), and the Naya Raipur Development Plan-2031 makes it mandatory for every development there. The Ground Water Act of 2022 would let the State enforce rainwater harvesting and recharge in notified over-exploited and critical blocks and stressed urban areas (s.7), but it comes into force only on a date notified by the State, and no such notification, and no notified area, was found [TBC]; five blocks were critical in 2024, among them Dharsiwa in Raipur district (CGWB). None of these gives an outlet rule or a storage that is empty when a storm arrives. RWH storage is not detention unless it is a dedicated empty-on-arrival volume with an outlet, and tanks are assumed full at the start of SW monsoon storms (Chapter 4). Recharge structures may count toward retention only where they meet Chapters 4 and 8, which give recharge priority in the critical and semi-critical urban blocks, with pre-treatment, a cavity check in the karstified limestone of the Raipur–Durg–Rajnandgaon belt and fluoride and arsenic checks where the groundwater carries them, and allow no infiltration credit on kanhar, on mine-disturbed ground or on ash ponds.

Instrument	Relationship
Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973 (No. 23 of 1973) and the Chhattisgarh Nagar Tatha Gram Nivesh Niyam, 1975	The Adhiniyam provides for development plans (s.19), for Special Area Development Authorities (ss.64–69) and for the concurrence of the Directorate of Town and Country Planning before State departments and local authorities build or develop land (ss.27 and 28, as a Housing and Environment Department letter of 23 November 2009 reminded every department), and it is the parent Act of the CGBVN 1984; its sections were read only as cited in notifications and letters [TBC]. Raipur is planned under its Development Plan (Revised) 2021, approved on 5 April 2008, and a draft plan for 2031 was discussed in 2022 but its approval was not found [TBC]; NRDA, the Special Area Development Authority of Nava Raipur Atal Nagar, exercises the Adhiniyam's powers there (notification No. 4066 of 23 May 2022). The Directorate's checklist of 2011 asks, for land next to a nala, canal, river or watercourse, for the no-objection of WRD or the Revenue Department and for the high flood point from WRD (No. 1569/स्टेनो/नग्रानि./2011), the only flood-level requirement found in the planning process. This Manual does not decide zoning; the drain reserves and regional facility sites of Chapters 4 and 7 are meant to be shown in the development plans, and its checks are meant to be made at the Directorate's site report and checklist.
Chhattisgarh Act on the regularisation of unauthorised development, 2002, s.7(1)(चार)	Continues. No unauthorised development may be regularised where it stands on the bed or the edge of a reservoir or on a natural drain (CG Rajpatra, 26 April 2002); whether the later Acts and Rules of regularisation keep this bar is to be confirmed [TBC]. Chapter 7 relies on it to keep drain and tank reserves out of regularisation.
Municipal Corporation Act, 1956 and Municipalities Act, 1961, as adapted in Chhattisgarh (the Chhattisgarh Nagar Palik Nigam Adhiniyam and Nagar Palika Adhiniyam)	Continue. Their Chhattisgarh texts were not read for this Manual [TBC]. In their Madhya Pradesh form, from which they were adapted, drains, sewers and culverts are among the Corporation's duties; public tanks, streams, drains and watercourses vest in the Corporation, and a vested tank shall not be used for any other purpose (s.82); no building may be erected over a drain vested in the Corporation without permission (s.198); and the Commissioner may require separate drains for sewage and for rain water (s.204). Whether the Chhattisgarh Acts keep these sections and numbers is to be confirmed [TBC]. A memo of RMC of 2011 cites the colonisers' rules made under them for the sewage treatment, rainwater harvesting and waste arrangements of new colonies (read from a poor scan [TBC]). This Manual does not change those powers; Chapter 7 treats tanks as drainage assets.
Chhattisgarh Land Revenue Code, 1959, ss.248 and 251	Continue. Tanks on unoccupied land over which the village community exercised rights of irrigation or nistar vest in the State from 6 April 1959 (s.251), and unauthorised occupation of land, including a pond recorded in the revenue record, is met by eviction and penalty (s.248), as at Baloda in Janjgir-Champa district after the NGT directed the Collector to remove encroachments (O.A. 78/2024, order of 14 November 2024). Chapter 7 relies on these sections, and on the land records of the tehsil, for the recorded extent of village talabs.
Wetlands (Conservation and Management) Rules, 2017; the Chhattisgarh State Wetland Authority; Kopra Jalashay, Ramsar Site No. 2583	Continue. The Rules bar construction of a permanent nature within 50 m of the mean high flood level of a notified wetland. The State Wetland Authority, whose reconstitution is reported as by an order of 17 December 2021 [TBC], has applied that bar, with the Supreme Court's rulings on wetlands of more than 2.25 ha, to halt works at the Karbala Talab in Raipur in September 2025, to order a probe at the Narhaiya Talab and to direct NRDA in March 2025 to withdraw tenders beside the Jhanjh wetland (as reported); no list of notified wetlands was found [TBC]. Kopra Jalashay (210 ha), managed by WRD, was designated a Ramsar site on 8 August 2025. Where a notification or an order is wider than the setback of Chapter 7, it governs (Clause 1.3.3).
Orders of the National Green Tribunal (Central Zone) and the High Court of Chhattisgarh on ponds and rivers	Apply where they bind. The NGT has directed the removal of encroachments from the ponds of Baloda (O.A. 78/2024) and, in January 2026, the completion within six months of Durg's treatment plants on the Shankar and Pulgaon nalas, which carry the city's sewage to the Seonath (as reported). The High Court has heard the pollution of the Arpa at Bilaspur (2025) and of the Kharun, the Seonath and the Arpa (May 2026), and its orders of 22 January 2019 (W.P.(C) 25/2016 and 2696/2016) led the State to stop requiring the permission of the Arpa special-area authority for land in its area (Housing and Environment Department letter of 14 February 2019). No floodplain zoning law or order, and no demarcation of a floodplain by WRD, was found [TBC]. Where the 1% AEP extent of Chapter 7 is mapped and wider, it governs.

Instrument	Relationship
Naya Raipur Development Plan-2031 (NRDA, preface of 17 June 2008)	Continues for Nava Raipur Atal Nagar. It takes "5 years Maximum rainfall intensity" for the design of the drain system, asks that any river embankment be safe against a 100-year flood and that roads be kept 0.5–1.0 m above the observed high flood level, applies IRC:SP:50 to road drainage, preserves the four major streams of the area and makes rainwater harvesting mandatory for every development (§10.3 and Chapters 7–9). It is a city plan, not a State standard. Where this Manual is stricter, as in the return periods of Table 2.1, this Manual applies under Clause 1.3.3; where the plan is stricter, as in its road levels above the high flood level, the plan applies.
Mining and power: the environmental clearances of coal mines and power plants; Coal Mines Regulations, 2017; the Directorate General of Mines Safety (DGMS)	Continue; they govern the leasehold and the plant, not the town. Coal-mine clearances require garland drains, settling ponds and no alteration of major water channels, and the Coal Mines Regulations are reported to govern the danger of surface water entering a mine (reg. 149, as reported; Gazette text not read [TBC]). The ash dykes of the power plants at Korba have breached several times in 2024–2026, the slurry reaching paddy land, a nala and the Hasdeo (as reported). No mine void, pit lake or ash pond is an outfall or a storage of this Manual without the consent of its operator and, for a mine, DGMS (Chapter 5).
Disaster Management Act, 2005; CGSDMA; the district flood control rooms	Continue. CGSDMA is reported to have been constituted in 2007; no State disaster management plan, urban flood plan or SOP of it was found [TBC]. Chapter 10 links its pre-monsoon checks to the dates of the monsoon in the State.
CPHEEO Manual on Storm Water Drainage Systems (2019)	Engineering reference. This Manual sets higher return periods under Clause 1.3.2.
NDMA Guidelines on Management of Urban Flooding (2010) and MoHUA SOP (2017)	This Manual puts into effect the NDMA directives on catchment-based design, detention and retention, waterbody protection and climate-adjusted IDF.
NDMA Urban Flood Risk Management Programme (UFRMP) and AMRUT 2.0	Works funded under these programmes shall comply with this Manual. The development controls of Chapters 4–5 protect the capacity those works create.
IRC:SP:42-2014 and IRC:SP:50-2013	Apply to road drainage only where they do not conflict with Chapter 6.
MBBL 2016, Chapter 9	Model for RWH and for the inspection regime in Chapter 10.

Commentary. RWH in Chhattisgarh is framed as a water supply and recharge measure. It works well for that purpose. It cannot, however, be relied on for flood control in the SW monsoon, because RWH sumps are usually full or the ground saturated by the time a July to September monsoon depression from the Bay of Bengal arrives after weeks of rain. The Manual therefore keeps harvest volume and attenuation volume apart, while letting recharge count as retention where it truly removes water from the system.

Chhattisgarh is beginning to spend on its city drains and nalas. RMC's plan under AMRUT proposed ₹1,191 crore of storm-drain works in 2015 to raise drain coverage from 49% (RMC); Raipur's Smart City company is reported to have proposed about ₹150 crore for "smart drainage" in 2019; and Raipur is reported to have been allotted about ₹222 crore under Phase II of the NDMA's Urban Flood Risk Management Programme in October 2025, for storm-drain improvement, rainwater harvesting and waterbody works and early warning (as reported). Much of the money for the nalas has gone to sewage: the treatment plants of Raipur at Chandanidih, Kara and Nimora intercept the city's nalas with diversion weirs, and in January 2021 Bilaspur was the only town of the State with a sewerage system (CECB report to the NGT). None of the storm-drainage works found has a published design return period, and AMRUT in the State funded none (CAG). No development-control condition stops new plots in the same catchments from using up the capacity such works create. Linking Chapters 4–5 to approvals closes that gap.

The State's law already holds many of the tools. It treats the unsuitability of a site as a question for every development permission (Rule 50 of the CGBVN 1984), asks for WRD's high flood point for land next to a nala or river (the Directorate's checklist of 2011), records the distance of a major nala at the site inspection (2018), vests

village tanks in the State and evicts encroachers on recorded ponds (Land Revenue Code, ss.251 and 248), bars the regularisation of construction on a reservoir bed or a natural drain (Act of 2002, s.7(1)(चार)), and, through the State Wetland Authority, keeps works 50 m back from the highest flood level of a wetland. What it lacks is a design standard. None of these instruments says how large a drain must be, which storm it must carry, how the highest water level of a nala or a talab is to be fixed, or how much storage a filled talab or a paved plot removes; the distances of Rule 50 are, as far as could be found, the inherited 9 m and 15 m, whose Chhattisgarh text has not been confirmed [TBC]. This Manual's checks are framed as part of the development permission of the Directorate of Town and Country Planning, the special-area permission of NRDA and the building and colony permissions of the local bodies, not as a separate NOC. This Manual is offered as a technical companion to these instruments, to the orders of the NGT and the High Court on the ponds and rivers and to the drainage works of the Municipal Corporations: it supplies the design storm, detention and buffer numbers they do not set, as a proposal for the Government and the authorities to consider. Neither the Urban Administration and Development Department, the Housing and Environment Department nor any other body has adopted it.

1.9 Roles and responsibilities

1.9.1 The responsibilities below apply under this Manual. They do not change any agency's statutory powers.

Table 1.4 — Roles and responsibilities under this Manual

Agency	Role under this Manual
Urban Administration and Development Department (UADD)	Oversees the urban local bodies and is the State's nodal department for AMRUT. Would issue the model ULB bye-law of Clause 1.3.1(b), apply this Manual to the drainage works of the Municipal Corporations and other local bodies that it funds, and, with T&CP, give regional support to Tier B authorities (Table 1.1) and maintain the empanelled reviewer list (Clause 10.7).
Housing and Environment Department	Makes the amendments to the CGBVN 1984 under section 85 of the 1973 Adhiniyam, approves development plans and oversees the Directorate of Town and Country Planning and the Special Area Development Authorities. Would prepare the amendment of Clause 1.3.1(a) for the State Government.
State Stormwater Technical Committee (Clause 1.3.2)	Would adopt higher standards, issue Annexes, tables and the compliance calculator, concur with Tier A alternatives, decide variances referred to it and review the Manual (Clause 1.12).
Directorate of Town and Country Planning (T&CP)	Prepares development plans under the 1973 Adhiniyam and grants development permission in planning areas through its regional offices at Raipur, Bilaspur, Durg, Korba, Jagdalpur, Rajnandgaon, Dhamtari, Raigarh and Ambikapur; its site reports ask whether a site is unsuitable under Rule 50 and its checklist asks for WRD's high flood point for land next to a nala or river; it concurs in the works of State departments and local authorities (ss.27 and 28). Would show flood-sensitive catchments, drain reserves and regional facility sites in development plans, and require the drainage report of Chapter 10 with each layout.
Raipur Municipal Corporation (RMC) and the other Municipal Corporations (Bilaspur, Durg, Bhilai, Bhilai-Charoda, Risali, Birgaon, Rajnandgaon, Korba, Raigarh, Jagdalpur, Ambikapur, Dhamtari and Chirmiri); Nagar Palika Parishads and Nagar Panchayats	Hold the public drains, nalas and tanks of their areas under the Municipal Corporation Act 1956 and the Municipalities Act 1961 as adapted (Chhattisgarh texts not read [TBC]), clean and maintain them, and grant building permission under the CGBVN 1984; RMC prepared the city's storm-drain plan under AMRUT (2015). Would apply this Manual to their works, receive handed-over assets (Clause 10.14), and inspect and enforce (Clauses 10.12–10.15).
Raipur Development Authority (RDA) and the other development authorities	Develop schemes and colonies in their scheme areas; RDA's constitution and its drainage functions were not verified for this Manual [TBC]. Would apply this Manual to their schemes and hand over drains and storage to the local body under Clause 10.14.

Agency	Role under this Manual
NRDA (Nava Raipur Atal Nagar Vikas Pradhikaran), the Special Area Development Authority of Nava Raipur Atal Nagar	Exercises the powers of a Special Area Development Authority under the 1973 Adhiniyam (notification No. 4066 of 23 May 2022); planned the new capital's roads, drains, streams and network of lakes and reservoirs under the Naya Raipur Development Plan-2031, which sets a 5-year design intensity for its drains. Would apply this Manual to its works and permissions, treat its lakes, among them Jhanjh, as protected waterbodies under Chapter 7, and hand over drains to the body that maintains them under Clause 10.14.
Arpa special-area authority, Bilaspur	Constituted for the Arpa riverfront, with a development plan approved in 2014; since 2019 its permission is no longer needed for the sale, construction or development of land in its area (Housing and Environment Department letter of 14 February 2019), and its present role is not known [TBC]. No duty is assigned to it under this Manual.
Chhattisgarh Housing Board	Builds housing schemes and colonies; its drainage practice was not researched for this Manual [TBC]. Would apply this Manual to its schemes and hand over drains and storage under Clause 10.14.
Bhilai Steel Plant (SAIL), Town Services	Maintains the drains of the steel plant's township at Bhilai, outside the regime of the Bhilai Municipal Corporation, which warned it over uncleaned township drains in July 2025 (as reported); the formal division of drains between the plant and the Corporations was not found [TBC]. Would apply this Manual to the township's drains and outfalls.
WRD	Holds the rivers, dams, reservoirs, barrages and canals of the State and their gates, among them Gangrel, Minimata Bango, the Hasdeo barrage at Darri, Kharang, the Arpa Bhaisajhar and Mongra barrages and the Kelo dam, and manages Kopra Jalashay; issues the daily tank gauge reports and gives the high flood point for land next to a river or nala. Also the State's groundwater agency for this Manual, with the State Ground Water Management and Regulatory Authority under the Ground Water Act of 2022 when that Act is brought into force [TBC]. Would provide design river and reservoir levels, maximum water levels and gate-operation rules for Chapter 5, obtain the Hirakud levels from Odisha, consent to outfalls into its works, and supply groundwater levels for Chapters 4 and 8 with CGWB.
Public Health Engineering Department (PHED)	Water supply outside the larger cities; its role in urban drainage was not researched [TBC]. Would apply Chapter 8 to the protection of supply sources from storm runoff.
Chhattisgarh Environment Conservation Board (CECB)	Monitors the rivers and drains and reports to the NGT on sewage and its treatment, including the interception of Raipur's nalas for its new treatment plants (2021). Supports the removal of sewage from storm drains (Chapter 8) and the control of runoff from industry, mines and ash ponds (Chapter 8).
Chhattisgarh State Wetland Authority	Constituted under the Wetlands Rules 2017, with the Forest and Climate Change Department as its nodal department (as reported); applies the 50 m line from the highest flood level to Raipur's talabs and the Jhanjh wetland (as reported). Would keep the list of wetlands and Ramsar sites and permit regulated activities in their zones (Chapter 7).
CGSDMA and the District Disaster Management Authorities	Approve the State and district disaster management plans; no urban flood plan of CGSDMA was found [TBC]. Would recommend flood-sensitive catchments, hold the record of flood-prone locations with the Collectors, and lead the post-event review (Clause 10.16).
Revenue and Disaster Management Department: the Collectors and the tehsil offices	Keep the land records in which talabs, nalas and their nistar rights are recorded; administer the village tanks vested in the State (Land Revenue Code, s.251) and evict encroachers on recorded ponds (s.248); lead flood relief. Would certify the recorded extent of a talab for Chapter 7.
Coal and power companies (SECL and the other mining lessees; NTPC, CSPGCL and BALCO) and the Directorate General of Mines Safety (DGMS), a body of the Union	The lessees build and maintain the garland drains, settling ponds and toe drains that their clearances require, and hold the mine voids, pit lakes and overburden dumps; the power companies hold the ash ponds and ash dykes of their plants; DGMS regulates the danger of inundation of mines. Their consent is needed before any mine void or ash pond is used as an outfall or as storage (Chapters 5 and 7), and the breach of an ash dyke is mapped as a hazard (Chapter 5).

Agency	Role under this Manual
Central Water Commission (CWC), a body of the Union	Publishes the Flood Estimation Reports for subzones 3(d), 3(g), 3(f), 1(d) and 3(c) used in Chapters 2 and 3, and the river levels at its sites, such as the Indravati at Jagdalpur, the only site in the State with published warning and danger levels found for this Manual. Would supply river stage records and forecasts for Chapter 5.
Central Ground Water Board (CGWB), a body of the Union	Assesses the groundwater units of the State and their categories (five critical blocks in 2024). Would supply groundwater levels and quality with WRD.
National Institute of Hydrology (NIH), Roorkee	Runs NIH-DHAARA (Chapter 2).
IMD (Meteorological Centre Raipur), and road and rail owners (the Public Works Department, NHAI, Indian Railways)	IMD's stations supply the rainfall records of Chapter 2. Road and rail owners would apply this Manual to road drainage (Chapter 6), culverts and linear works (Clause 1.5.7).

Commentary. Stormwater in Chhattisgarh is split between the Housing and Environment Department and T&CP, through the CGBVN 1984, development plans and development permission; UADD and the Municipal Corporations and other local bodies, through building permission and the drains vested in them; NRDA in Nava Raipur Atal Nagar, and RDA and the Chhattisgarh Housing Board in their schemes; the Bhilai Steel Plant in its township; WRD, through the rivers, dams, barrages and their gates; the Revenue and Disaster Management Department, through the village tanks and land records; the State Wetland Authority, with the NGT and the High Court setting lines around ponds and rivers; and, in the coalfields, the mining lessees, the power companies and DGMS. Raipur shows why the split matters. The city's nalas carry both storm water and sewage to the Kharun, now through the diversion weirs of new treatment plants; its talabs vest variously in the Corporation and, where they are village tanks, in the State; and the river below them is WRD's. The Municipal Corporation Act is understood to vest public drains, streams and tanks in the Corporation, as its Madhya Pradesh text does (s.82; Chhattisgarh text not read [TBC]), but no rule or order found divides the natural nalas from the municipal drains, or the township drains of Bhilai from those of its Corporations, between the bodies concerned [TBC]. Until such a rule is made, this Manual assigns the drainage elements as in Table 1.5, as Chapter 6 does for the drains themselves. The assignment is this Manual's working split for applying its clauses; it does not change any agency's powers.

Table 1.5 — Working assignment of drainage elements under this Manual

Element	Assigned to
Rivers and their beds, banks and floodplains; dams, reservoirs and barrages with their gates and spillways (such as Gangrel, Minimata Bango, the Hasdeo barrage at Darri, Kharang, Maniyari, the Kelo dam, Kopra, and the Mongra, Arpa Bhaisajhar and Mahanadi barrages); flood embankments and irrigation canals	WRD
Natural nalas, public drains and tanks within the limits of a local body, other than those assigned to WRD	The Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat (in Raipur, RMC)
Roads, drains, streams, lakes and reservoirs of Nava Raipur Atal Nagar	NRDA
Drains and storage within a scheme of RDA, the Chhattisgarh Housing Board or a colony developer, until handed over in writing	The body that built the scheme
Drains of the township of the Bhilai Steel Plant	The Bhilai Steel Plant (SAIL); drains of Bhilai outside the township to the Bhilai and Risali Municipal Corporations [TBC]
Village tanks with rights of irrigation or nistar, vested in the State	The Revenue and Disaster Management Department (Collector), with those rights preserved

Element	Assigned to
Sewers, interceptors and the interception and diversion of nalas, and sewage treatment plants	The Municipal Corporation or the body executing the work for it
Garland drains, settling ponds, toe drains, mine voids and pit lakes within a mining lease; ash ponds, ash dykes and plant drains of a power plant	The lessee, under its environmental clearance and DGMS; the operator of the plant
Drains of State and national highways	The Public Works Department or NHAI
Wetlands and Ramsar sites	The Chhattisgarh State Wetland Authority, with the owner of the land (Kopra Jalashay: WRD)

This Manual names departments and authorities as the statutes, orders and records name them. A manual that names one owner for each task does not fix institutions by itself, but it removes ambiguity about who signs what.

1.10 Variances and waivers

1.10.1 **Variance.** The Authority may grant a variance from a numeric requirement where the Applicant shows, through a report certified under Clause 10.6, that:

- (a) compliance is impracticable because of site conditions such as a high groundwater table, contaminated soil, rock, or a site area too small for the required facility; and
- (b) the proposed alternative gives equal or better performance at the downstream point of Chapter 5, or the shortfall is made up by fee-in-lieu or off-site works in the same catchment.

1.10.2 **Waiver.** The Authority may waive Chapter 4 detention requirements (not retention) where a hydraulic analysis certified under Clause 10.6 shows that the site discharges directly to the sea, or to a tidal reach with no flood-prone property downstream, and that undetained flow does not worsen the outfall-closed condition of Chapter 5. Chhattisgarh has no coast and no tidal reach, so no site in the State qualifies for this waiver. A site that drains to a river raised by dam or barrage releases, as the Mahanadi below Gangrel and the Hasdeo below Minimata Bango and Darri, to the backwater of the Hirakud reservoir, to a reservoir, barrage pond or talab, or to a mine void, is in the outfall-closed condition of Chapter 5 whenever the receiving water is high, and detention is not waived for it.

1.10.3 **No variance or waiver.** No variance or waiver shall be granted for:

- (a) a sewage connection to a storm drain;
- (b) the lowest habitable floor level of Chapter 6;
- (c) any reduction of a waterbody setback below the Zone 1 minimum of Clause 7.4.7, or any fill or construction in the no-fill zone of Chapter 7; or
- (d) the retention requirement in a flood-sensitive catchment, except by fee-in-lieu under Clause 1.7.

1.10.4 **Procedure.**

- (a) The request shall be made on **Form 1-2** (Variance / Waiver Request) with the preliminary drainage report.
- (b) A Tier B authority shall refer any variance on a site of 1 ha or more to the State Stormwater Technical Committee.
- (c) A decision shall state its reasons and any conditions, and shall be entered in the public register of Clause 10.17.
- (d) A variance runs with the approval. It lapses if the approval lapses.

> **Commentary.** The 2006 APWA 5600 included a waivers section (5601.6), and Georgia allows waivers where downstream analysis shows a peak control is unnecessary. The waiver of Clause 1.10.2 is narrow: on a tidal coast, undetained flow can help by passing through before the tide closes the outfall, but only an analysis can show that. This Manual requires every decision to be published because publication deters case-by-case erosion of the standard; in Chhattisgarh, Raipur is reported to keep only about 37–40 of the 130 or so ponds it once had, residents reported after the waterlogging of July 2026 that drains had been narrowed by encroachment from seven or eight feet to four or five, and the State Wetland Authority has had to halt works within 50 m of the highest flood level of the Karbala Talab in Raipur and direct NRDA to withdraw tenders beside the Jhanjh wetland in Nava Raipur Atal Nagar (all as reported).

1.11 Transition provisions

1.11.1 **Effective date.** This Manual takes effect for new applications **6 months** after notification (the effective date).

1.11.2 **Applications in process.** An application filed complete before the effective date may be decided under the rules in force when it was filed, if a decision is issued within 12 months of the effective date. Otherwise, this Manual applies.

1.11.3 **Phased start for Tier B.** In Tier B authorities, Chapters 4 and 5 apply from **24 months** after notification, except in flood-sensitive catchments, where they apply from the effective date.

1.11.4 **Public works.** DPRs sanctioned before the effective date may proceed as sanctioned. Any DPR revised or re-tendered after the effective date shall comply. Departments shall report non-compliant works in progress to the State Stormwater Technical Committee within 12 months.

1.11.5 **Before the annexes are issued.** Until an annex referred to in this Manual is issued, the Applicant shall use the method stated in the clause, with values agreed with the Authority and recorded in the drainage report.

> **Commentary.** The 6-month lead time matches the lead time for NIH-DHAARA updates in Clause 2.14.2. Singapore gave a similar interval for its 2025 detention addendum (issued 16 Apr 2025, applied from 1 Oct 2025). The 12- and 24-month periods are set by this Manual to let Tier B authorities train staff and to let the State issue the design-aid tables (Clause 1.7).

1.12 Review and amendment

1.12.1 The State Stormwater Technical Committee shall review this Manual every **10 years**, and after any event that exceeds the 1% AEP design depth as set out in Clause 2.14.1.

1.12.2 Amendments shall be notified in the Gazette, published with a change log, and take effect 6 months after notification unless the notification states otherwise.

1.12.3 Any person may propose an amendment to the Committee. Proposals and the Committee's decisions shall be published.

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Annexes and forms

Annex / Form	Content
1A	List of Tier A authorities, with the boundaries of the Municipal Corporations, of the planning area of Raipur and of the special area of Nava Raipur Atal Nagar
1B	Maps of flood-sensitive catchments

Annex / Form	Content
1C	Policy Supplement: text of alternatives S-1 to S-5 and model council resolution
1D	Model CGBVN 1984 amendment and model ULB bye-law
Form 1-1	Applicability Determination
Form 1-2	Variance / Waiver Request

CHAPTER 2

Design Rainfall and Design Storms

2.1 Purpose and scope

2.1.1 This chapter sets the rainfall inputs for every hydrologic calculation in this Manual: drain and inlet sizing (Chapter 6), retention and detention sizing (Chapter 4), downstream and tailwater analysis (Chapter 5), and waterbody and tank works (Chapter 7).

2.1.2 It specifies:

- (a) the approved sources of design rainfall depth and intensity;
- (b) how depths are converted between durations, between point and area, and from present to future climate;
- (c) how design storms (hyetographs) are constructed; and
- (d) the storms that must be applied to pre- and post-development conditions.

2.1.3 This chapter applies throughout Chhattisgarh. Where a Municipal Corporation, Nagar Palika Parishad, Nagar Panchayat, NRDA or a development authority has adopted a stricter rainfall standard under Chapter 1, the stricter standard governs.

> **Commentary.** Most past drainage failures in Chhattisgarh trace to the rainfall input, not the hydraulics. Chhattisgarh shows how. The only design storm found for a town's drains in the State is the "5 years Maximum rainfall intensity" of the Naya Raipur Development Plan-2031, read from a chart "Based on IMD Data" that names no station, period of record or method (NRDA, 2008). Raipur's storm-drain plan under AMRUT records that the drains covered 49% of the city and overflowed in the rains, and proposes ₹1,191 crore of works without a return period, an intensity or a runoff coefficient (RMC, 2015); no storm-drain DPR of any other town was found for this Manual. The rainfall figures that circulate after a flood are often unsourced or conflicting: the Bilaspur rain of July 2026 is reported as 28 cm and as 415.6 mm in 24 hours, the 24-hour fall in Janjgir-Champa in August 2020 as 211.9 mm and as 215.63 mm, and the Indravati at Jagdalpur in September 2026 as 12.56 m on a local gauge and as 544.891 m at the CWC site (as reported). The published sources that seem to fill the gap do not. IMD Das et al. (2022) has no station in the State; constants for Raipur attributed to Ram Babu et al. (1979) circulate without their source; and NIH-DHAARA's reanalysis-based depths are too high at Durg, Korba and Jagdalpur, too low at Ambikapur and Jashpur Nagar, below the station records at Raipur and Rajnandgaon, and low at 1 hour everywhere (Clause 2.3.6). The CWC Flood Estimation Reports are the only regional sub-daily source, and the five subzones that cover the State give 1-hour ratios of 0.32–0.39 and loss rates of about 1.5–5 mm/h. Fixing one transparent source and method for the whole state is the main purpose of this chapter.

2.2 Definitions

Term	Meaning in this Manual
Annual exceedance probability (AEP)	Probability that a given rainfall depth is equalled or exceeded in any one year. A 1% AEP storm is the "100-year" storm. Both forms are used here: T-year = 1/AEP.
IDF / DDF	Intensity–duration–frequency and depth–duration–frequency relationships for a location.
Point rainfall	Rainfall at a single location, as measured by a rain gauge.
Design storm	A hyetograph (rainfall depth versus time) of stated AEP, duration and temporal pattern.
Critical duration	The storm duration that produces the largest peak flow, water level or storage requirement for the element being designed.
Areal reduction factor (ARF)	Ratio of catchment-average rainfall to point rainfall for the same AEP and duration.
SW monsoon season (JJAS)	June–September, with about 90% of the State's annual rainfall falling in these months and nearly 58% in July and August (IMD, normals of 1961–2010). The State's normal is 1,268.1 mm, from 1,011.2 mm in Kabirdham and 1,061.5 mm in Durg, through 1,129.3 mm in Raipur and 1,110.5 mm in Bilaspur, to 1,575.7 mm in Bastar and 1,839.3 mm in Dantewada (IMD). The monsoon is brought mainly by low-pressure areas and depressions from the Bay of Bengal that cross the State westwards or north-westwards. The normal date of onset is 11–13 June at Jagdalpur, 14 June at Kondagaon, 15 June at Ambagarh Chowki, 16 June at Raipur and Pendra Road and 17 June at Bilaspur and Balrampur, and of withdrawal 7 October at Pendra Road, 8 October at Bilaspur, Ambagarh Chowki and Balrampur, 9 October at Raipur, 11 October at Kondagaon and 12–14 October at Jagdalpur (IMD, new normals of 2020); the Meteorological Centre Raipur uses 10 June and 15 October for the State. The antecedent moisture rule of Clause 2.11.4 refers to this season; the pre-monsoon deadline of 31 May is set ahead of the earliest onset (Chapter 10).
Storms outside the season	Large falls come early in the season, before the monsoon is established, and after it. Several stations hold their highest day in June: 365.0 mm at Jashpurnagar on 30 June 2001, 337.0 mm at Durg on 15 June 2004, 297.0 mm at Raipur on 30 June 2007, 259.7 mm at Jagdalpur on 17 June 1974 and 292.4 mm at Bailadila on 18 June 1999 (IMD). Before the monsoon, Bailadila recorded 207.0 mm on 11 May 1990; after it, Pendra Road recorded 228.1 mm on 8 October 1994, Ambikapur 206.0 mm on 1 October 1961, its highest day, Raipur 148.6 mm on 8 October 1900 and Jagdalpur 139.1 mm on 8 October 2003 (IMD). Of the 100 storms and depressions that affected the State in 1891–2014, 21 came in October, more than in September (IMD), and cyclones from the Bay crossing the Andhra coast occasionally bring heavy rain in October (Meteorological Centre Raipur). Design statistics use annual maxima from all months, and no works deadline or operating rule shall assume that heavy rain is confined to June–September.
Climate factor (CF)	Multiplier applied to present-day design depths to represent the design-life climate.
Approved source	A rainfall source listed in Clause 2.3.

2.3 Approved sources of design rainfall

2.3.1 **Primary source.** Design rainfall depths and intensities shall be taken from the National Institute of Hydrology, Roorkee **NIH-DHAARA** platform (Design Hydrology and Advanced Rainfall Analysis, nihdhaara.in). Use its IDF / frequency tools for the site's coordinates, for each duration and AEP required by Clause 2.4. In Chhattisgarh this clause is suspended until Annex 2A is published: Clause 2.3.6 sets the interim source.

2.3.2 **Station IDF.** A station-specific IDF relationship may be used in place of 2.3.1 only if all of the following hold:

- (a) it is derived from at least 30 years of recording-gauge (hourly or finer) data;
- (b) it has been published or approved by IMD, the State Hydrology Authority or the approving authority; and

- (c) the gauge is within 10 km of the site, with no major change of terrain or coastal exposure in between.

Where a station IDF is used, the design value shall be the **greater** of the station value and the NIH-DHAARA value.

2.3.3 **Cross-checks (mandatory, not alternatives).** The designer shall report, but need not adopt, the following values for comparison:

- (a) the IMD isopluvial / IDF values of Das et al. (2022), *MAUSAM* 73(4), for durations of 1–24 h;
- (b) for catchments of 25–5,000 km², the 24-hour point rainfall from the CWC Flood Estimation Report for subzone 3(d) (Mahanadi), 3(g) (Indravati), 3(f) (Lower Godavari), 1(d) (Sone) or 3(c) (Upper Narmada and Tapi), as applicable.

Where the design value is lower than the CWC 3(d), 3(g), 3(f), 1(d) or 3(c) value for the same AEP, the CWC value shall be adopted as a floor.

2.3.4 **Not permitted.** None of the following shall be used as a design rainfall source:

- (a) generalised all-India IDF formulae (for example Kothyari & Garde, 1992);
- (b) rainfall values that cite no source, including vendor or web-calculator tables;
- (c) single-event maxima; or
- (d) design intensities carried forward from a previous project without a stated AEP.

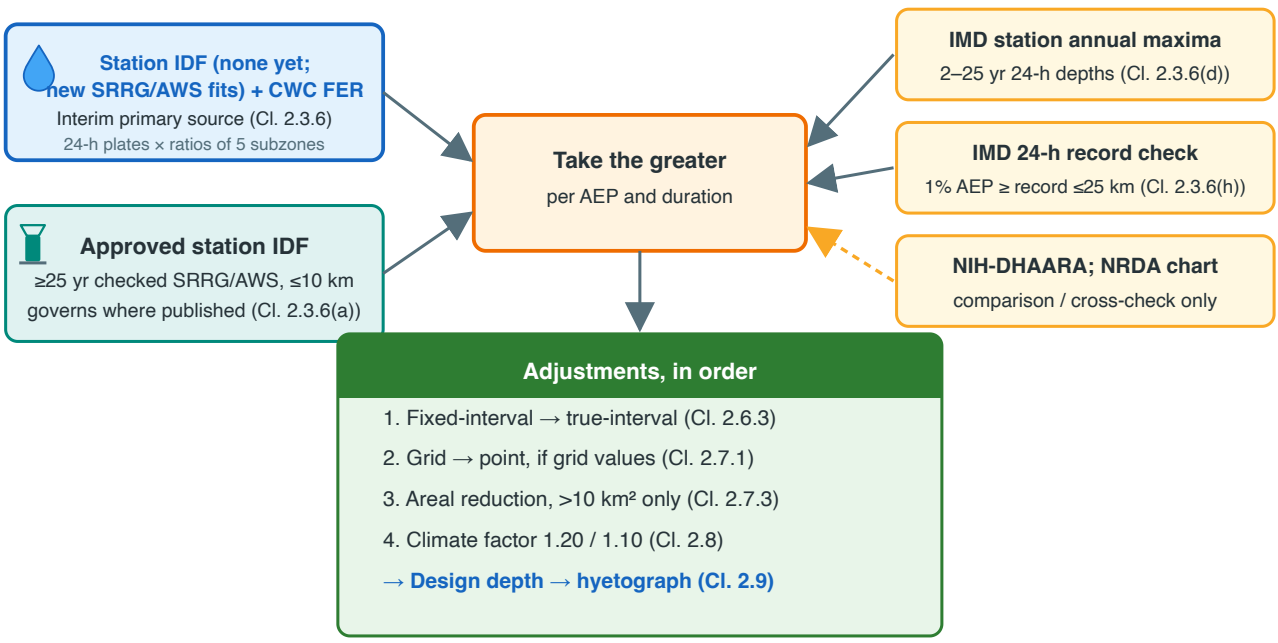

Figure 2.1 — Design rainfall source hierarchy and adjustment sequence. Original drawing for this Manual.

2.3.5 **Version control.** Every submittal shall record the source, its version or access date, the coordinates queried, and the values extracted (Clause 2.13).

2.3.6 **Interim source in Chhattisgarh.** Until Annex 2A is published and confirms NIH-DHAARA, or an approved station IDF, for a zone, Clause 2.3.1 does not apply in Chhattisgarh, and design rainfall shall be taken as follows:

- (a) **Station IDF.** Where a station IDF fitted to a checked IMD recording-gauge (SRRG) or automatic weather station record of at least 25 years has been published in Annex 2B, it governs, within 10 km of the station, for the durations and AEPs it covers, in place of (f) and (g); the comparison of Clause 2.3.2 is then made with the 24-hour and longer depths of (c) to (e) and (h), and the CWC floor of Clause 2.3.3 still applies. A record is checked when its annual totals and annual maxima have been compared with the IMD normals and station extremes for

the station, and every year that disagrees has been explained or removed. An IDF derived from daily data by the one-third rule (Clause 2.6.4) or by any other fixed ratio is not a station IDF. In Chhattisgarh Clause 2.3.2(a) is read with 25 years in place of 30 (set by this Manual, so that the recording gauges of the State's main towns can qualify once analysed). No IDF has yet been published in Annex 2B, and no station IDF with verified constants was found for any town in the State: constants for Raipur attributed to Ram Babu et al. (1979) were seen only in a search extract, without their column headings, and are not used until the bulletin has been read [TBC]. The first data request of this Manual is for the recording-gauge and automatic weather station records of the IMD stations of the main towns, to be fitted under this paragraph: Raipur (Obsy), 42875 (daily record 1901–2012), and Raipur (Mana) (A), 11203 (1979–2012), for Raipur and Nava Raipur Atal Nagar; Pendra Road, 42779 (1903–2012); Jagdalpur, 43041 (1910–2012); Kanker, 42953 (1927–2008); Champa, 42783 (1945–2012); Raigarh, 42884 (1950–2012); Ambikapur, 42693 (1951–2012); Jashpurnagar, 42790 (1965–2012); and Durg, 42876 (1969–2012) (IMD); the years of recording-gauge data held at each are to be confirmed [TBC].

- (b) **Subzone.** The site's CWC subzone is that of the Flood Estimation Report whose area contains it, by the boundaries drawn on the reports' plates, which in Chhattisgarh follow the basin divides. Five subzones cover the State. 3(d) (Mahanadi, M-3(d)/R-3/25/1996) covers the Mahanadi basin, 56% of the State, and the Brahmani basin, about 1%, including **Raipur**, **Nava Raipur Atal Nagar**, Birgaon, **Durg**, **Bhilai**, Risali, Bhilai-Charoda, **Rajnandgaon** (except its Wainganga side), Bemetara, Balod, Baloda Bazar, **Bilaspur**, Mungeli, Kawardha (except its Narmada fringe), **Korba**, Janjgir, Champa, Sakti, **Raigarh**, Sarangarh, **Dhamtari**, Mahasamund, Rajim, Gariaband, Kanker (except the Kotri side), Manendragarh, Chirmiri, Baikunthpur and Jashpur Nagar. 3(g) (Indravati, I/21/1992) covers the Indravati basin, including **Jagdalpur**, Kondagaon, Narayanpur, Dantewada and Bijapur on the Indravati side, and Pakhanjur and the west of Antagarh on the Kotri. 3(f) (Lower Godavari, LG-3(f)/R-2/24/1995) covers the Sabari and the Godavari frontage, including Sukma, Dornapal and Konta, and the Wainganga side of Rajnandgaon, Khairagarh and Mohla-Manpur. 1(d) (Sone, S/15/1987) covers the Rihand, Kanhar and Gopad basins, including **Ambikapur**, Bishrampur, Surajpur, Balrampur, Ramanujganj, Wadrafnagar and the north of the Koriya area (Bharatpur, Janakpur). 3(c) (Upper Narmada and Tapi, UNT-3(c)/R-6/46/2002) covers the Narmada fringe of Kawardha and Rajnandgaon, about 744 km², with no town. There is no subzone 3(e) in the State. Within 5 km of a subzone boundary, the higher of the two plate values is used, and at each duration the higher of the two products under (f) (set by this Manual). This applies in particular to **Jagdalpur**, which the 3(g) plates show on the line between 3(g) and 3(f): the higher of the 3(g) and 3(f) values is used throughout the Municipal Corporation area, although the city drains to the Indravati (set by this Manual); to **Ambikapur**, which lies just inside 1(d) on the divide with the Mand and the Hasdeo; to Kanker, Rajnandgaon and Khairagarh, near the lines of 3(d) with 3(g) and 3(f); and to the 3(c) fringe, where, until the 3(c) plates have been read for the State, the 3(d) plate value is used with the higher of the 3(c) and 3(d) ratios [TBC]. The district and town lists are a guide: the plate boundaries govern, and are to be confirmed on the reports' index maps [TBC]. The rainfall zone and sub-area of the site are those of Annex 2A.
- (c) **24-hour depth, 2% and 1% AEP (50- and 100-yr).** The 24-hour point value at the site read from the 50- and 100-year isopluvial plates of the report for its subzone (Plates 9 and 10 of 3(d); Plates 10 and 11 of 3(g); Plates 9 and 10 of 1(d), by their titles, although the text of that report calls Plate 10 the 50-year map; and the plates of 3(f) and 3(c), which were not read for the State [TBC]), whatever the catchment area. Where the site lies between two contours, the higher contour value is used; where it lies inside a closed contour with no higher contour within it, the value is that contour plus one contour interval; and where it lies inside a closed low cell marked "<", the value is that of the enclosing contour (all set by this Manual). Plates are read by the coordinates of the site, not by the town labels. Until the 3(f) plates have been read, a site in 3(f) takes the value of the 3(g) plates where they extend over it, and otherwise the highest contour of the 3(g) plates within 25 km of it [TBC]. For orientation only, read by eye to about one contour on scans re-rendered for this Manual, the 50-year plates give about 280–320 mm at Raipur and Nava Raipur Atal Nagar, 240–280 mm at Durg and Bhilai, about 280 mm at Rajnandgaon, on the contour, 200–240 mm at Bilaspur, at the edge of a closed cell of less than 200 mm over Mungeli and Kawardha, 240–280 mm at Korba and Champa, about 320 mm at Dhamtari, 320–360 mm at Mahasamund and Rajim, about 360 mm at Gariaband, with a closed cell of more than 440 mm on the Odisha border south-east of it, 280–320 mm at Kanker, 240–280 mm at Jagdalpur (3(g)), 200–240 mm at Sukma, at the edge of the 3(g) plate, and a closed contour of more than 280 mm around Ambikapur and Bishrampur (1(d)). At Raigarh the 50-year contours are unlabelled, and the value, about 320 mm, is inferred from the 100-year plate, on which the 360 mm line

passes through the Kelo bridge just west of the town [TBC]; at Manendragarh and Jashpur Nagar the lines run too close together to read, about 200–280 mm [TBC]. The 100-year values are about one contour (40 mm) higher: about 320–360 mm at Raipur, 280–320 mm at Durg and Korba, 240–280 mm at Bilaspur and Jagdalpur, about 360 mm at Raigarh, and a closed contour of more than 320 mm around Ambikapur. The worked example of the 3(d) report, the Sondur at railway bridge No. 385, a river that rises in Chhattisgarh and is bridged in Odisha, reads 32.0 cm at 50 years. Where the Gumbel estimate of (d) for the station of the site is higher for the same AEP, it governs; the 24-hour depth is then checked under (h).

- (d) **24-hour depth, 50%, 20%, 10% and 4% AEP (2-, 5-, 10- and 25-yr).** The depth is the Gumbel (EV1) estimate from the annual maximum daily rainfall series, of all months, of the IMD station agreed with the Authority, checked as in (a), multiplied by the fixed-interval factor of Clause 2.6.3; in 3(d) and 3(g), the 4% AEP depth is not less than the 25-year plate value (Plate 8 of 3(d); Plate 9 of 3(g)) read as in (c) (set by this Manual). The station is, for example, the higher of Raipur (Obsy), 42875, and Raipur (Mana) (A), 11203, for Raipur and Nava Raipur Atal Nagar, because the two gauges, about 15 km apart, hold different records (370.3 mm and 273.0 mm) and on 30 June 2007 read 297.0 mm and 273.0 mm (IMD); Durg, 42876, for Durg, Bhilai and Risali; Rajnandgaon, 11204 (1981–2008), with Durg as a check, for Rajnandgaon; Champa, 42783, for Champa and Janjgir; Raigarh, 42884, for Raigarh; Pendra Road, 42779, for Pendra and Gaurela; Kanker, 42953, for Kanker; Jagdalpur, 43041, for Jagdalpur; Ambikapur, 42693, for Ambikapur and Bishrampur; and Jashpurnagar, 42790, for Jashpur Nagar. Bilaspur, Korba, Dhamtari, Mahasamund, Kawardha, Dantewada and Sukma have no IMD station in IMD's tables of station extremes; for them, and elsewhere, the station is the nearest IMD station with 30 or more years of record, from the candidates in Annex 2A, or a district rain-gauge series of 30 or more years supplied by IMD [TBC]. A day of record excluded under (h) is left out of the series, and the fit with it is reported beside the fit without it on Form 2-1 (set by this Manual).
- (e) **48- and 72-hour depths.** The Gumbel estimate from the annual maximum 2-day and 3-day rainfall series of the same station, with the fixed-interval factors of Clause 2.6.3, and not less than the 24-hour depth for the same AEP, as checked under (h).
- (f) **Durations of 1 to 24 hours.** The 24-hour depth of (c) or (d), before the check of (h), multiplied by the ratios of the report for the site's subzone in Table 2B.1, at every AEP: in 3(d), the ratios 0.390 at 1 hour, 0.610 at 3 hours, 0.760 at 6 hours, 0.820 at 9 hours, 0.860 at 12 hours, 0.880 at 15 hours and 0.920 at 18 hours, derived from the 10-year depths of the recording gauges inside the subzone; in 3(g), 0.320, 0.500, 0.635, 0.730, 0.800, 0.864 and 0.917; in 3(f), 0.32, 0.52, 0.65, 0.73, 0.79, 0.85 and 0.91; in 1(d), the 50-year ratios 0.37, 0.58, 0.70, 0.80, 0.89, 0.94 and 0.98 (the 18-hour value from an OCR of the report, not checked against the page image [TBC]); and in 3(c), 0.34, 0.50, 0.63, 0.72, 0.79, 0.86 and 0.91, at the same durations. The 1(d) report found its ratios independent of the return period from 5 years upward; the 3(d), 3(g), 3(f) and 3(c) ratios, on the reports' 10-year basis, are applied at every AEP in the same way (set by this Manual). Between the tabulated durations the ratio is interpolated on logarithmic axes. The 24-hour depth itself is that of (c) or (d) as checked under (h).
- (g) **Short storms.** No absolute floor on the 1-hour or 3-hour depth is set in Chhattisgarh. The depths of (f) are used as they stand at every AEP, and are not reduced towards the recorded hourly maxima, towards NIH-DHAARA or towards the NRDA chart. The reason is that the evidence found does not show the subzone ratios understating short storms in the State; it points the other way. Every hourly maximum in the recording-gauge annexes of the reports for the State, 42–80 mm in 1969–1986, is below or near the lower end of the 50-year 1-hour depth that the plates and ratios give near the gauge: Kanker's 79 mm on 26 June 1974 (printed as 75.3 mm for the same day in the 3(g) report) against about 109–125 mm, at 0.39 times its plate value of 280–320 mm (CWC FER 3(d)); Jagdalpur's 80.0 mm against about 77–90 mm there; and Paralkote's 75.0 mm, on the Kotri, where the plates rise above 320 mm, against about 100 mm (CWC FER 3(g)). At 3 hours, Kanker's 115 mm (18 September 1980) is below its value of about 171–195 mm, and Bhopalpatnam's 145.5 mm (8 August 1974) is close to its value of about 140–160 mm, within the reading error of one contour (CWC FER 3(g)). With 10–17 years of record at each gauge, one or two maxima near the 50-year value are to be expected. Two rows add nothing: Raigarh's 42 mm in 1 hour comes from a record of less than five years, and the Champa row of the 3(d) report prints 139 mm at 1 hour and at 3 hours beside 87 mm at 12 hours and a date in February, and is not used [TBC]. The other evidence agrees: NIH-DHAARA's 50-year 1-hour depths in the State are 54–85 mm; the NRDA chart for Nava Raipur gives about 40 mm/h over 3 hours, about 120 mm, for the 50-year storm, against about 171–195 mm from the 3(d) plates and ratios; and the Raipur constants attributed to Ram Babu et al. (1979), unverified, give about 71 mm in

1 hour [TBC]. The depths of (f) are therefore taken to be conservative at 1 to 3 hours, and no floor is added. They are not reduced either, because the hourly records are short and end in 1986, the NRDA chart states no station, period of record or method, the Ram Babu constants are unverified and NIH-DHAARA's values are grid values (Clause 2.7.1). In Nava Raipur Atal Nagar the depths of the NRDA chart for 2, 5 and 10 years (Naya Raipur Development Plan-2031, Fig. 10.1), read by eye to about 10–15%, may be reported on Form 2-1 beside those of (f) as a cross-check; they are not design values and not a floor. This decision is interim and is to be revisited when Annex 2B publishes frequency analyses of the IMD recording-gauge and automatic weather station records of the stations of (a), beginning with Raipur (Obsy), Raipur (Mana), Jagdalpur, Pendra Road and Ambikapur, or when the Ram Babu et al. (1979) bulletin or the station data behind the NRDA chart has been obtained: if any of them shows 1-hour or 3-hour depths above those of (f) for a zone, a floor shall be set for that zone; if together they show depths well below them, a reduction of the ratios may be proposed to the State Stormwater Technical Committee. No IMD hourly maximum for a recent storm in the State was found in research for this Manual [TBC]. The depths of (f) are present-day, true-interval values once the fixed-interval factor has been applied to the 24-hour depth; the climate factor of Clause 2.8 is then applied.

- (h) **24-hour record check.** The 1% AEP (100-yr) 24-hour depth of (c) or (d) shall be not less than the highest qualifying 24-hour rainfall recorded by IMD at any station or district rain gauge within 25 km of the site, where that record is higher; the record is used as printed, without the fixed-interval factor, and is treated as a 1% AEP depth; the 2% AEP (50-yr) 24-hour depth is then not less than the 1% AEP depth so raised less one contour interval of the plates, 40 mm (all set by this Manual). A record qualifies only when it is printed by IMD (in *Extremes of Temperature and Rainfall for Indian Stations* or the *Climate of Chhattisgarh*, or a later IMD table); the length of daily record at its gauge is confirmed by an IMD table as at least 20 years; it falls in a month in which a fall of that size is plausible for the station; and it is not contradicted by another IMD table for the same gauge and day. A record that fails any test is excluded until IMD confirms what it lacks (set by this Manual). The records that qualify and raise the 100-year depth above the plates, read as in (c), are **370.3 mm at Raipur (Obsy), 42875, on 4 August 1910** (daily record from 1901), against about 320–360 mm; **365.0 mm at Jashpurnagar, 42790, on 30 June 2001** (from 1965), against about 240–320 mm; **358.0 mm at Korba on 12 August 2004** (a gauge of 23 years), against about 280–320 mm; and **347.2 mm at Champa, 42783, on 26 July 1947** (from 1945), against about 280–320 mm (IMD). Excluded pending confirmation of the length of record at the gauge [TBC]: **392.0 mm at Rajnandgaon (21 September 1965)** and **337.0 mm at Durg (15 June 2004)**, which come from district gauges, not from the IMD stations of those names in IMD's table of station extremes (Rajnandgaon, 11204, whose record begins in 1981, and Durg, 42876, whose highest day is 260.4 mm); Saraipali's 508.3 mm and Basna's 453.0 mm (29 August 2003); Samaria's 540.0 mm (13 August 2003); and the other district-gauge records listed in Annex 2A. Each is admitted, from the date of IMD's confirmation, once IMD confirms at least 20 years of daily record at its gauge; until then it is reported on Form 2-1 but does not set the design depth. Excluded until IMD confirms the value itself [TBC]: Wadrafnagar's 917.2 mm (8 September 2003), which IMD prints as the State's highest day but which comes from a gauge of about 10 years and is more than twice any other value in the State; Ambikapur's 456.7 mm (24 July 1991), which IMD's table of station extremes contradicts with 174.0 mm on 23 July 1991; and two values in improbable months, Kathgora's 433.6 mm (24 November 1958) and Bharatanagar's 301.0 mm (9 January 1995). Until Annex 2A is published, the designer lists on Form 2-1 every IMD record within 25 km of the site, with the tests it passes or fails. The check raises the 24-hour depth and, through (e), the 48- and 72-hour depths. It does not raise the depths of (f) for 1 to 18 hours, because the records are daily totals of long Bay of Bengal storms, no hourly record of any of them exists, and multiplying them by the ratios of (f) would give about 144 mm in 1 hour at Raipur, almost twice the largest hourly fall recorded in the State. The check is made because several long-record gauges have recorded more than the plates' 100-year depth, most of them since the plates were drawn, so that the plates cannot be relied on in the tail on which floors, storage and spillways depend (commentary).
- (i) **Durations below 1 hour.** No sub-hourly frequency analysis for any station in the State was found in research for this Manual. Until Annex 2B gives one, the ratio of the depth for a duration below 1 hour to the 1-hour design depth is agreed with the Authority, from a published recording-gauge analysis whose source is stated, and recorded on Form 2-1 [TBC]; the average intensity for a duration below 1 hour shall be not less than the 1-hour design intensity for the same AEP (set by this Manual). In Nava Raipur Atal Nagar the 15-, 30- and 45-minute points of the NRDA chart may be reported as a cross-check under (g).

- (j) **Order of depths.** For the same AEP, no depth shall be less than the depth for a shorter duration.
- (k) **NIH-DHAARA.** Its present-day values at the site are reported on Form 2-1 beside the interim values, for comparison only: in the State they are close to the plates at Raipur, Rajnandgaon, Dhamtari, Bilaspur and Raigarh, too high at Durg and Bhilai, Korba, Nava Raipur, Mahasamund and Jagdalpur, too low at Ambikapur, Kanker and Jashpur Nagar, below the station records at Raipur, Rajnandgaon, Korba, Raigarh and Jashpurnagar at 100 years, and low at 1 hour everywhere (commentary). Its ratios of the t-hour to the 24-hour depth are tabulated in Annex 2B for comparison, and are not used as design ratios. Its projected depths are not used under Clause 2.8.4; the ratio of its projected to its present-day depths may be reported as a check on the climate factors of Table 2.2, which apply.
- (l) **IMD Das et al. (2022).** The paper has no panel for any station in Chhattisgarh; its nearest panels, Jabalpur and Bhubaneswar, are outside the State and in other subzones. The cross-check of Clause 2.3.3(a) is recorded on Form 2-1 as not available, and the paper is not used as a value, a check or a floor.
- (m) **Earlier design values.** The figures found for the State are not a design source. The Naya Raipur Development Plan-2031 (NRDA, 2008) states that "5 years Maximum rainfall intensity should be taken for design of system", asks that any river embankment be "safe against 100 yrs frequency flood" and that roads be kept "at least 0.5 to 1.0 m above the observed high flood level", applies IRC:SP:50 to road drainage, and prints a chart of rainfall intensity against time of concentration for 2, 5, 10 and 50 years "Based on IMD Data", with no station, period of record or fitting method; it is the only design storm found for a town's drains in the State, and it is a city plan, not a State standard. RMC's storm-water plan under AMRUT (2015) records network coverage of 49%, about 480 km of pucca drains and 11 road stretches that flood two or three times a year, and proposes ₹1,191 crore of works, without stating a return period, rainfall intensity or runoff coefficient. The return periods of Table 2.1 and the depths of this clause apply instead, in Nava Raipur Atal Nagar as elsewhere: NRDA's 5-year criterion is cited as the history of practice, and its chart is used only as in (g) and (i).
- (n) **Other clauses.** Where Clauses 2.3.2, 2.6.1, 2.7.1 and 2.13, Annexes 2A to 2C and Form 2-1 refer to the NIH-DHAARA value, the value under this clause is meant.

Commentary. NIH-DHAARA is a national, publicly accessible product from a Government of India institution, and it can be queried at any point in the State. That makes it a reproducible default that an approving authority can check. NIH-DHAARA's sub-daily values are derived from hourly records, not disaggregated from daily totals. That makes it the only public source suitable for sizing drains, which depend on short-duration intensity. Its values should still be checked against the long IMD daily record at 24 hours and longer (Annex 2A). In Chhattisgarh the platform does not yet meet that purpose. Its present-day values there come from IMDAA reanalysis (about 12 km), not from gauges, and they err in both directions. Its 50-year 24-hour depths are close to the CWC plates at Raipur (313 mm, against about 280–320 mm), Rajnandgaon (307 mm), Dhamtari (302 mm), Bilaspur (217 mm, against 200–240 mm) and Raigarh (316 mm); above them at Durg (333 mm) and Bhilai (326 mm), against about 240–280 mm, at Korba (306 mm), Nava Raipur (349 mm), Mahasamund (380 mm, against 320–360 mm) and Jagdalpur (340 mm, against 240–280 mm); and below them at Ambikapur (249 mm, against a closed contour of more than 280 mm) and Kanker (247 mm, against 280–320 mm). Its 100-year depths are below the station records at Raipur (353 mm, against 370.3 mm), Rajnandgaon (346 mm, against 392.0 mm), Korba (344 mm, against 358.0 mm), Raigarh (355 mm, against 360.9 mm) and, by far, Jashpur Nagar (233 mm, against 365.0 mm). Its 50-year 1-hour depths, 54–85 mm, are only 0.20–0.29 of its 24-hour depths, all below the CWC ratios of their subzones, 0.32–0.39 (NIH-DHAARA, queried 7 Nov 2026; IMD; CWC). A source that errs by these margins cannot be adopted, so Clause 2.3.6 uses the CWC reports and the long IMD station records until Annex 2A has been done, and uses NIH-DHAARA for comparison and as a check on the climate factors only. In one respect Chhattisgarh is like Madhya Pradesh and Gujarat: the old recording-gauge maxima of the reports do not exceed the 1-hour depths that the plates and ratios give, and every other source found gives less, so the clause sets no short-storm floor and names the data that would change that (Clause 2.3.6(g)). In another it differs: at several gauges of confirmed long record the 24-hour records exceed the 100-year depths of the plates, at Raipur (Obsy) since 1910 and Champa since 1947, and at Jashpurnagar and Korba since the plates were drawn, in 2001 and 2004, so the plates understate the tail on which floors, storage and spillways depend. Clause 2.3.6(h) answers that with a check rather than a new statistic. A recorded day is a depth the design must at least carry; a record that is the highest of more than 100 years at one gauge, as Raipur's is, has an empirical return period of the order

of 100 years, and for a shorter record the check is conservative. Its test of record length keeps a gauge of a few years, or one whose length is not known, from setting the depth for a whole town: higher district-gauge values at Rajnandgaon, Durg, Saraipali, Basna and Samaria wait for IMD to confirm their records. Its radius of 25 km matches the spread of the State's record storms: on 4 August 1910 Raipur recorded 370.3 mm, Kurud 381.0 mm and Arang 318.0 mm, and on 30 June 2007 the Raipur city gauge and the airport gauge 15 km away read 297.0 mm and 273.0 mm (IMD). The check stops at 24 hours and longer because a record day says nothing about its peak hour. The State is crossed by five CWC subzones whose 1-hour ratios run from 0.32 (3(g), 3(f)) to 0.39 (3(d)), so that for the same 24-hour depth a 3(d) town such as Raipur gets a 1-hour depth about 22% higher than Jagdalpur in 3(g), and Jagdalpur itself lies on a subzone line; that is why Clause 2.3.6(b) assigns every district and applies the higher value near the lines. The interim rules for the more frequent AEPs, the 48- and 72-hour depths, the reading of the plates, the subzone of each district, the record check, the use of the NRDA chart and durations below 1 hour are set by this Manual. Several points are unresolved. The plate values were read by eye on scans, to about one contour; the 50-year contours at Raigarh are unlabelled, and those at Manendragarh and Jashpur Nagar run too close to read [TBC]. The copy of the 3(f) report obtained has no plates, and its loss rate is given as 2 mm/h in one source and as 0.5 or 0.2 cm/h by the 3(g) report, which adopts its relations [TBC]; the 3(c) plates were not read for the State, and the 3(d) areal reduction table was not re-read [TBC]. The 1(d) ratio at 18 hours was read from OCR text [TBC]. In the recording-gauge annexes, the Champa row of 3(d) is garbled, the Jagdalpur row of 3(g) prints the same 80.0 mm at 1 and 3 hours on a date that reads as February, and the 12- to 24-hour columns of the 3(g) annex were not read [TBC]. Wadrafnagar's 917.2 mm, which IMD prints as the State's record, and Ambikapur's 456.7 mm await IMD's confirmation, and the record lengths of the district gauges at Rajnandgaon, Durg, Saraipali, Basna, Samaria and elsewhere are to be obtained from IMD (Clause 2.3.6(h)). No storm-drain DPR for any town of the State with its design return period and rainfall analysis was found for this Manual, other than the NRDA plan [TBC], so the design basis of the drainage works now being built could not be compared with this clause.

2.4 Design return periods (summary)

2.4.1 Design AEPs are set in the chapters for each element. They are summarised here so that the correct rainfall is extracted at the start of design.

Table 2.1 — Design return periods (summary)

Element	Design AEP (return period)	Check event	Chapter
Minor drains: Raipur, Bhilai, Durg, Bilaspur and Korba core and high-density areas	10% (10-yr)	1% (100-yr) overland path	6
Minor drains: other Class I urban areas	20% (5-yr)	1% overland path	6
Minor drains: other areas	50% (2-yr)	1% overland path	6
Arterial roads and crossings	2–4% (25–50-yr)	next higher AEP	6
Collector / local roads	4% / 10% (25 / 10-yr)	2% / 4% (50 / 25-yr), plus the 1% overland path	6
Major (overland) system	1% (100-yr)	—	6
Retention volume	Percentile daily rainfall (Clause 2.10)	—	4
Detention: standard catchments	50%, 10%, 1% (2, 10, 100-yr)	1% emergency spillway	4
Detention: flood-sensitive catchments	Mixed-frequency rule, Clause 2.11	1%	4

Element	Design AEP (return period)	Check event	Chapter
Building floor levels	1% + 0.5 m freeboard	—	6
Detention emergency spillway	1% with outlet blocked	0.2% (500-yr) where high hazard	4

> **Commentary.** These values exceed the CPHEEO (2019) national advisory minimums of 5-yr (Class I cities) and 2-yr (other areas). CPHEEO allows a State to set higher values. The values are set in Chapters 4 and 6, and the table is reproduced here for convenience.

2.5 Design durations

2.5.1 **Conveyance elements** (pipes, open drains, inlets, culverts) shall be designed with the intensity for a duration equal to the time of concentration (Tc) of the contributing area (Chapter 3). The minimum duration is **10 minutes**.

2.5.2 **Storage elements** (detention, retention, tanks, pumped sumps) shall be checked for every duration in the set {1, 2, 3, 6, 12, 24, 48, 72 h}. The largest required storage or water level governs. The 24-, 48- and 72-hour storms are **mandatory** checks at every site, whatever the critical-duration result.

2.5.3 **Tidally influenced outfalls.** For storage draining to a tidally influenced outfall (Chapter 5), the 48- and 72-hour storms shall also be run with the tailwater boundary of Clause 2.12.

> **Commentary.** Chhattisgarh's worst floods have come when a long wet spell fills the reservoirs and raises the rivers. On 28–29 August 2020 Akaltara recorded 282.7 mm, the highest fall of the season (IMD), the Kharun at Pathardihi rose above its highest level of 1994 and the Mahanadi set new highest levels at Kalma and Surajgarh (CWC data as transcribed by SANDRP); in 1994, 21 of the State's 27 districts had excessive rainfall, and the Kharun's previous highest level dates from that July (IMD; as reported). In September 2026 all 14 gates of Gangrel were open while the Kharun overflowed at Raipur and the Indravati rose above its 1973 level at Jagdalpur (as reported). The monthly records show how much rain a single month can bring: 988.8 mm at Raipur in July 1884, 952.7 mm at Raigarh in July 1961, 904.7 mm at Jagdalpur in August 1925 and 1,606.0 mm at Bailadila in August 1994 (IMD). Storms move across the State over several days: Raigarh recorded 360.9 mm on 10 July 1958 and Lakholi, Sandi and Pallari 341–354 mm on 13 July, and in June–July 2001 heavy falls at Pankhajur (413.0 mm), Jashpurnagar (365.0 mm) and Charama (497.8 mm) came within four weeks of each other (IMD). Gangrel stood at 99.49% of its capacity in October 2025 when it released 55,000 cusecs through eight gates (as reported), so the drains that outfall below it, and below Bango and Darri on the Hasdeo, meet a raised water level that the whole wet spell, not its peak hour, has set. A rule based only on critical duration, applied to a small site, picks a 1–2 hour storm and misses this failure mode. That is why the long-duration checks are mandatory.

2.6 Sub-daily rainfall

2.6.1 Where NIH-DHAARA or an approved station IDF provides values directly for the required duration, those values shall be used.

2.6.2 Where only daily (24-hour) depths are available for a duration below 24 hours, the sub-daily depth shall be derived using the **ratio set of Annex 2B**. These are duration-to-24-hour ratios calibrated to Chhattisgarh recording gauges. They shall not be lower than the ratios of the CWC Flood Estimation Report for the subzone (3(d), 3(g), 3(f), 1(d) or 3(c)). IMD Das et al. (2022) has no Chhattisgarh station panel and is not used as a floor in Chhattisgarh.

2.6.3 **Fixed-interval adjustment.** Depths from fixed observation windows (for example 08:30–08:30 IST daily data, or clock-hour data) shall be converted to sliding-window ("true interval") depths:

- 1.13 for a single fixed observation interval;
- 1.04 for 2 intervals;
- 1.02 for 3–4 intervals.

This adjustment does not apply where the approved source already gives true-interval values.

2.6.4 The empirical relationship P(t) = P(24)·(t/24)^(1/3) shall not be used for design unless Annex 2B confirms it for the zone concerned.

> **Commentary.** The one-third power rule is common in Indian project reports, but no primary IMD source for it was found during research for this Manual. It applies one fixed curve shape everywhere: P(1h)/P(24h) = 0.35, P(3h) = 0.50, P(6h) = 0.63 and P(12h) = 0.79. It cannot represent the difference between a short convective burst and a long SW monsoon soak, and it tends to understate short-duration depths in convective coastal storms. Annex 2B shall tabulate the NIH-DHAARA ratios P(t)/P(24h) against these values by zone. That comparison shows whether the rule is acceptable anywhere in the State and corrects older daily-based analyses. The fixed-interval factors are the standard WMO (Hershfield) values. The IMD 2022 study applies similar clock-hour to any-hour corrections.

2.7 Point, grid and areal rainfall

2.7.1 Design depths are point values. Where an approved source provides grid-average values (for example 0.25° cells), they shall be converted to point values with the grid-to-point factors in Annex 2C before use.

2.7.2 For catchments **≤ 10 km²**, ARF = 1.0.

2.7.3 For catchments **> 10 km²**, apply ARF as follows:

- for 25–5,000 km² in subzone 3(d), 3(g), 3(f), 1(d) or 3(c): use the ARF curves of the applicable CWC Flood Estimation Report;
- for 10–25 km²: interpolate linearly between ARF = 1.0 at 10 km² and the CWC value at 25 km².

2.7.4 An ARF shall never be applied to storage design for a single development site.

> **Commentary.** Gridded and district-average return levels are much lower than point values. For example, NIH-DHAARA's IMDAA cell at Jashpur gives a 100-year 24-hour depth of about 233 mm, while the Jashpurnagar gauge recorded 365.0 mm on 30 June 2001, and its cell at Raipur about 353 mm, while the Raipur observatory recorded 370.3 mm on 4 August 1910 (NIH-DHAARA, queried 7 Nov 2026; IMD). At Jagdalpur the same product errs the other way: about 340 mm at 50 years, against a plate value of 240–280 mm and a station record of 259.7 mm. Single storms are patchy at a scale that no cell of about 12 km can show: on 30 June 2007 the Raipur city gauge recorded 297.0 mm and the airport gauge at Mana, about 15 km away, 273.0 mm, while Dhamdha, in Durg district, recorded 127.0 mm (IMD). Neither error can be corrected without the factors of Annex 2C. Using grid values as point values without correction would make site-level design unsafe.

2.8 Climate factor

2.8.1 Until a statewide future-climate rainfall product is adopted, present-day design depths shall be multiplied by the following interim climate factors:

Table 2.2 — Interim climate factors

Duration	Climate factor (CF)
≤ 6 h	**1.20**
> 6 h to 72 h	**1.10**

2.8.2 The CF applies to all elements with a design life of more than 25 years. For temporary works (design life ≤ 5 years), CF = 1.0.

2.8.3 The climate factor does not apply to the retention depth (Clause 2.10) or to the water quality volume depth (Chapter 8). Those depths are percentile or frequent-event statistics of everyday rainfall, not AEP design values.

2.8.4 Where NIH-DHAARA, or a product adopted on review under Clause 2.14, provides projected future-climate depths, those depths shall replace the interim factors.

> Commentary. CPHEEO (2019) considers a fixed climate factor "not feasible" nationally and recommends trend testing instead. That leaves designers without a usable number. The 1.20 short-duration factor matches the UK value cited by CPHEEO itself. It is at or below published projections for India: Ali and Mishra (2018) project 3-hourly 100-year maxima rising by about 20% at 1.5 °C and 25% at 2.0 °C of warming. No design uplift factor has been adopted in Chhattisgarh, and the State Action Plan on Climate Change of 2014 is summarised as finding falling pre-monsoon and monsoon totals and a variability of monsoon rainfall more than twice the national average (as summarised [TBC]). Totals and extremes point different ways. A study of 15 stations of the Mahanadi basin in the State over 1971–2021 is reported to find rising maximum 1-day and 5-day rainfall, strongest at Seorinarayan and Rajim (seen only through a summary; journal and authors not verified [TBC]); a study of the districts finds no significant trend in heavy-rain days in most of them (2019); and a regional atlas projects more days of more than 100 mm in every district of the State in the 2030s (CSTEP, 2022, as summarised). Recent extremes have not fallen: Jashpurnagar recorded its highest day, 365.0 mm, in 2001, Durg 337.0 mm and Korba 358.0 mm in 2004, Bilaspur 277.0 mm in 2005, Konta 340.0 mm and Sukma 325.0 mm in 2006 and Raipur 297.0 mm in 2007, all after the CWC plates of 1987–1996 were drawn (IMD). NIH-DHAARA offers a CMIP6 projection but no historical run of the same model, so no ratio of future to present depths can be formed; under Clause 2.3.6(k) it would serve only as a check. The factors of Table 2.2 are therefore interim values, not Chhattisgarh estimates.

2.9 Temporal patterns (design hyetographs)

2.9.1 **Storms up to 24 hours.** Use the nested **alternating-block** method:

- construct the storm from the same IDF relationship (after the climate factor), at time steps no longer than Tc/3 and no longer than 15 minutes;
- place the peak block at **50%** of the storm duration.

2.9.2 **48- and 72-hour storms (SW monsoon pattern).** Construct as follows:

- (a) The total storm depth equals the design depth for the full duration (48 or 72 h).

- (b) The **final 24 hours** contain the 24-hour design depth for the same AEP, distributed by the alternating-block method of 2.9.1.
- (c) The remaining depth (total minus the 24-hour depth) is spread uniformly over the preceding 24 or 48 hours.

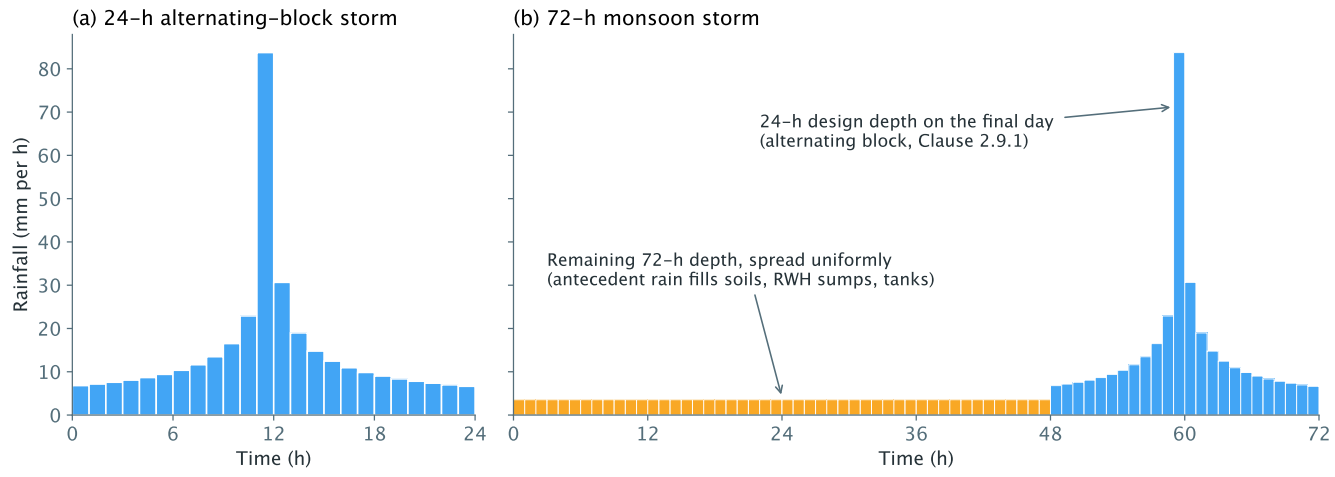

Figure 2.2 — (a) 24-hour alternating-block design storm; (b) 72-hour SW monsoon design storm with the 24-hour design depth on the final day. Illustrative depths only. Original drawing for this Manual. The alternating-block method follows Chow, Maidment & Mays, Applied Hydrology (1988), §14.4.

2.9.3 **Other temporal patterns.** Huff-type or mass-curve patterns derived from Chhattisgarh storms may be used instead of 2.9.1 and 2.9.2 only after they are published in Annex 2D. SCS Type I, IA, II and III distributions shall not be used.

2.9.4 **Large catchments.** For catchments of 25–5,000 km², the CWC Flood Estimation Report time-distribution curves may be used, and a sensitivity run with 2.9.1 shall be reported.

> **Commentary.** The alternating-block storm embeds every shorter-duration design intensity in one hyetograph. That makes it conservative and consistent across durations. The 72-hour pattern puts the most intense day last. This reproduces the monsoon sequence of Chhattisgarh: by the time the largest falls arrive in July–September, the kanhar clays of the plain have closed their cracks, the bunded paddy fields and the talabs are full, Gangrel, Minimata Bango and the barrages may already be releasing, the Hirakud reservoir may be backing water up the Mahanadi towards Raigarh, and the water table is within a few metres of the surface over most of the State. SCS distributions are excluded because they come from US convective and Gulf storm data and have no basis in Chhattisgarh records.

2.10 Retention rainfall depth

2.10.1 The **retention design depth** is the 24-hour rainfall depth not exceeded on **90%** of days with rain (≥ 2.5 mm) in the long-term record, read from the zone value in Annex 2E.

2.10.2 The site must retain on site the runoff from this depth (Chapter 4). Retention earns a detention credit equal to the reduction in required detention storage found by routing with the retention in place (Clause 4.4.6). The credit is not the retained volume subtracted one-for-one.

> **Commentary.** This follows the Kansas City Metro APWA 5600 (2025.01) Required Retention Volume concept and Georgia's 1.0-inch runoff reduction standard. Both are based on percentile storms. A percentile depth is a statistic of everyday rainfall, not an AEP value, so it is computed from the daily record rather than from the frequency curves.

2.11 Design storms for pre- and post-development comparison

2.11.1 **Same storm for both scenarios.** Pre- and post-development runs shall use the same design storms:

- same AEP, duration, temporal pattern and climate factor;
- same antecedent moisture condition (Clause 2.11.4).

The two runs differ only in land surface parameters and Tc.

2.11.2 **Standard catchments.** For each of the 50%, 10% and 1% AEP storms and each duration in Clause 2.5.2:

> $Q_{post}(T) \leq 0.9 \times Q_{pre}(T)$

2.11.3 **Flood-sensitive catchments** (as listed in Chapter 4: the catchments of the Kharun nalas at Raipur, of the Arpa at Bilaspur, of the Seonath nalas at Durg–Bhilai, of the Hasdeo below Minimata Bango and Darri at Korba, of the Hirakud backwater at Raigarh and Sarangarh, and of the Indravati at Jagdalpur, and any catchment notified by the authority). The **mixed-frequency rule** applies:

> Q_{post}(1% AEP, 100-yr) ≤ Q_{pre}(10% AEP, 10-yr), and Q_{post}(50% AEP, 2-yr) ≤ Q_{pre}(50% AEP, 2-yr),

The post-development peak is checked for each duration in Clause 2.5.2. The caps are taken at the **pre-development critical duration**. The 10-yr cap is the largest pre-development 10-yr peak over all durations, and the 2-yr cap is the largest pre-development 2-yr peak over all durations. Each cap applies to every post-development duration.

2.11.4 **Antecedent moisture.** Use **AMC III** (wet conditions) for every design storm, whatever its duration, from 1 July to 30 September at a site on kanhar or dorsa paddy lowland, at a site that drains to a talab, and on a valley floor, in any sub-area of Annex 2A. Use AMC III for every design storm at any time of year in a catchment that drains to a reservoir or barrage pond, to the backwater of the Hirakud reservoir, to a mine void, or to a river reach whose CWC site stands at or above its warning level in the coincident case of Chapter 5. At any other site or time, use AMC III for a design storm that follows, or contains, a 5-day rainfall total of more than 53 mm. Use AMC II (average conditions) for every other storm. AMC I is not used for design.

2.11.5 **Pre-development condition.** Pre-development is defined in Chapter 3: greenfield, open land in good condition for the mapped hydrologic soil group. Existing impervious cover is not counted as pre-development.

> **Commentary.** Matching each site's post-development peak to its own pre-development peak does not protect the catchment. Denver's analysis found that 100% matching at every site still produced 106% of historic peak downstream, which led to the 90% rule. The mixed-frequency rule is stricter. It caps the 100-yr post-development release at what the undeveloped site produced in a 10-yr storm. Malaysia (DID 2008: all ARIs ≤ pre-development 2-yr) is a tropical precedent for this kind of cap. AMC III for long storms reflects that SW monsoon extremes arrive after weeks of rain. In Chhattisgarh the soil and the land use of the plain, more than the season alone, decide how

wet the ground is. The plain is a catena of four soils: bhata on the crests, shallow and dense over murram; matasi on the upper slopes, a sandy or silty loam; dorsa on the lower slopes, a clay loam; and kanhar in the lowest flats, a deep black clay with 40–50% clay and low permeability (CGWB; Mandal et al., 2018, for Durg district). Much of the dorsa and kanhar land is bunded and puddled for paddy from the onset of the monsoon, so that by July the lowland is standing water, and the talabs, the reservoirs and the barrage ponds fill through the season and stay full. July and August bring nearly 58% of the annual rainfall (IMD), and after the monsoon about 76% of the observation wells of the State have water within 5 m of the surface (CGWB, 2023). The loss rates of the CWC report for the Mahanadi subzone, 1.5–3.5 mm/h, were derived from plain catchments with paddy land and reflect saturated August conditions, against a measured basic infiltration rate of 7.3–7.6 mm/h on an upland sandy loam at Raipur (Ranjan and Singh, 2018). This Manual therefore uses AMC III from 1 July to 30 September on kanhar and dorsa paddy lowland, at talab-fed sites and on valley floors; at any time of year where the outfall is a reservoir or barrage pond, the Hirakud backwater, a mine void or a river at or above its CWC warning level, because those receiving waters are high only when the catchment is wet; and elsewhere only after a 5-day total of more than 53 mm, the NRCS growing-season threshold for AMC III; AMC II otherwise (Clause 2.11.4). AMC I is not used: the bhata uplands, the driest soils, are shallow and saturate early. Kanhar remains hydrologic soil group D whatever its cracks (Chapter 3). These are interim choices set by this Manual; no study in the State has yet validated AMC classes against observed runoff.

Illustrative scale of the mixed-frequency rule. The values below are not design values. They assume a 2 ha site in a colony draining to the Kharun in Raipur with 24-h depths of 200 mm (10-yr) and 350 mm (100-yr), SCS-CN runoff with Ia = 0.2S, and CN 70 pre-development and CN 92 post-development:

Table 2.3 — Illustrative scale of the mixed-frequency rule (not design values)

Case	Runoff depth	Runoff volume (2 ha)
Pre-development, 10-yr 24 h	111 mm	2,210 m³
Post-development, 100-yr 24 h	325 mm	6,500 m³

The post-development 100-yr storm generates about three times the runoff volume of the pre-development 10-yr storm. A peak-rate cap at the pre-development 10-yr level means most of the difference must be stored and released slowly, so storage of the order of 0.2 m³ per m² of site may be needed. For this reason the rule applies only in flood-sensitive catchments (Clause 2.11.3); elsewhere the 0.9 × Q_pre rule of Clause 2.11.2 applies.

2.12 Joint occurrence with tailwater

2.12.1 For outfalls under tidal or river backwater influence, the design rainfall of this chapter shall be combined with the tailwater conditions of Chapter 5 as follows. The more severe case governs.

- (a) Design rainfall (AEP as required) with a mean high water springs (MHWS) tailwater plus the sea-level allowance.
- (b) A 10% AEP (10-yr) 48-hour storm with the 1% AEP still-water level (tide plus surge) plus the sea-level allowance.

Chhattisgarh has no coast, so no outfall in the State is tidal and cases (a) and (b) do not arise. The tailwater is the river, reservoir, barrage pond, talab or mine void into which a town drains. For an outfall under river or reservoir backwater, the site storm is combined with the receiving-water level under Clause 5.8.4, which pairs a design event on one side with a 10-yr event on the other in the same way as case (b), and the levels are those of Clause 5.8.5. Only one CWC site in the State has published warning and danger levels: the Indravati at Jagdalpur, 539.49 m and 540.80 m (CWC advisories of 2019, seen as search extracts), whose highest flood level of 544.68 m (9 July 1973) was exceeded on 26 September 2026, when the river reached 544.891 m (as reported). A local gauge at Jagdalpur read 12.56 m on that day, "4.26

metres above the danger mark" (as reported); its scale is not that of the CWC site, and the two are not mixed. The Kharun at Pathardihi, Raipur's river, has a highest flood level of 281.68 m (29 August 2020), 2.47 m above the 279.21 m of 12 July 1994, and no published warning or danger level; the Hasdeo set new highest levels at Katghora, 313.58 m (20 July 2026), and Bangodam, 303.5 m (17 August 2026), the Mand at Kurubhata reached 220.4 m and the Mahanadi 202.43 m at Kalma and 201.06 m at Surajgarh in August 2020 (CWC data as transcribed by SANDRP). The reservoirs and barrage ponds that receive or control the towns' outfalls have full reservoir levels of 348.700 m at Gangrel (Ravishankar Sagar), 359.660 m at Minimata Bango, 294.110 m at Kharang, 302.000 m at the Arpa Bhaisajhar barrage, 337.150 m at the Mongra barrage on the Seonath and 233.000 m at the Kelo dam (WRD daily tank gauge report of 10 October 2024). The low outfalls of Raigarh and Sarangarh lie within the backwater of the Hirakud reservoir in Odisha, whose levels are to be obtained through WRD [TBC]. These rivers rise with the releases of Gangrel, Bango, Darri and the barrages, and the reservoirs stay full for weeks once filled, so storage and pumps are checked for the outfall-closed condition, with the local storm falling before, during or after the release. No document found in research for this Manual sets a design receiving-water level for the outfalls of any town in the State [TBC], and Chapter 5 sets the interim rule: for a reservoir, barrage pond or talab, its maximum water level, or its full reservoir level plus the surcharge of the design flood, from WRD or the custodian, and, where neither is published, the full reservoir level plus 0.5 m; for a river, a level related to the highest recorded at the nearest gauge. Every level is stated on a named datum, as Chapter 5 requires; no source found states the datum of the levels quoted here [TBC].

> Commentary. Chhattisgarh has no coast, so no outfall is tidal and there is no storm surge; the coincident load is the level of the rivers, reservoirs and barrage ponds that receive the town drains. The coincidence is on record. On 28–29 August 2020 the Kharun at Pathardihi, below Raipur, rose to its highest recorded level while the same storm fell on the city, and the Mahanadi and the Mand set new levels downstream (CWC data as transcribed by SANDRP); in September 2024 the Kharun cut the road from Raipur to Bhilai at Mahadev Ghat, and in September 2026 it overflowed there again while all 14 gates of Gangrel were open on the Mahanadi and the Indravati at Jagdalpur rose above its highest level of 1973 (as reported). At Korba, the releases of Minimata Bango, about 30,000 cusecs through six gates, and of the barrage at Darri flooded parts of the town in August 2024 (as reported). Downstream, the low outfalls of Raigarh and Sarangarh are held by the backwater of the Hirakud reservoir in Odisha, whose gates have been opened at Chhattisgarh's request in floods (as reported). These levels are set by rain over catchments upstream and by the operation of gates, so they give no warning of a local storm that falls while they are high. The levels quoted by different sources are on different gauges: at Jagdalpur a local gauge read 12.56 m on the day the CWC site read 544.891 m (as reported), which is why Chapter 5 requires every level to name its datum. Because rainfall and surge share a weather cause (in Chhattisgarh, which has no coast, the counterpart of surge is the stage of the rivers and reservoirs that receive the town drains, raised by the same Bay of Bengal depressions that bring the heaviest rain to the towns and by the releases of the dams and barrages that those systems fill: in September 2026 all 14 gates of Gangrel were open and the Indravati set a new highest level at Jagdalpur while the Kharun overflowed at Mahadev Ghat in Raipur, all as reported), they cannot be treated as independent. Case (b) is a simple joint-probability surrogate until a statistical joint analysis is adopted.

2.13 Submittal requirements

Every hydrologic submittal shall include a **Design Rainfall Sheet** (Form 2-1) stating:

1. the site coordinates and zone;
2. the source (Clause 2.3), version or access date, and a screenshot or export of the values extracted;
3. a depth table for each AEP and duration used, before and after the fixed-interval factor, grid-to-point factor, ARF and climate factor;
4. the cross-check values of Clause 2.3.3 and the floor check;
5. the hyetographs used, as a table and a plot, for each design storm; and
6. the AMC and pre-development definition adopted.

Figure 2.3 shows the steps of this chapter in the order in which the sheet records them.

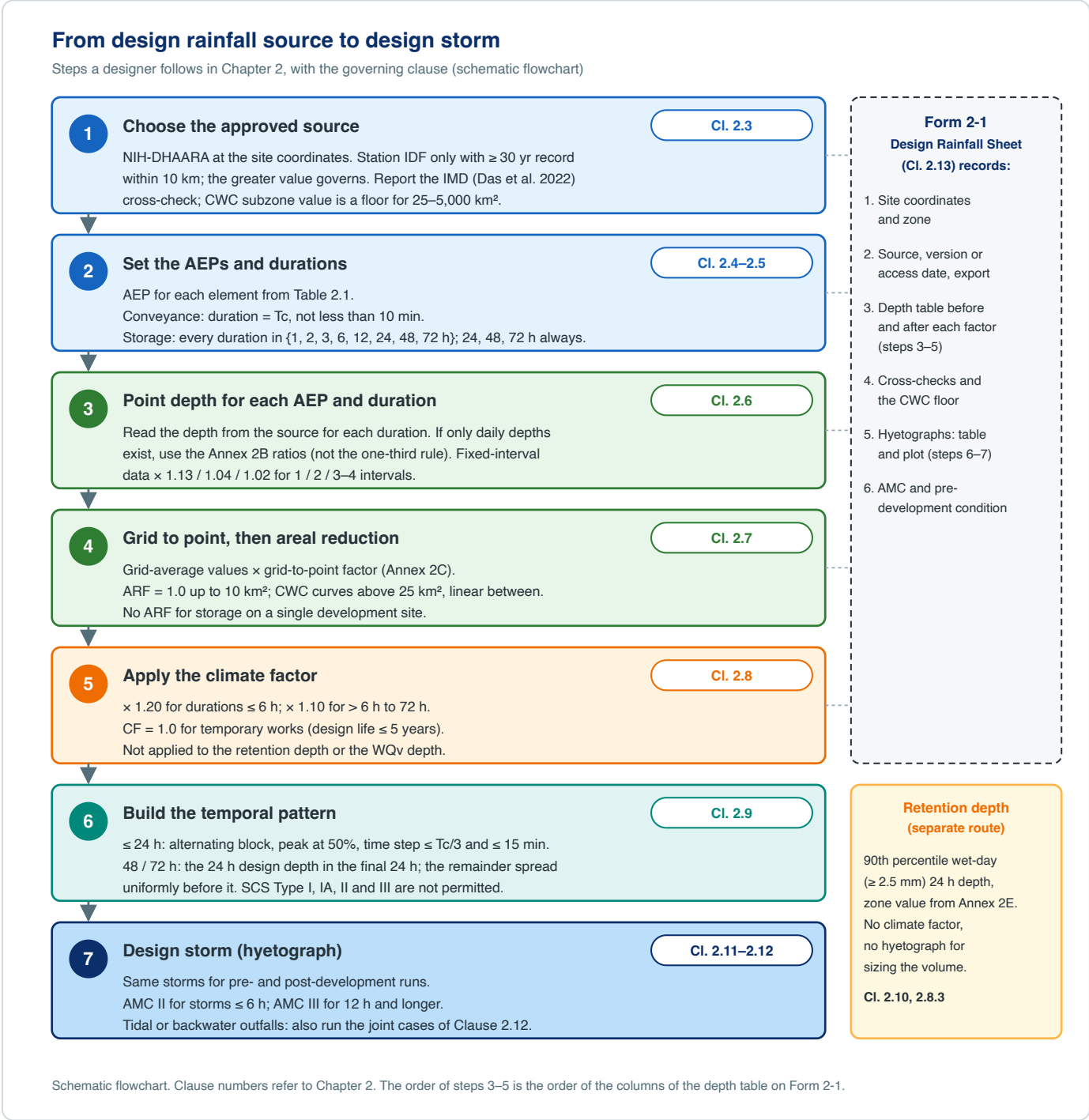

Figure 2.3 — From design rainfall source to design storm: the steps a designer follows (source, AEPs and durations, point depth, grid-to-point and areal reduction, climate factor, temporal pattern, design storm), the clause that governs each step, what Form 2-1 records, and the separate route for the retention depth. Schematic. Original drawing for this Manual.

2.14 Review and update

2.14.1 The approving authority shall review this chapter **every 10 years**. It shall also review it within 2 years of any event in which recorded rainfall exceeds the 1% AEP 24-hour or 72-hour design depth at two or more approved gauges in the State.

2.14.2 Updates to NIH-DHAARA take effect for new submittals 6 months after release, unless the authority notifies otherwise.

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Annexes

Annex	Content
2A	NIH-DHAARA vs IMD station and IMD gridded checks at benchmark points; zone map
2B	Sub-daily ratios P(t)/P(24h) by zone: NIH-DHAARA (hourly-based) vs the one-third rule
2C	Grid-to-point factors
2D	Chhattisgarh temporal patterns (Huff curves)
2E	Retention percentile depths by zone

Annex	Content
Form 2-1	Design Rainfall Sheet

CHAPTER 3

Hydrologic Methods

3.1 Purpose and scope

3.1.1 This chapter sets out how design rainfall from Chapter 2 is turned into runoff peaks, volumes and hydrographs for:

- (a) drain, inlet and culvert sizing (Chapter 6);
- (b) retention and detention sizing, and the pre- vs post-development comparison (Chapter 4);
- (c) downstream and tailwater analysis (Chapter 5); and
- (d) waterbody, tank (talab) and regional facility studies (Chapter 7).

3.1.2 It specifies:

- (a) which method may be used for which purpose and catchment size (Clause 3.4);
- (b) how the pre- and post-development land surfaces are defined (Clause 3.3);
- (c) the parameters of each method: runoff coefficients, time of concentration, soils, curve numbers, unit hydrographs and routing (Clauses 3.5–3.12);
- (d) when continuous simulation and model calibration are required (Clauses 3.13 and 3.15);
- (e) the step-by-step pre- vs post-development comparison (Clause 3.14); and
- (f) software acceptance and documentation (Clauses 3.16 and 3.17).

3.1.3 Rainfall inputs (depth, duration, areal reduction, climate factor, temporal pattern and antecedent moisture) are governed by Chapter 2 and shall not be varied in this chapter.

> **Commentary.** The CPHEEO (2019) Manual on Storm Water Drainage Systems gives a full Rational Method procedure and names SWMM for hydrograph work, but it is a drain-sizing guide. It does not say how a regulator should define "pre-development" or which method must be used to test a development against it (CPHEEO Part A §4.4, §8.5.1). APWA Section 5600 does both: it limits the Rational Method, names baseline hydrograph methods, and fixes the land-use assumptions for existing and proposed conditions (KC Metro APWA 5600, 2011, §5602.2–5602.3; Version 2025.01, §5603.4). This chapter follows that model.

3.2 Definitions

Term	Meaning in this Manual
Pre-development condition	The reference land surface defined in Clause 3.3.2, against which post-development runoff is tested. It is not the existing condition.
Post-development condition	The land surface after the proposed works are complete, as defined in Clause 3.3.3.
Point of comparison (POC)	A location where site runoff leaves the site, at which pre- and post-development flows are compared (Clause 3.14).

Term	Meaning in this Manual
Time of concentration (Tc)	The time for runoff to travel from the hydraulically most distant point of the catchment to the point of interest.
Lag (tL)	The time from the centroid of rainfall excess to the peak of the runoff hydrograph.
Hydrologic soil group (HSG)	The NRCS classification (A, B, C, D) of a soil by its minimum infiltration rate when wet (USDA NRCS, NEH Part 630, Ch. 7).
Curve number (CN)	The NRCS parameter that combines soil group, land cover and antecedent moisture into a single runoff-potential index (USDA NRCS, NEH Part 630, Ch. 9–10).
Antecedent moisture condition (AMC)	The soil wetness at the start of a storm: AMC I (dry), II (average), III (wet).
Directly connected impervious area (DCIA)	Impervious surface that drains to a drain, pipe or channel without first flowing over a pervious surface.
Disconnected impervious area	Impervious surface whose runoff flows as sheet flow over a pervious surface before reaching the drainage system, and meets Clause 3.9.3.
Peak rate factor (PRF)	The constant in the unit hydrograph peak equation (484 in US customary units; 0.208 in the SI form of Clause 3.11.2).
Continuous simulation	A model run over a long rainfall record (years), with soil moisture, storage and tank levels carried forward between storms.
Tank cascade	A chain of tanks (talabs) in which the surplus of one tank feeds the next.

3.3 Pre-development and post-development land surface

3.3.1 **Same site, same storm.** Pre- and post-development runs shall cover the same area draining to each POC, and use the same design storms (Clause 2.11.1). They shall differ only in the land-surface parameters of this clause, the resulting Tc, and the stormwater controls.

3.3.2 **Pre-development condition.** The pre-development land surface shall be **greenfield: open land in good condition for the mapped hydrologic soil group**, as follows:

- (a) All land within the site, including land that is now built on, paved, compacted or cultivated, is treated as open land (grass cover > 75%) in good condition. **Existing impervious cover shall not be counted.**
- (b) The curve number is the TR-55 value for "open space, good condition" for the HSG of Clause 3.7: **CN 39 (A), 61 (B), 74 (C), 80 (D)** at AMC II (Table 3.5).
- (c) The HSG is the natural HSG of Clause 3.7, including the dual-group rule for high water table (Clause 3.10.1). No HSG improvement for drainage works is allowed.
- (d) Tc is computed on the pre-development ground surface using unchannelled overland and shallow concentrated flow and natural channels only (Clause 3.6). Existing pipes, lined drains and kerbs are ignored.
- (e) Existing waterbodies, tank waterspreads up to the full tank level (FTL), natural channels and supply or surplus channels are kept as they are, and modelled as water surfaces or channels, not as open land.

3.3.3 **Post-development condition.** The post-development land surface shall represent the proposed works at full build-out:

- (a) Impervious areas shall be measured from the site plan, separately for roofs, paved surfaces, roads and other sealed surfaces, and classified as directly connected or disconnected (Clause 3.9). Lot-size residential rows of TR-55 shall not be used for the site itself.

- (b) Pervious areas disturbed by construction (graded, filled or trafficked) shall be assigned one HSG lower in infiltration capacity than the natural HSG (A to B, B to C, C to D), unless post-construction infiltration tests (Clause 3.7.3) show otherwise. Soil disturbed by construction shall never be classed as HSG A.
- (c) Rainwater harvesting (RWH) sumps shall be treated as full at the start of storms of 12 hours or more, and the roof area draining to them as directly connected for those storms (Chapter 4).
- (d) Retention and detention facilities shall be represented explicitly as storage (Clause 3.12), not by lowering CN.

3.3.4 **Redevelopment.** The pre-development condition of Clause 3.3.2 applies to redevelopment as well. A site that is already fully paved is tested against greenfield, not against its existing state. For the constrained-redevelopment route of Clause 4.5.6, the existing condition shall also be modelled, with existing impervious areas as surveyed, so that the 50% minimum can be checked.

3.3.5 **Off-site areas.**

- (a) For conveyance design (Chapter 6), areas upstream of the site shall be taken at their ultimate land use under the approved master plan or development plan. Where no plan exists, upstream undeveloped land shall be assumed to be at least 35% impervious.
- (b) For the pre- vs post-development comparison, off-site run-on shall be modelled identically in both runs, at its existing condition (Clause 3.14.2).

> **Commentary.** A greenfield reference stops each redevelopment from being judged against an already degraded baseline. The UK non-statutory standards and KC APWA take the same approach in different forms: APWA caps existing agricultural land at no more than "good condition pasture, grassland or range (C = 0.30, CN = 74)" for HSG C, and forbids disturbed soils from being classed as HSG A or B (KC Metro APWA 5600, 2011, §5602.3A and E). The TR-55 "open space, good condition" values used in (b) are identical to TR-55's "pasture, grassland or range, good" values (USDA SCS TR-55, 1986, Tables 2-2a and 2-2c), so this Manual's pre-development CN equals the APWA cap on HSG C soils. The one-class downgrade for disturbed soils in 3.3.3(b) is set by this Manual as a simpler, less strict version of the APWA rule, suited to the kanhar, dorsa, matasi and bhata soils of the Chhattisgarh plain, the limestone belt, Bastar granite and laterite, and the Gondwana coalfields, where an HSG A to C jump would be excessive. The 35% minimum imperviousness for unplanned upstream land in 3.3.5(a) is the APWA value (2011, §5602.3E.2). Using the master-plan land use for conveyance follows CPHEEO, which requires C to reflect "ultimate development of the catchment as per the master plan" (CPHEEO 2019, Part A §4.4.1.5).

3.4 Selection of method

3.4.1 The method shall be selected from Table 3.1 by purpose and catchment size. Figure 3.1 shows the same rules as a flowchart.

Table 3.1 — Permitted hydrologic methods

Purpose	Catchment / condition	Permitted method	Clause
Peak flow for one conveyance element (inlet, pipe reach, drain reach, culvert)	≤ 16 ha contributing, no upstream storage, no backwater	Rational Method	3.5
Peak flow for conveyance	> 16 ha, or any upstream storage, tank or pond	Hydrograph method (SCS-CN with SCS unit hydrograph, SWMM or HEC-HMS)	3.8–3.12
Retention, detention, any storage; pre- vs post-development comparison	Any size	Hydrograph method; **Rational Method not permitted**	3.8–3.12, 3.14

Purpose	Catchment / condition	Permitted method	Clause
Pipe or drain network with backwater, tidal or tank tailwater	Any size	Hydrograph method with dynamic-wave hydraulic routing	3.12.4; Chapter 5
Catchment-scale flows	25–5,000 km² in CWC subzone 3(d), 3(g), 3(f), 1(d) or 3(c)	HEC-HMS or SWMM, benchmarked against the CWC synthetic unit hydrograph (SUH)	3.11.4
Regional facilities	Draining > 25 km²	Design-storm hydrograph method **plus** continuous simulation	3.13
Tank (talab) cascades	Any size	Design-storm hydrograph method **plus** continuous simulation	3.13
Tier B: small sites under Chapter 1	Within the limits of the design aid	Design aids issued under Chapter 4, which are themselves derived by the methods of this chapter	3.4.4
Small sites, any authority	≤ 1 ha, within the limits of Clause 4.14.3	Fixed-coefficient route of Chapter 4 (fixed release coefficients, rainfall-mass storage)	3.4.4; 4.14.3

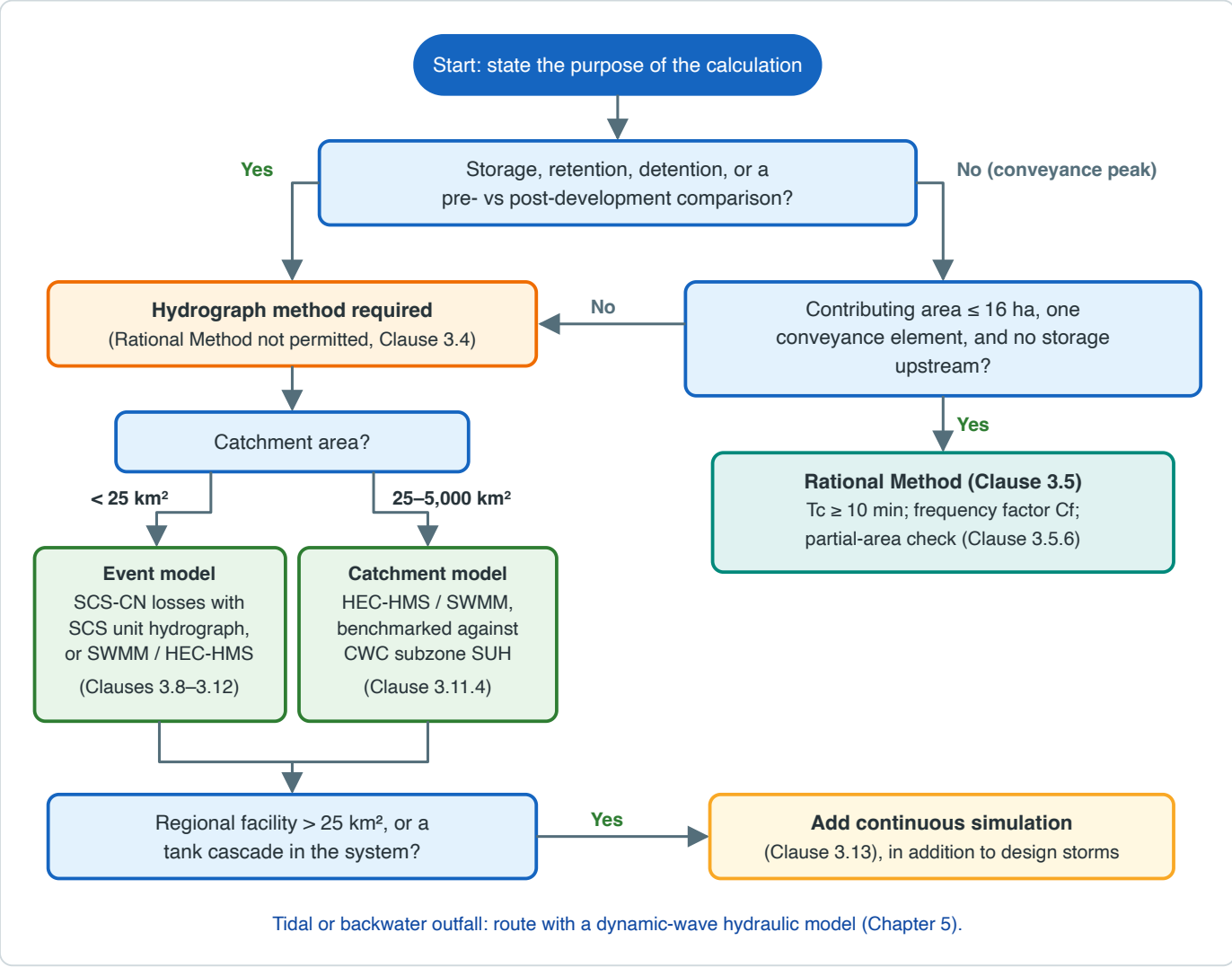

Figure 3.1 — Selection of hydrologic method. Original drawing for this Manual, adapted from KC Metro APWA, Section 5600 Storm Drainage Systems and Facilities, 2011 (§5602.2) and Version 2025.01 (§5603.4, Policy Supplement, 2025).

3.4.2 **Rational Method limits.** The Rational Method shall be used only for catchments of **16 ha or less** and only to compute the peak flow to a **single conveyance element**. It shall **never** be used to size retention, detention or any other storage, nor to derive a hydrograph or runoff volume.

3.4.3 **Hydrograph methods.** Above 16 ha, and for every storage calculation, a hydrograph method shall be used: SCS-CN losses with the SCS (NRCS) unit hydrograph, EPA SWMM, or HEC-HMS. For 25–5,000 km² the CWC synthetic unit hydrograph shall also be computed as a benchmark (Clause 3.11.4).

3.4.4 **Simplified routes.** In Tier B areas (Chapter 1), sites within the limits stated on a design aid issued under Chapter 4 may use that design aid in place of a site-specific hydrograph model. In any area, sites of 1 ha or less may use the fixed-coefficient route of Clause 4.14.3; it is the only exception to the rule in 3.4.2 that rainfall-based peak methods shall not be used for storage. The Rational Method remains available for conveyance under 3.4.2.

3.4.5 **Catchments above 5,000 km².** These are outside the scope of this Manual and require a basin study approved by the Water Resources Department (WRD).

> Commentary. The 16 ha limit and the single-element rule follow APWA 5600 Version 2025.01, which cut the Rational Method from 200 ac to 40 ac and restricted it to "a single pipe segment". The MARC Policy Supplement adds that it "cannot be used to size runoff reduction practices or determine retention volumes and detention volumes" (MARC, APWA Stormwater Standards Supplement, 2025). CPHEEO (2019, Part A §4.4.1.3) allows the Rational Method up to 8–10 km², which is too large for a method that assumes uniform rainfall intensity over the whole catchment for the whole of Tc and gives no volume. The Rational Method is excluded from storage design because it gives a peak, not a hydrograph, and storage depends on volume and timing.

3.5 Rational Method

3.5.1 **Equation.** The peak flow shall be computed as:

$$Q = C \cdot Cf \cdot i \cdot A / 360$$

where Q is the peak flow (m³/s); C is the runoff coefficient (Table 3.2); Cf is the frequency factor (Table 3.3), with $C \cdot Cf \le 1.0$; i is the rainfall intensity (mm/h) for a duration equal to Tc, from Chapter 2 (with the climate factor of Clause 2.8); and A is the contributing area (ha).

> Commentary. This is the CPHEEO equation Qp = 10 C I A, with Qp in m³/h (CPHEEO 2019, Part A §4.4.1.1–4.4.1.3), converted to m³/s.

3.5.2 **Runoff coefficients.** C shall be taken from Table 3.2. Within a range, the designer shall use the upper half of the range unless site data justify a lower value.

Table 3.2 — Runoff coefficient C (for AEP of 10% or more frequent)

Surface or land use	C	Source
Commercial	0.70–0.95	CPHEEO 2019, Table 4.2
Industrial	0.60–0.90	CPHEEO 2019, Table 4.2

Surface or land use	C	Source
Institutional	0.70–0.95	CPHEEO 2019, Table 4.2
Residential, high density	0.60–0.75	CPHEEO 2019, Table 4.2
Residential, low density	0.40–0.60	CPHEEO 2019, Table 4.2
Recreational	0.10–0.25	CPHEEO 2019, Table 4.2
Asphalt	0.70–0.95	CPHEEO 2019, Table 4.2
Concrete	0.80–0.95	CPHEEO 2019, Table 4.2
Brick paving	0.70–0.85	CPHEEO 2019, Table 4.2
Roofs: tiles	0.80–0.90	CPHEEO 2019, Table 4.2
Roofs: metal	0.70–0.90	CPHEEO 2019, Table 4.2
Roofs: concrete	0.70–0.90	CPHEEO 2019, Table 4.2
Lawns, sandy soil, flat (≤ 2%)	0.05–0.10	ASCE/WPCF 1969
Lawns, sandy soil, average (2–7%)	0.10–0.15	ASCE/WPCF 1969
Lawns, sandy soil, steep (> 7%)	0.15–0.20	ASCE/WPCF 1969
Lawns, heavy (clay) soil, flat (≤ 2%)	0.13–0.17	ASCE/WPCF 1969
Lawns, heavy soil, average (2–7%)	0.18–0.22	ASCE/WPCF 1969
Lawns, heavy soil, steep (> 7%)	0.25–0.35	ASCE/WPCF 1969
Undeveloped land (maximum for any condition)	0.30	KC APWA 5600 (2011), §5602.3E

3.5.3 **Frequency factor.** For design AEPs rarer than 10%, C shall be multiplied by Cf from Table 3.3. The product C · Cf shall not exceed 1.0.

Table 3.3 — Frequency factor Cf

Design AEP (return period)	Cf
10% (10-yr) and more frequent	1.00
4% (25-yr)	1.10
2% (50-yr)	1.20
1% (100-yr)	1.25

3.5.4 **Composite C.** Where a catchment contains more than one surface, C shall be the area-weighted value $C = \Sigma(C_j \cdot A_j) / \Sigma A_j$, computed from the measured areas of each surface. Cf is applied to the composite C.

3.5.5 **Time of concentration and networks.**

- (a) Tc shall be computed by Clause 3.6 and shall not be less than **10 minutes**.
- (b) For each reach of a network within the 16 ha limit, the peak shall be computed with the cumulative $\Sigma(C \cdot A)$ upstream of that reach and the longest Tc to that reach. Peaks from upstream reaches shall not be added.

3.5.6 **Partial-area check.** Where a compact, highly impervious part of the catchment has a much shorter Tc than the whole, the peak shall also be computed for that part alone, using its own area, C and Tc. The larger peak governs.

> **Commentary.** The developed-surface coefficients in Table 3.2 are CPHEEO Table 4.2, which CPHEEO adapted from ASCE/WPCF (1969). CPHEEO keeps C constant for all return periods. Table 3.3 instead adopts the APWA antecedent-precipitation factors (KC Metro APWA 5600, 2011, §5602.2A: K = 1.1, 1.2 and 1.25 for 4%, 2% and 1% storms, with C·K ≤ 1.0). These factors matter here because Chhattisgarh's rare storms arrive on wet ground (Clause 2.11.4). The 10-minute minimum Tc matches the minimum design duration of Clause 2.5.1. APWA 2011 bounds inlet time between 5 and 15 minutes (§5602.7A). The partial-area check follows CPHEEO's treatment of "partial area effect" (CPHEEO 2019, Part A §4.4.1.7). This Manual requires the upper half of each range in 3.5.2 because Indian urban plots are typically more densely covered than the US land uses behind the ASCE ranges.

3.6 Time of concentration

3.6.1 **General.** Tc shall be computed separately for the pre- and post-development conditions, along the flow path that gives the longest travel time to the point of interest, as the sum of travel times in each flow segment. Tc shall not be less than **10 minutes** for either condition.

3.6.2 **NRCS velocity (segment) method.** The NRCS segment method is the default for all catchments. Tc = Tt(sheet) + Tt(shallow) + Tt(channel), where:

- (a) **Sheet flow:**

  > Tt = 0.091 (n · L)^0.8 / (P2^0.5 · S^0.4)

  where Tt is in hours; n is the sheet-flow roughness from Table 3.4; L is the sheet-flow length (m); P2 is the present-day 50% AEP (2-yr) 24-hour rainfall (mm) from Clause 2.3, without the climate factor; and S is the land slope (m/m).

- (b) **Sheet-flow length.** In urban areas, and for all post-development conditions, the sheet-flow length shall not exceed **30 m**. For pre-development greenfield surfaces, up to 90 m may be used only where survey shows unchannelled planar ground; otherwise 30 m applies.

- (c) **Shallow concentrated flow:** Tt = L / (3,600 V), with V = 4.918 S^0.5 (unpaved) or V = 6.196 S^0.5 (paved), V in m/s, L in m and S in m/m.

- (d) **Channel and pipe flow:** Tt = L / (3,600 V), with V from Manning's equation at bank-full or pipe-full flow.

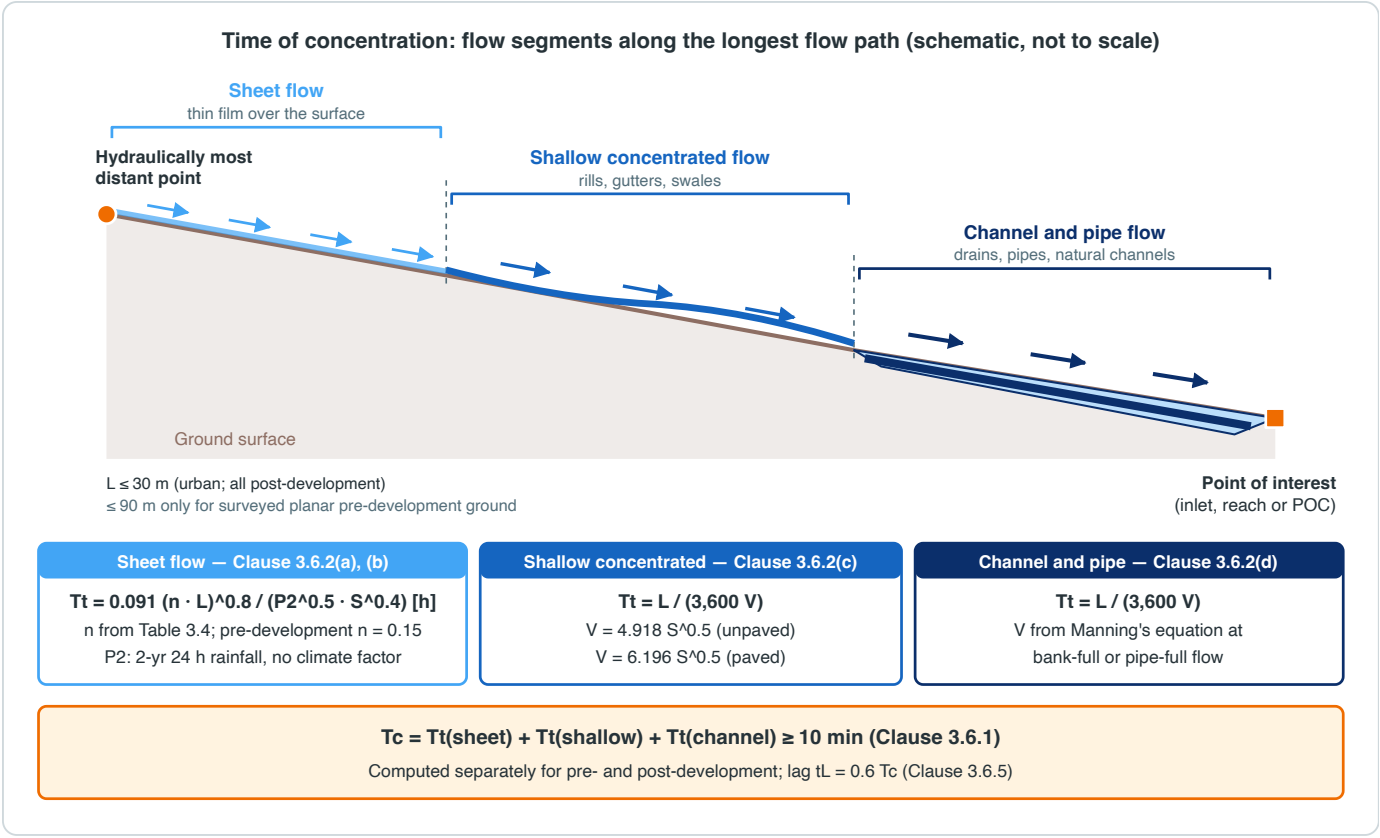

Figure 3.2 — Time of concentration by the NRCS segment method: sheet flow, shallow concentrated flow and channel or pipe flow along the longest flow path, with the travel-time equations and limits of Clause 3.6. Schematic, not to scale. Original drawing for this Manual, adapted from USDA SCS TR-55 (1986), Ch. 3, and USDA NRCS, NEH Part 630, Ch. 15 (2010).

Table 3.4 — Manning's n for sheet flow (USDA SCS TR-55, 1986, Table 3-1)

Surface	n
Smooth surfaces (concrete, asphalt, gravel, bare soil)	0.011
Fallow (no residue)	0.05
Cultivated, residue cover ≤ 20%	0.06
Cultivated, residue cover > 20%	0.17
Grass: short-grass prairie	0.15
Grass: dense grasses	0.24
Range (natural)	0.13
Woods: light underbrush	0.40
Woods: dense underbrush	0.80

3.6.3 **Pre-development roughness.** For the pre-development greenfield surface of Clause 3.3.2, sheet-flow n shall be **0.15** unless the designer justifies another Table 3.4 value from the natural vegetation of the area.

3.6.4 **Kirpich formula.** The Kirpich formula may be used only as a cross-check, or for pre-development rural catchments with a defined main channel:

> tc = 0.0195 L^0.77 S^−0.385

where tc is in minutes, L is the length of the main flow path (m) and S is its average slope (m/m). It shall not be used for urban or post-development catchments.

3.6.5 **Lag.** Where a unit hydrograph method requires lag, tL = 0.6 Tc.

3.6.6 **Check.** Post-development Tc shall normally be shorter than pre-development Tc. Where it is longer, the designer shall explain why in the submittal.

> Commentary. The segment method and its equations are from TR-55 (USDA SCS, 1986, Ch. 3) and NEH Part 630, Ch. 15 (USDA NRCS, 2010). The SI constants are direct conversions of the US equations: 0.007 becomes 0.091 with L in m and P2 in mm, and 16.1345 and 20.3282 ft/s become 4.918 and 6.196 m/s. TR-55 (1986) allowed sheet flow up to 300 ft (≈ 90 m). NEH 630 Ch. 15 states that sheet flow typically lasts no more than about 100 ft (≈ 30 m) before becoming shallow concentrated flow (citing Merkel, 2001), and many US agencies now cap it there. This Manual sets the 30 m urban limit in 3.6.2(b) on that basis: in Indian urban plots, flow reaches a drain, kerb or compound-wall weep hole within a few metres. The undisturbed P2 is used in the sheet-flow equation because P2 is a parameter of the travel-time relationship, not a design load. The default n = 0.15 for greenfield is the TR-55 short-grass value; this Manual adopts it to avoid long pre-development Tc values that would make Q_pre artificially low. The Kirpich formula is the SI form given by Chow, Maidment and Mays (1988, Table 15.1.2), from small agricultural catchments in Tennessee (Kirpich, 1940), so it does not suit urban surfaces. The lag ratio 0.6 is the NRCS value, also used by APWA (2011, §5602.7C).

3.7 Soils and hydrologic soil groups

3.7.1 **Mapping.** HSG shall be assigned from the soil series mapped by the National Bureau of Soil Survey and Land Use Planning (NBSS&LUP) or the Chhattisgarh Department of Agriculture (soil survey, with Indira Gandhi Krishi Vishwavidyalaya, Raipur) [TBC], using the soil-series-to-HSG lookup of Annex 3A. The source map and its scale shall be stated.

3.7.2 **Assignment rule.** Where Annex 3A is not yet available for a series, the HSG shall be assigned from the measured saturated hydraulic conductivity and depth to an impermeable layer or water table using the criteria of NEH Part 630, Ch. 7. Until Annex 3A is published, this Manual sets the following interim groups for Chhattisgarh, using the four traditional soil classes of the Chhattisgarh plain: kanhar, the deep black cracking clay (Vertisol) of the lowest flats, HSG D for every design storm, whatever a test on dry ground shows; no infiltration practice founded in kanhar earns retention credit for a storm after 15 July, the date this Manual takes for the closing of its cracks, or on the strength of a test made on dry, cracked ground, and Clause 4.11 bars infiltration practices on kanhar altogether; dorsa, the sandy clay loam to clay loam (Alfisol, often vertic) of the lower slopes, HSG C, and HSG D where it is vertic or is puddled paddy land; matasi, the sandy loam to silt loam (Inceptisol) of the upper slopes, HSG B, and HSG C where it lies less than 50 cm over murram or rock or is puddled paddy land; bhata, the gravelly soil (Entisol) less than 30 cm deep over murram or ferricrete on the crests and ridges, HSG C, and HSG D where rock or ferricrete lies within 50 cm of the surface, and never HSG A or B, whatever its texture; on the Bastar plateau, the deep soils of the uplands and side slopes, HSG B, and the lithic soils of the hills and rock outcrop, HSG D; in the northern hills of Surguja, Koriya and Jashpur, the red and yellow soils and laterite, HSG B or C by texture and depth, and HSG D where rock or a laterite crust lies within 50 cm of the surface; the soils of the Gondwana sandstone and shale of the coalfields, HSG C; the alluvium along the larger rivers, HSG B or C by texture; made ground and urban fill, HSG C or D, the less permeable unless a field test shows otherwise; and overburden dumps and all other mine-disturbed ground (backfilled or reclaimed mine land, compacted mine benches and haul roads), and ash ponds and ash dykes, HSG D for every design storm, whatever their texture, with

no infiltration credit (Clause 4.4). A soil over the Chandi and Charmuria limestone of the Raipur–Durg–Rajnandgaon belt takes the group of the soil, not of the rock; water lost into the karst below it is not counted as a loss in design. A field infiltration test is required on every site larger than the limits of the simplified method of Clause 4.14.1, made as a pit or borehole test at the depth of the proposed facility after pre-wetting; on smaller sites, a soil that has not been tested shall be given the less permeable of the groups listed for it. Clause 3.10.1 applies to all of these groups.

3.7.3 **Site verification.** For every site that relies on retention by infiltration (Chapter 4), the mapped HSG shall be verified by site infiltration tests at the location and depth of each infiltration facility. Where the tests show a lower infiltration capacity than the mapped HSG, the test result governs for both the retention design and the post-development CN. Where they show a higher capacity, the pre-development HSG shall not be changed without the approval of the authority.

3.7.4 **Disturbed and filled ground.** Clause 3.3.3(b) applies.

> Commentary. NBSS&LUP soil maps are typically at 1:250,000 to 1:50,000 scale and classify soils by series and taxonomy, not by HSG. A lookup table is therefore needed (Annex 3A). Site tests matter most on the Chhattisgarh plain, where the four traditional soil classes follow one another down every slope, from bhata on the crests through matasi and dorsa to kanhar in the lowest flats, so that one site can span two or three groups, and where a test on dry, cracked kanhar in May or June shows a rate that the wet soil never reaches; on bhata, where a loamy soil that looks free-draining lies less than 30 cm over murram or ferricrete; in the limestone belt of Raipur, Durg and Rajnandgaon, where a high rate can mean a cavity rather than a permeable soil (Commentary to Clause 4.11); and on the urban fill of Raipur, Bhilai and Korba and the mine ground of the coalfields, where fill, overburden and compacted ground lie side by side on one site. No statewide HSG map of Chhattisgarh, and no plains-wide area shares of its soil classes, were found during research for this Manual [TBC]. The State has three belts: the northern hills of Surguja, Koriya, Jashpur, Surajpur and Balrampur, the Chhattisgarh plain in the centre, and the Bastar plateau in the south (IMD, *Climate of Chhattisgarh*, 2015), and about 87% of it is underlain by hard rock (CGWB, *Dynamic Ground Water Resources of Chhattisgarh 2024*). On the plain, farmers and agronomists divide the soil into four classes that follow the slope. A study of Durg district gives bhata as Entisols of loamy fine sand to silt loam less than 30 cm deep, on slopes of 5–8% and with a bulk density of 1.71–1.85 g/cm³; matasi as Inceptisols of sandy loam to silt loam 30–80 cm deep; dorsa as Alfisols of sandy clay loam to clay loam 80–150 cm deep; and kanhar as Vertisols of clay loam to clay more than 150 cm deep on slopes below 2%; in that district matasi covered about 44% of the area, dorsa about 41%, kanhar 9% and bhata 5% (Mandal et al., 2018, a low-tier journal; its abstract and table differ slightly for dorsa). The four names are not in the Hindi dictionaries consulted and are taken here from agronomy papers; nor do they map one-to-one onto taxonomy: a study near Raipur reportedly classes both dorsa and kanhar pedons as fine smectitic Haplusterts with 29–62% clay (as reported [TBC]). Kanhar is a "heavy soil with clay content of 40 to 50 percent", whose "permeability is generally low" (CGWB, Rajnandgaon district brochure); the shrink–swell soils on the shales of the Chhattisgarh basin carry more than 50% clay (Karthikeyan et al., 2021, as reported). The only measured infiltration rates found are from two fields of "deep and well drained sandy loam soil" on the IGKV farm at Raipur, by double-ring infiltrometer in November 2017: basic infiltration rates of 7.3 and 7.6 mm/h, reached after about three hours, from average rates of 41 and 64 mm/h (Ranjan and Singh, 2018). They are point measurements on a matasi-type upland soil, at the top of the range of HSG B, not on kanhar, urban fill or compacted ground; a model of per-class rates for the plain (Kindo, Agrawal and Tripathi, 2025) was seen only as an abstract [TBC], and the SCS-CN studies of the Kharun, Seonath, Hasdeo and Kelo basins were seen only in snippets and are not used. In the Bastar plateau the soils on granite-gneiss are lithic on the hills and Alfisols and Inceptisols on the side slopes and plains, light textured and shallow (as reported); for the red and yellow soils of the northern hills no hydraulic data were found [TBC]. The Chandi and Charmuria limestones of the Raipur–Durg–Rajnandgaon belt are officially described as "Karstified and Cavernous", with field permeabilities of 4–65 m/day (CGWB, 2024), and their solution cavities along the bedding are "filled with clay material" (CGWB, Rajnandgaon brochure); a high rate in a test pit there may come from a cavity, which is why Chapter 4 asks for a cavity check before an infiltration practice is credited. The only regional measures of losses are the design loss rates of the CWC subzones: in 3(d) a curve that falls from about 3.5 mm/h for a 3-hour storm to 1.5 mm/h for storms of 13 hours or more, with 2.1 mm/h in the worked example for an 8-

hour storm; 5 mm/h on catchments larger than 75 km² and 2 mm/h on smaller ones in 3(g); 2 mm/h in 3(f); 2.5 mm/h in 1(d); and 1 mm/h in 3(c) (Annex 3E). The 3(d) rates are two to five times below the measured basic rate on the matasi-type soil at Raipur, because they are φ-indices fitted to the floods of whole rural catchments in the monsoon, five of them Chhattisgarh plain catchments with paddy (the Kelo, the Lilagar, the Parri, the Borai and the Karwar), when the kanhar and dorsa lowlands are wet and bunded fields stand full; they are not the infiltration capacity of any soil, and they shall not be used to assign an HSG or to size an infiltration practice. This Manual does not adopt one group for the whole State. A single group D would be conservative for peak flows, and is close to the truth on kanhar, bhata over ferricrete and mine ground, but it would deny infiltration retention (Clause 4.4) on the matasi uplands round Raipur and Bhilai, where the measured rates are high and on which much of the cities' growth has taken place. The date of 15 July for kanhar is set by this Manual: the monsoon reaches Raipur by about 16 June and Bilaspur by about 17 June (IMD, Pai et al., 2020) and covers most of the State by about 20 June, and July and August bring nearly 58% of the annual rain (IMD, *Climate of Chhattisgarh*), enough by mid-July in a normal year to wet the clay and close its cracks; no field record of crack closure in the State was found [TBC]. The interim groups of Clause 3.7.2, the 15 July limit for kanhar, the 50 cm depth-to-rock and depth-to-ferricrete rule, the bar on rating bhata HSG A or B, the group D for overburden dumps, mine-disturbed ground and ash ponds, the pre-wetted test and the rule for untested soil on small sites are set by this Manual pending a soil-series lookup for Chhattisgarh. Steep ground matters as much as soil on the Bastar plateau and in the northern hills, where runoff from laterite crust and bare rock reaches a channel within minutes, so post-development times of concentration will often be at the 10-minute minimum of Clause 3.6.1. There the sheet-flow segment over bare rock or crust shall use the smooth-surface n of Table 3.4, and a catchment that runs from a hill flank onto the plain shall be split at the break of slope, with the hill sub-area modelled on its own, so that a long, flat lower reach does not average away the fast response of the hill; these two rules are set by this Manual.

3.8 SCS curve number (SCS-CN) loss method

3.8.1 **Runoff equation.** Runoff depth shall be computed by:

> $Q = (P - Ia)^2 / (P - Ia + S)$ for $P > Ia$; $Q = 0$ otherwise $S = 25{,}400 / CN - 254$

where Q is runoff depth (mm), P is the cumulative rainfall depth (mm), S is potential maximum retention (mm) and Ia is initial abstraction (mm). In hydrograph models the equation is applied to the cumulative rainfall at each time step to obtain incremental rainfall excess.

3.8.2 **Curve numbers.** CN at AMC II shall be taken from Table 3.5, for the HSG of Clause 3.7.

Table 3.5 — Runoff curve numbers, AMC II, Ia = 0.2S (USDA SCS TR-55, 1986, Tables 2-2a and 2-2c)

Cover type	HSG A	HSG B	HSG C	HSG D
Pre-development reference (Clause 3.3.2): open space, good condition (grass > 75%)	39	61	74	80
Open space, fair condition (grass 50–75%)	49	69	79	84
Open space, poor condition (grass < 50%)	68	79	86	89
Impervious: roofs, paved parking, driveways, paved roads with kerbs and drains	98	98	98	98
Paved roads with open ditches (including right-of-way)	83	89	92	93
Gravel roads (including right-of-way)	76	85	89	91

Cover type	HSG A	HSG B	HSG C	HSG D
Dirt roads (including right-of-way)	72	82	87	89
Newly graded areas (pervious, no vegetation)	77	86	91	94
Fallow, bare soil	77	86	91	94
Meadow (continuous grass, protected from grazing)	30	58	71	78
Brush, good condition	30	48	65	73
Woods, good condition	30	55	70	77
Woods, fair condition	36	60	73	79
Woods, poor condition	45	66	77	83
Water surfaces, tank waterspreads, wet detention basins	Runoff = rainfall (no losses)			

3.8.3 **Use of Table 3.5.**

- (a) Post-development CN shall be built up from measured surfaces (Clause 3.9). The TR-55 urban district and residential lot-size rows shall not be used for the site.
- (b) Pervious areas in the post-development condition shall use "open space" in the condition the landscape plan will achieve. Where no landscape plan is submitted, fair condition applies.
- (c) Cultivated rows of TR-55 may be used for off-site existing conditions, but never for the pre-development reference of Clause 3.3.2.

3.8.4 **Antecedent moisture.** AMC shall follow Clause 2.11.4: AMC III for every design storm, whatever its duration, from 1 July to 30 September at a site on kanhar or dorsa paddy lowland, at a site that drains to a talab, and on a valley floor; AMC III for every design storm at any time of year in a catchment that drains to a reservoir or barrage pond, to the Hirakud backwater, to a mine void, or to a river reach whose CWC site stands at or above its warning level; at any other site or time, AMC III for a design storm that follows, or contains, a 5-day rainfall total of more than 53 mm; and AMC II for every other storm. CN for AMC I and III shall be obtained from AMC II by:

> CN(I) = 4.2 CN(II) / (10 − 0.058 CN(II)) CN(III) = 23 CN(II) / (10 + 0.13 CN(II))

The conversion shall be applied **to the pervious component only**. Impervious surfaces stay at CN 98 for all AMC. Composite CNs shall not be converted directly.

Illustrative — not design values. Pre-development reference CNs at AMC III are: HSG A 59.5; HSG B 78.2; HSG C 86.7; HSG D 90.2.

3.8.5 **Initial abstraction.**

- (a) **Default:** Ia = 0.2S, with the CNs of Table 3.5.
- (b) **Sensitivity run (mandatory):** every storage design shall also be run with Ia = 0.05S. For this run, S shall be converted as S0.05 = 1.33 · S0.20^1.15 (S in inches), which in SI is:

  > S0.05 = 25.4 × 1.33 × (S0.20 / 25.4)^1.15 (mm)

- (c) The design shall be based on the default run. The sensitivity results shall be reported on Form 3-2. Where the sensitivity run increases the required retention volume or water quality volume, the larger value governs for that volume.

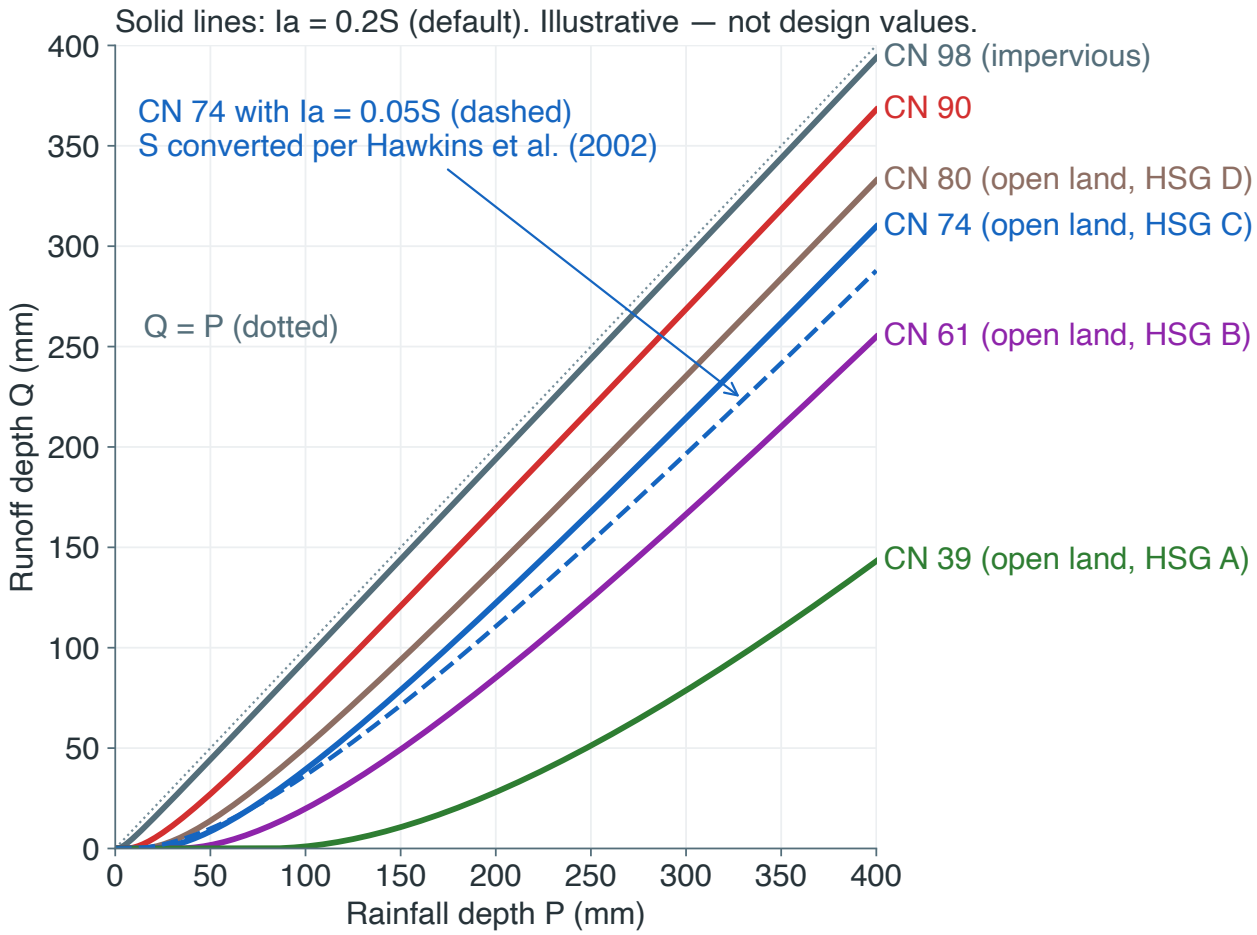

Figure 3.3 — SCS-CN runoff depth versus rainfall depth for selected curve numbers at Ia = 0.2S, with the Ia = 0.05S sensitivity case for CN 74. Original drawing for this Manual, computed from the runoff equation of USDA NRCS, NEH Part 630, Ch. 10 (2004) and USDA SCS TR-55 (1986), and the S-conversion of Hawkins et al. (2002). Illustrative — not design values.

Commentary. Table 3.5 reproduces standard NRCS TR-55 (1986) values. They were derived from US catchments and have not been calibrated in Chhattisgarh. The AMC conversion equations are the standard NEH-4 relationships as given by Chow, Maidment and Mays (1988, §5.5). Converting only the pervious component follows APWA, which states that "direct conversion of composite antecedent moisture conditions is not appropriate" (KC Metro APWA 5600, 2011, §5602.2C). Converting a composite CN would wrongly lower the CN of the impervious part.

Ia = 0.2S is the original NRCS ratio. Analysis of US rainfall–runoff data found that 0.05S fits better on average (Hawkins et al., 2002; Woodward et al., 2003), and gave the conversion of 3.8.5(b). NRCS has not formally replaced 0.2S, so this Manual keeps it as the default. Figure 3.3 shows why the sensitivity run is needed: for CN 74, the 0.05S case gives **more** runoff for small storms (about 2.0 mm vs 0.5 mm from 25 mm of rain) but **less** for large storms (about 242 mm vs 262 mm from 350 mm). *Illustrative — not design values.* The ratio therefore matters most for retention and water quality volumes, and the default 0.2S is the conservative choice for peak

and detention design. The rule in 3.8.5(c) that the larger retention or water-quality volume governs has no direct precedent; this Manual sets it because the 0.05S case gives more runoff from the small storms that set those volumes.

3.9 Impervious connectivity

3.9.1 **Classification.** Every impervious surface in the post-development condition shall be classified as directly connected (DCIA) or disconnected, and shown on the drainage plan.

3.9.2 **Preferred representation.** In hydrograph models, DCIA and pervious areas shall be modelled as separate sub-areas (or as the separate impervious and pervious fractions of a SWMM subcatchment), each with its own loss parameters. The runoff from disconnected impervious areas shall be routed onto the receiving pervious area.

3.9.3 **Disconnection criteria.** An impervious area may be classed as disconnected only if all of the following hold:

- (a) its runoff discharges as sheet flow onto a pervious area, without a pipe, channel or kerb;
- (b) the pervious flow path is at least as long as the impervious flow path draining to it;
- (c) the pervious area is not compacted, not HSG D, and is protected from paving by the approved plan; and
- (d) for storms of 12 hours or more, it does not drain to an RWH sump or tank (Clause 3.3.3(c)).

3.9.4 **Composite CN.** Where a single composite CN is used instead of 3.9.2, it shall be computed as:

- for directly connected impervious area: $CNc = CNp + (Pimp / 100)(98 - CNp)$;
- where total impervious area is below 30% and part of it is disconnected: $CNc = CNp + (Pimp / 100)(98 - CNp)(1 - 0.5\,R)$,

where CNp is the pervious CN, Pimp is the percentage of impervious area and R is the ratio of disconnected to total impervious area. The composite CN shall not be used for retention or water quality volumes.

> **Commentary.** The composite equations are from TR-55 (USDA SCS, 1986, Ch. 2, Figures 2-3 and 2-4). This Manual prefers separate representation in 3.9.2 because a composite CN spreads the impervious response over the whole area, which suppresses runoff from small storms. For a site that is 50% directly connected impervious on HSG B open space (CNp 61), 25 mm of rain gives about 9.9 mm of runoff when the two parts are modelled separately, but only about 1.8 mm with the composite CN 79.5. At 200 mm the two methods agree (139.5 mm vs 138.4 mm). *Illustrative — not design values.* The error is largest for the frequent storms that set retention and water-quality volumes. The disconnection criteria of 3.9.3 are set by this Manual, drawing on common practice in US runoff-reduction manuals. Criterion (d) follows the rule of Clause 3.3.3(c) that RWH sumps are full at the start of SW monsoon storms.

3.10 Groundwater, high water table and tank backwater

3.10.1 **High water table (dual HSG).** Where the seasonal high groundwater table is within 0.6 m of the ground surface, the soil shall be assigned HSG D for both the pre- and post-development runs, whatever its texture, unless it is permanently drained by works that are maintained under Chapter 10. This rule applies in every case to storms of 12 hours or more, which are run at AMC III (Clause 2.11.4).

3.10.2 **Water-table data.** The seasonal high water table shall be taken as the highest level recorded in the SW monsoon season (June–September), from site piezometers or trial pits, or from the nearest Central Ground Water Board (CGWB) or Water Resources Department observation well, adjusted for ground level.

3.10.3 **Infiltration-based losses.** Where Green–Ampt, Horton or soil-moisture-accounting losses are used instead of SCS-CN, the cumulative infiltration in any storm shall not exceed the storage available in the soil above the seasonal high water table.

3.10.4 **Tank and waterbody backwater.** Where a site or sub-catchment drains to a tank, talab, marsh or other waterbody:

- (a) the water level in the receiving waterbody shall be applied as the downstream boundary of the model, per Chapter 5 and Chapter 7;
- (b) land below the level of the receiving waterbody in the design event (including the tank foreshore below FTL) shall be modelled as water surface or storage, with no losses; and
- (c) where the outfall is submerged by the waterbody level, the routing shall use the dynamic-wave method of Clause 3.12.4.

> Commentary. The dual-group rule follows NEH Part 630, Ch. 7 (USDA NRCS, 2009), which places wet soils with a water table within 60 cm (24 in) of the surface in group D in their undrained state, with a dual group (A/D, B/D, C/D) where drainage is feasible. This Manual applies the undrained group to long storms because the water table of the Chhattisgarh plain rises close to the surface in the monsoon. In the 2023 assessment, 52% of the State's observation wells stood at 5–10 m below ground before the monsoon, but after it about 76% stood within 5 m, 17.6% of them within 2 m (CGWB, *Dynamic Ground Water Resources of Chhattisgarh 2024*). In Raipur district the depth was 1.69–13.97 m before the monsoon and 0.87–7.05 m after it, and in Durg district 1.31–24.9 m and 0.7–17.93 m (CGWB district brochures, about 2007), while open wells at Nava Raipur stood at 6–10 m (Naya Raipur Development Plan-2031). The wet cases are: the kanhar and dorsa paddy lowlands, whose clay is slow to drain and whose bunded fields stand under water through the season; the floodplains of the Kharun, the Seonath, the Arpa, the Hasdeo, the Kelo, the Mahanadi and the Indravati; the margins of the talabs of Raipur, of the lakes of Nava Raipur and of the reservoirs and barrage ponds; and the land of Raigarh and Sarangarh-Bilaigarh reached by the backwater of the Hirakud reservoir in Odisha, which partly submerges 33 villages of Chhattisgarh (as reported). In the Gondwana coalfields groundwater "sometimes, occurs under flowing conditions" (CGWB, 2024), while round the Korba mines dewatering has drawn the water table down over about 1.2 km from the Kusmunda and Gevra pits (Singh et al., 2023, as reported), so one coalfield site can be wet and the next dry. No monsoon water-table depths for individual sites, and no map of the seasonal high water table of the lowlands or the floodplains, were obtained [TBC]; Annex 3A is to supply them. Clause 3.10.4 reflects the State's receiving waters, which are talabs, lakes, reservoirs, barrage ponds and rivers held up by dams. Raipur drains to its talabs and to the Kharun, whose highest level at Pathardihi is 281.68 m (29 August 2020), raised from 279.21 m in 1994; no warning or danger level is published for it (CWC, as transcribed by SANDRP). Bilaspur drains to the Arpa below the Bhaisajhar barrage (full reservoir level 302.000 m, WRD), Korba to the Hasdeo below the Darri barrage and Minimata Bango, and Jagdalpur to the Indravati, the only river in the State with CWC warning and danger levels found, 539.49 m and 540.80 m, whose highest level of 544.68 m (9 July 1973) was raised to 544.891 m on 26 September 2026 (CWC, as reported). A press report that the Indravati stood at "12.56 m", 4.26 m above its danger mark, on 26 September 2026 refers to a local gauge, and the two systems shall not be mixed (Chapter 5).

3.11 Unit hydrographs and runoff transformation

3.11.1 **Permitted transforms.**

- (a) SCS (NRCS) dimensionless unit hydrograph, curvilinear or triangular, for catchments below 25 km² and for any sub-area of a larger model;
- (b) the SWMM non-linear reservoir with kinematic overland flow;
- (c) Clark or ModClark unit hydrograph in HEC-HMS, where parameters are calibrated under Clause 3.15; and
- (d) the CWC synthetic unit hydrograph, for 25–5,000 km² (Clause 3.11.4).

3.11.2 **SCS unit hydrograph.** The peak and time to peak of the unit hydrograph shall be:

> qp = PRF' · A · Q / tp, with PRF' = 0.208 (SI form of the standard peak rate factor 484) tp = Δt / 2 + tL, tL = 0.6 Tc

where qp is the peak flow (m³/s); A is the area (km²); Q is the runoff depth (mm), equal to 1 mm for the unit hydrograph; tp is the time to peak (h); and Δt is the computation interval (h).

3.11.3 **Computation interval and sub-areas.**

- (a) Δt shall not exceed 0.133 Tc of the smallest sub-area, nor the hyetograph time step of Clause 2.9.1.
- (b) Sub-areas shall be reasonably uniform in HSG, land cover and slope. Pervious and directly connected impervious areas shall be separate sub-areas, per Clause 3.9.2.

3.11.4 **CWC synthetic unit hydrograph.** For catchments of 25–5,000 km² in CWC subzone 3(d) (Mahanadi), 3(g) (Indravati), 3(f) (Lower Godavari), 1(d) (Sone) or 3(c) (Upper Narmada and Tapi):

- (a) the SUH parameters, design storm duration, loss rate, base flow, areal reduction and time distribution shall be taken from the applicable CWC Flood Estimation Report, using the catchment's physiographic parameters measured from survey or a DEM;
- (b) the 24-hour design rainfall shall follow Clause 2.3.3(b) and Clause 2.7.3, and the climate factor of Clause 2.8 shall be applied;
- (c) the SUH result shall be reported alongside the HEC-HMS or SWMM result for the existing natural condition; and
- (d) where the hydrograph model gives a lower 1% AEP peak than the CWC SUH, the designer shall either justify the model by calibration under Clause 3.15 or adopt the CWC SUH peak.

3.11.5 **Peak rate factor for flat coastal land.** The standard factor (484; PRF' = 0.208) is the default. A lower factor may be used only:

- (a) for unchannelled, flat coastal-plain land without piped or lined drainage;
- (b) in the same way in both the pre- and post-development runs for any land that stays unchannelled; and
- (c) after the authority adopts calibrated values in Annex 3D.

Land drained by pipes, lined drains or kerbs in the post-development condition shall use the standard factor.

> Commentary. The SCS unit hydrograph, the 0.6 lag ratio and the 0.133 Tc interval are from NEH Part 630, Ch. 16 (USDA NRCS, 2007). The interval follows from the unit hydrograph geometry: with tp = Δt/2 + 0.6 Tc and Tc + Δt = 1.7 tp, Δt = 0.133 Tc. The SI constant 0.208 is the direct conversion of 484 (cfs, mi², in, h). APWA uses the SCS dimensionless unit hydrograph as its basis (2011, §5602.5). The CWC Flood Estimation Reports give regression equations for SUH parameters, a design storm duration, a constant loss rate and ARF and time-distribution curves for each subzone (CWC 1987, 1993, 1995, 1997 and 2002; see Chapter 2). They were derived from natural, mostly rural catchments with data that predate the reports of 1987 to 2002 (the five Chhattisgarh catchments behind the subzone 3(d) relationships were gauged between 1959 and 1994), so they represent pre-development conditions, not urbanised ones. That is why the SUH is used as a benchmark rather than as the post-development model. Clause 3.11.4(d) mirrors the floor principle of Clause 2.3.3. HEC-HMS has been calibrated against CWC subzone SUH responses in Indian studies, which shows the two can be linked.
>
> The lower-factor option of 3.11.5 is based on the NRCS Delmarva unit hydrograph, which uses a peak rate factor of 284 (PRF' ≈ 0.122) for flat, poorly channelled coastal-plain catchments on the Delmarva Peninsula (Welle et al., 1980, as adopted by NRCS; New Jersey Department of Agriculture, Delmarva bulletin, which uses 285). A lower

factor gives a lower, later peak. Used only in the pre-development run, it would lower Q_pre and make the release cap stricter; used in a piped post-development run, it would understate Q_post. Clause 3.11.5(b) prevents both biases.

3.12 Routing

3.12.1 **Storage routing.** Routing through detention and retention facilities, tanks, ponds and any storage behind embankments shall use level-pool (modified Puls, storage-indication) routing with:

- (a) a stage–storage relationship from the design grading or survey;
- (b) a stage–discharge relationship for every outlet (orifices, weirs, surplus weirs, pumps), including the minimum orifice and trash screen of Chapter 4; and
- (c) the tailwater boundary of Chapter 5, where the tailwater affects the outlet rating.

3.12.2 **Channel routing.** Routing through channels and drains shall use the Muskingum–Cunge method, or the Muskingum method with parameters K and X that are derived from channel geometry or calibration. For the Muskingum method, the routing interval Δt shall satisfy $2KX \le \Delta t \le 2K(1 - X)$.

3.12.3 **Incidental storage.** Storage behind road embankments, culverts or boundary walls that is not part of an approved facility shall not be counted in the design unless the authority is satisfied that it will be maintained for the design life.

3.12.4 **Dynamic-wave routing.** Dynamic-wave (full Saint-Venant) routing, in SWMM or HEC-RAS unsteady flow, shall be used where:

- (a) the outfall is under tidal, river or tank backwater (Chapter 5, Clause 3.10.4);
- (b) flow can reverse, or the network has loops or overflow links; or
- (c) the network surcharges in the design event and the hydraulic grade line sets the design.

3.12.5 **Tank cascades.** Tanks in a cascade shall each be routed by Clause 3.12.1, with the surplus weir and sluice ratings and the documented operating rule of Chapter 7. The starting water level in each tank shall be FTL unless continuous simulation (Clause 3.13) supports a lower level.

Commentary. Modified Puls routing for storage and Muskingum–Cunge for channels are the APWA requirements (KC Metro APWA 5600, 2011, §5602.8). APWA also excludes "incidental detention" behind embankments unless it will be maintained, which 3.12.3 adopts. CPHEEO's detention procedure uses the same three inputs as 3.12.1: an inflow hydrograph, a stage–storage curve and a stage–discharge curve (CPHEEO 2019, Part A §8.5.3). The Muskingum stability limits are standard (Chow, Maidment and Mays, 1988, §8.4). This Manual starts talabs, lakes and reservoirs at FTL in 3.12.5 because Chhattisgarh's heaviest rain falls in the middle and late monsoon, on waterbodies that have taken the earlier rains and are not drawn down for drainage. About 90% of the State's annual rain falls in June to September, and July and August bring "nearly 58%" of it; of the 100 storms and depressions that affected the State in 1891–2014, 27 came in July, 19 in August, 8 in September and 21 in October (IMD, *Climate of Chhattisgarh*, 2015). The stations' largest 24-hour falls came in every monsoon month and in October: 297.0 mm at Raipur on 30 June 2007, 275.2 mm at Raipur on 14 July 2009, 370.3 mm at Raipur (Obsy) on 4 August 1910, the city's record, 392.0 mm at Rajnandgaon on 21 September 1965 and 228.1 mm at Pendra Road on 8 October 1994 (IMD). Raipur's talabs, about 40 of them over 69.239 ha, are reported to be "polluted and heavily silted" (as reported), and no talab in the city has a published operating rule or a record of water levels [TBC]. Nava Raipur Atal Nagar was planned round a network of natural reservoirs, 55 of them by one account, among them the Sendh, Jhanjh and Khandawa lakes, fed by channels and parks that carry storm water to them, and its development plan asks that rain water from the storm drainage system be stored "into dug up or natural lakes/ depressions" (Naya Raipur Development Plan-2031, §10.3.2; the count is as reported); a lake built to store storm water is full when the next storm comes. The reservoirs and barrages of WRD round the cities, such

as Kumhari (full reservoir level 289.170 m) and Pendrawan (288.110 m) in Raipur district and the Arpa Bhaisajhar barrage (302.000 m) above Bilaspur, are storage works whose levels WRD reports against their full reservoir levels (WRD, daily tank gauge report of 10 October 2024), and Ravishankar Sagar (Gangrel) stood at 348.68 m, 99.49% full, when it released about 55,000 cusecs in early October 2025 (as reported). No talab, lake or reservoir in the State's cities was found with a published operating rule for flood drawdown [TBC]. Talabs, lakes and reservoirs should therefore be assumed full from July, a gated reservoir or barrage should be modelled with its gate rule from WRD, and a waterbody should be modelled below FTL only where a continuous simulation under Clause 3.13 or a notified operating rule supports it. The window of 1 July to 15 October in Clause 3.13.5(a) is set by this Manual. It starts after the monsoon normally covers the State, "most parts of Chhattisgarh by 20th June" (new normal onset 11–13 June at Jagdalpur, 16 June at Raipur and Pendra Road and 17 June at Bilaspur and Balrampur; IMD, Pai et al., 2020), because a late onset or a dry June, such as the stalled advance of 2026, when the State had 54% less rain than normal from 1 June to 2 July (as reported), would lower the starting level. It ends on 15 October, the normal withdrawal date that IMD's Meteorological Centre at Raipur uses, because the new normal withdrawal falls on 7–9 October at Pendra Road, Bilaspur and Raipur and 12–14 October at Jagdalpur (IMD, Pai et al., 2020), and October storms are frequent: 21 of the 100 storms and depressions came in that month, and Ambikapur recorded 206.0 mm on 1 October 1961 (IMD). Pumps, gates and flap valves stay on standby until 20 October, and until 31 October in the Bastar division (Chapter 10), a little longer than the window, to cover the Bay of Bengal systems that cross the State after withdrawal.

3.13 Continuous simulation

3.13.1 **When required.** Continuous simulation shall be carried out, in addition to the design-storm analysis, for:

- (a) regional facilities draining more than 25 km²; and
- (b) any system that includes a tank cascade whose storage is counted in the design (Chapter 7).

The authority may also require it for any facility where the starting storage level governs the design.

3.13.2 **Rainfall input.**

- (a) The longest available hourly record from IMD or another approved recording gauge representative of the catchment shall be used.
- (b) Where hourly data are shorter than 30 years, the record shall be extended with the IMD 0.25° gridded daily data. The full gridded record (1901 to present) should be used; at least the most recent 30 years shall be. The daily data shall be disaggregated to hourly by a method stated in the submittal and checked against the hourly gauge for the overlapping years. Grid values shall be converted to point values under Clause 2.7.1 where applicable.
- (c) A future-climate run shall scale the record by the climate factors of Clause 2.8.

3.13.3 **Model content.** The model shall include soil-moisture accounting with evaporation (IMD pan evaporation or equivalent), groundwater or base flow, tank storage with operating rules and irrigation withdrawals, and the tailwater series of Chapter 5 (predicted tides plus a surge allowance for coastal outfalls).

3.13.4 **Use of results.** Annual maxima of water level, storage and volume, and the duration of outfall closure, shall be extracted and fitted by frequency analysis. Results shall be compared with the design-storm results. The more severe result at each AEP governs. Peak flows in drains and channels with a response time of less than one day shall be taken from the design-storm analysis where the continuous record was disaggregated from daily data, because the disaggregated hourly peaks are synthetic.

3.13.5 **Two-step approach.** The continuous simulation need not use the full hydraulic model. It may use a simplified hydrologic and storage model, at a daily or hourly step, that represents the soil moisture, groundwater, tank storage, operating rules and tailwater of 3.13.3. Its results shall then be passed to the design-storm runs of the full hydraulic

model as:

- (a) the **starting level of each tank**, taken as the level exceeded on 10% of the days from 1 July to 15 October over the record; and
- (b) the **antecedent soil-moisture condition**, taken from the same days and compared with the AMC rule of Clause 3.8.

The designer shall show that the simplified model reproduces the water balance of the full model for at least one recorded event.

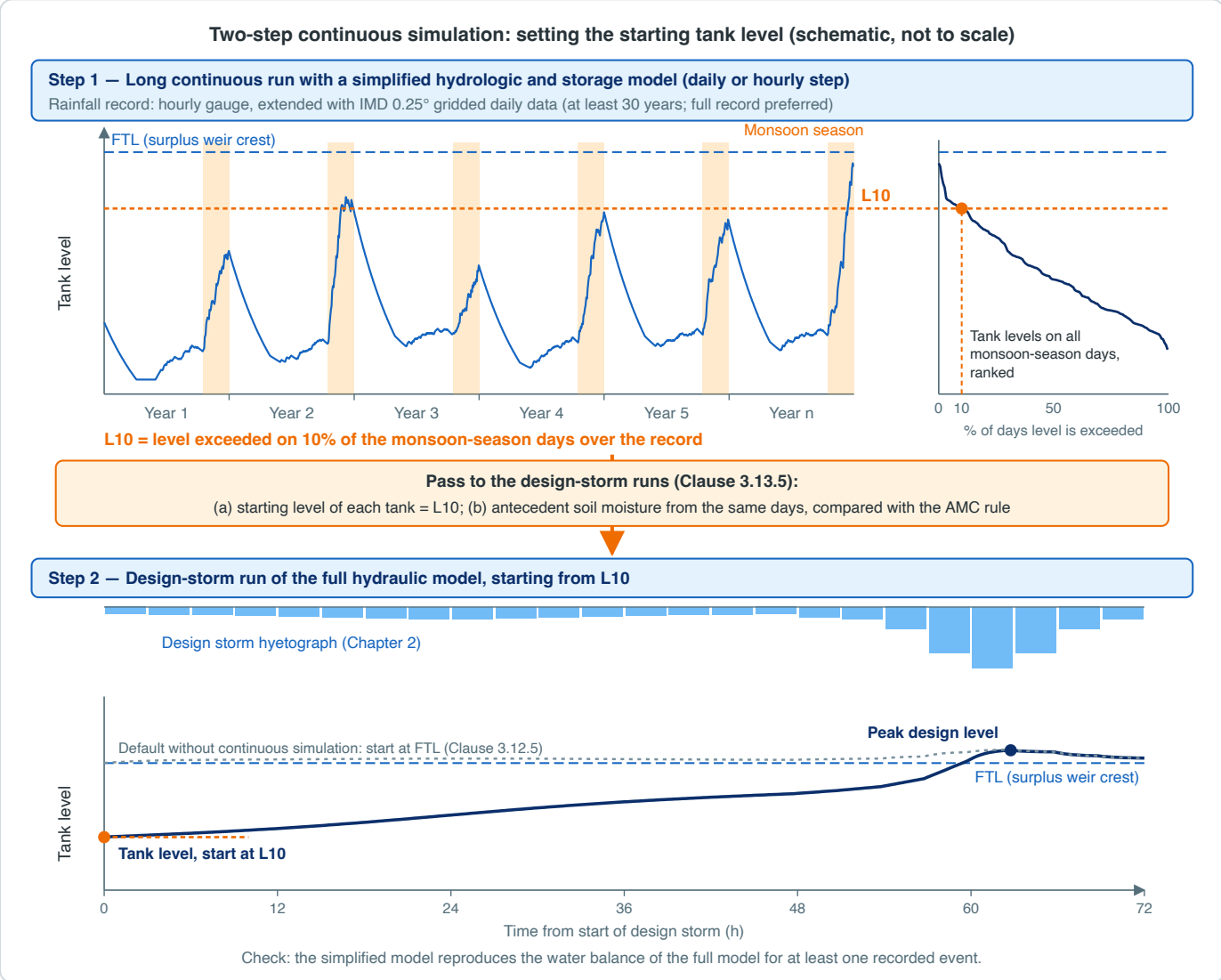

Figure 3.4 — Two-step approach of Clause 3.13.5. Step 1: a long continuous run with a simplified hydrologic and storage model gives the tank level exceeded on 10% of the days from 1 July to 15 October (L10). Step 2: the design storms are run in the full hydraulic model from that starting level, in place of the default start at FTL of Clause 3.12.5. Schematic, not real data. Original drawing for this Manual, illustrating the two-step approach set by this Manual in Clause 3.13.5.

> **Commentary.** Design storms assume one starting condition, but in a tank cascade the governing variable is how full the tanks are when the storm arrives, which depends on the weeks of rain before it. An Australian study of 20 real on-site detention systems found that designs based on one temporal pattern, or even an ensemble of ten, "cannot reliably produce an OSD system design that will always be effective" under real rainfall, and continuous simulation showed frequent-event peaks exceeding pre-development levels (Water Science and Engineering, 2019). The 30-year minimum in 3.13.2(b) matches the station IDF rule of Clause 2.3.2; a longer record is better,

and the IMD gridded daily record, which starts in 1901, is the practical long series for Chhattisgarh. The record length matters less than what the simulation is for. Its main purpose here is to find how full the tanks and soils usually are when a south-west monsoon storm arrives, and how often heavy rain coincides with a closed outfall. A design storm cannot answer either question, because it starts from one assumed condition. It is not intended to replace design storms for the peak flow in urban drains. Clause 3.13.5 therefore allows the long run to be made with a simple storage model, which takes minutes, and passes the starting tank levels to the detailed hydraulic model, which is then run only for the design storms. Running a detailed 1D–2D city model continuously for decades is rarely practical and is not required. The 10% exceedance level in 3.13.5(a) is set by this Manual: it represents a wet but not exceptional monsoon, so that the design storm arrives on tanks that are already well filled, as they were in early October 2025, when Ravishankar Sagar (Gangrel) stood at 348.68 m, 99.49% full, and released about 55,000 cusecs through eight gates, and late September 2026, when all 14 of its gates were opened and the Indravati set a new highest level at Jagdalpur (as reported).

3.14 Pre- vs post-development comparison procedure

3.14.1 The comparison required by Clause 2.11 and Chapter 4 shall be carried out by the following steps. The results shall be recorded on Form 3-2.

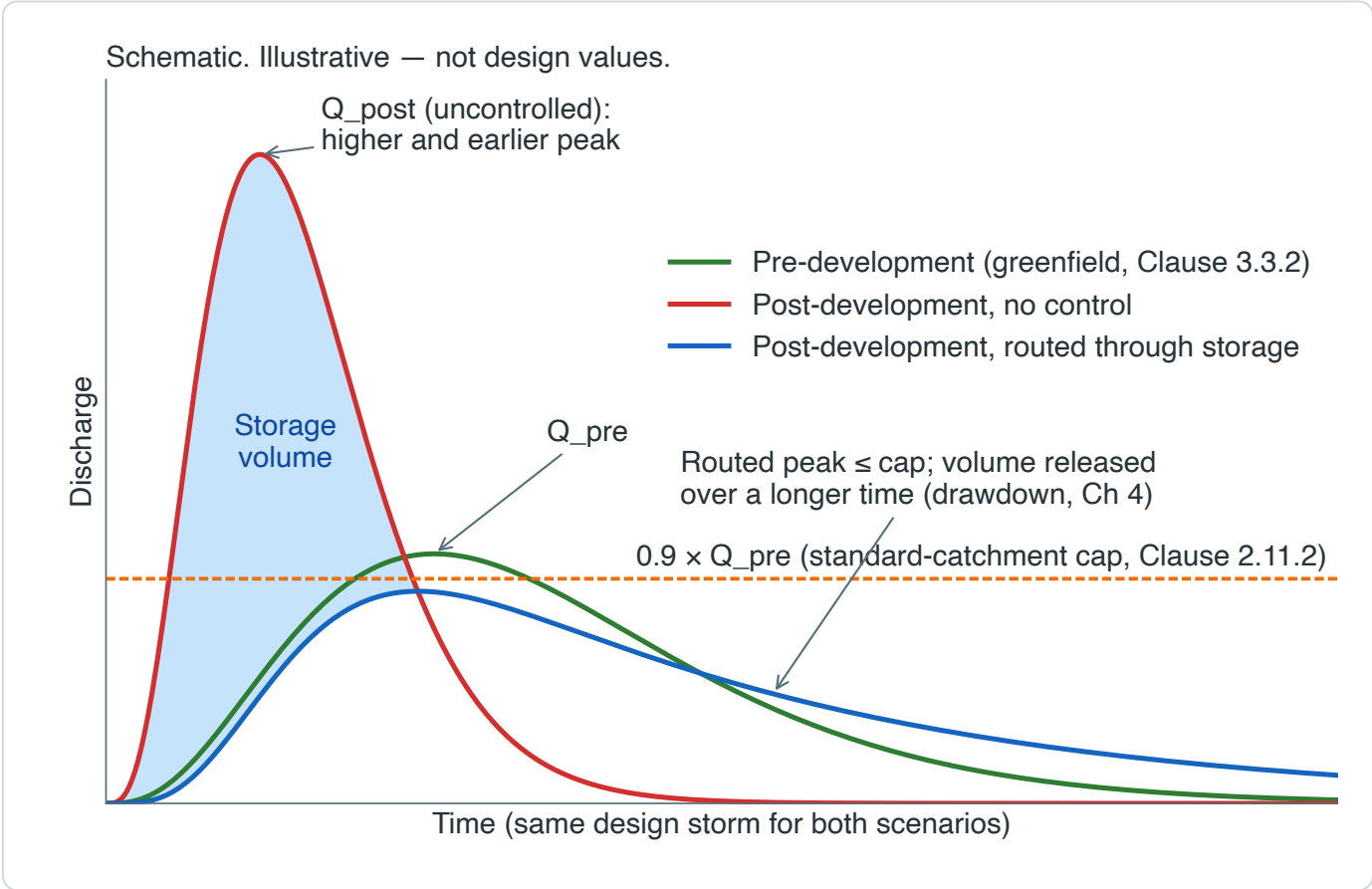

Figure 3.5 — Pre-development, uncontrolled post-development and routed post-development hydrographs for the same design storm. Original drawing for this Manual, adapted from the pre- vs post-development detention concept in CPHEEO, Manual on Storm Water Drainage Systems, Part A §8.5, 2019, and KC Metro APWA Section 5600, 2025.01. Schematic. Illustrative — not design values.

3.14.2 **Step 1 — Define the site, catchments and points of comparison.**

- (a) Delineate the site and every area that drains to it, from survey.

- (b) Identify each POC where runoff leaves the site. Each POC shall be tested separately. Runoff shall not be diverted from one natural catchment to another to meet the criterion.
- (c) Model off-site run-on identically in both runs, at its existing condition (Clause 3.3.5(b)). Where it passes through the site's storage, the criterion applies to the total outflow at the POC.

3.14.3 **Step 2 — Soils.** Map HSG (Clause 3.7), apply the high-water-table rule (Clause 3.10.1) and record the infiltration tests.

3.14.4 **Step 3 — Pre-development model.** Build the pre-development model per Clause 3.3.2: greenfield CN (Table 3.5), Tc (Clause 3.6, with n = 0.15), and existing waterbodies and channels. Compute CN at AMC II and AMC III.

3.14.5 **Step 4 — Post-development model.** Build the post-development model per Clause 3.3.3: measured impervious areas, connectivity (Clause 3.9), disturbed-soil HSG, CN, Tc, and the proposed retention, detention and outlets as storage (Clause 3.12.1).

3.14.6 **Step 5 — Design storms.** From Chapter 2, prepare for each required AEP (50%, 10% and 1%; 2-, 10- and 100-yr) and each duration in {1, 2, 3, 6, 12, 24, 48, 72 h}:

- the depth with the climate factor (Clause 2.8), ARF = 1.0 for the site (Clause 2.7.4);
- the hyetograph (Clause 2.9); and
- the AMC (Clause 2.11.4).

The same storms shall be used in both runs (Clause 2.11.1).

3.14.7 **Step 6 — Run the pre-development model** for every storm and tabulate $Q_{pre}(T, d)$. For flood-sensitive catchments, identify the largest 10% AEP pre-development peak over all durations; that value is the cap of Clause 2.11.3.

3.14.8 **Step 7 — Run the post-development model without controls** and tabulate $Q_{post}(T, d)$ and runoff volumes. This shows the size of the problem and is reported, not tested.

3.14.9 **Step 8 — Add retention and detention.**

- (a) Represent the retention volume of Chapter 4 as storage that is lost to infiltration or reuse (not released to the POC) within the period verified under Chapter 4.
- (b) Route the remaining hydrograph through the detention storage and outlets (Clause 3.12.1).
- (c) Apply the tailwater of Chapter 5, including the joint cases of Clause 2.12, where the outfall is tidal or under backwater.
- (d) Adjust storage and outlets until Step 9 is met.

3.14.10 **Step 9 — Test the criterion** at each POC, for every AEP and every duration:

- (a) standard catchments: $Q_{post}(T) \le 0.9 \times Q_{pre}(T)$ for the 50%, 10% and 1% AEP (Clause 2.11.2);
- (b) flood-sensitive catchments: Q_{post}(1% AEP) ≤ Q_{pre}(10% AEP), and Q_{post}(50% AEP) ≤ Q_{pre}(50% AEP) (Clause 2.11.3).

The governing duration for storage is the one that needs the largest storage or gives the highest water level; the 24, 48 and 72 h runs are always reported (Clause 2.5.2).

3.14.11 **Step 10 — Sensitivity runs.** Repeat Steps 6–9 for the governing storms with $I_a = 0.05S$ (Clause 3.8.5) and, where a lower peak rate factor is used, with the standard factor (Clause 3.11.5). Report the change in peaks and required storage.

3.14.12 **Step 11 — Associated checks.** Using the final routed model, check the drawdown times, minimum orifice and emergency spillway of Chapter 4, the outfall-closed storage check of Chapter 5, and the downstream analysis of Chapter 5.

3.14.13 **Step 12 — Document** the results on Forms 3-1 and 3-2 (Clause 3.17).

> Commentary. The step sequence follows the standard event-based workflow: delineate; set pre-development CN and Tc; set post-development CN and shorter Tc; run identical storms through a unit hydrograph or kinematic model; compare peaks and volumes; and size storage by modified Puls routing. It adds the Chhattisgarh elements: the 48–72 h storms, AMC III, and tailwater. Figure 3.5 shows why the long-duration runs matter: the routed peak can meet the cap while the storage holds the volume difference between the post- and pre-development hydrographs, which must then drain within the Chapter 4 limits. The ban on diverting runoff between catchments in 3.14.2(b) stops a site meeting the rule at one POC by moving flow to a neighbour's drain.

3.15 Model calibration and validation

3.15.1 **When required.** Calibration and validation are required for:

- (a) catchment models of 25 km² or more;
- (b) regional facilities and continuous simulation under Clause 3.13;
- (c) city or basin drainage master plans; and
- (d) any model whose parameters differ from the defaults of this chapter (for example a Clark transform or a lower peak rate factor).

3.15.2 **Data.** Calibration shall use observed rainfall (gauges, and radar where available) and observed flows or water levels (WRD gauging sites, RMC or CGSDMA level sensors, tank gauges, or surveyed flood marks).

3.15.3 **Events.** At least three independent events shall be used for calibration and at least one further event for validation. At least one event shall be a long-duration SW monsoon or cyclone event (for example the Raipur storm of 30 June 2007 (297.0 mm at Raipur and 273.0 mm at Mana (Raipur airport) in 24 hours, IMD), the upper Mahanadi flood of 28–29 August 2020, when the Kharun set its highest level at Pathardihi (as reported), the Bastar floods of 25–28 August 2025, or the statewide flood of 24–27 September 2026, when the Indravati set a new highest level at Jagdalpur (as reported), where data exist).

3.15.4 **Performance.** The submittal shall report, for each event, the observed and modelled peak, volume and time to peak, and for continuous models the Nash–Sutcliffe efficiency (NSE) and percent bias. As a guide, a calibrated model should reproduce event peaks and volumes within ±25%, and a continuous model should achieve NSE ≥ 0.5 and percent bias within ±25%.

3.15.5 **Limits on calibration.** Calibration shall not be used to justify design flows lower than those given by the default methods of this chapter, unless site-specific factors are thoroughly examined and documented and the authority agrees.

3.15.6 **Reasonableness check.** Design peak flows shall be plotted as discharge per unit area against catchment area and compared with the CWC SUH results, regional gauged estimates and known historical floods.

> Commentary. Clauses 3.15.5 and 3.15.6 follow APWA, which states that "model calibration shall not be used to justify discharge estimates that are lower than those provided by the baseline unit hydrograph method" and requires unit discharges to be plotted against tributary area and compared with regression, gauge and historical values (KC Metro APWA 5600, 2011, §5602.9). The event-count and performance values in 3.15.3 and 3.15.4 are set by this Manual. The NSE ≥ 0.5 and ±25% bias thresholds are the "satisfactory" streamflow ratings of Moriasi et al. (2007), which were developed for monthly continuous simulation; their use for event peaks and volumes here is an extension.

3.16 Software acceptance

3.16.1 **Accepted software.** The following are accepted without further demonstration, in a version current at the time of submittal:

- (a) EPA Storm Water Management Model (SWMM 5.x);
- (b) USACE HEC-HMS and HEC-RAS;
- (c) USDA NRCS WinTR-55 and WinTR-20; and
- (d) spreadsheets for the Rational Method and single-area SCS-CN calculations, where every formula and input is shown.

3.16.2 **Other software.** Commercial packages built on the SWMM or HEC engines are accepted if the engine and version are stated. Any other method or software is accepted only if the designer reports, for every sub-area, the peak flow, lag, runoff volume and equivalent CN from both that method and a baseline method of 3.16.1, identifies every difference greater than 5%, and justifies every difference that gives a less conservative result.

3.16.3 **Benchmark problems.** The authority shall publish benchmark test problems (Annex 3C). Software not listed in 3.16.1 shall reproduce them within the stated tolerance.

3.16.4 **Model files.** Native model input and output files shall be submitted with every hydrograph-based design.

> Commentary. The 5% equivalence test is the APWA rule for alternative methods (KC Metro APWA 5600, 2011, §5602.2D). APWA names TR-55, TR-20, HEC-1 and HEC-HMS as baseline methods (§5602.2B). CPHEEO names SWMM for hydrograph and continuous modelling (CPHEEO 2019, Part A §4.4.2). Submitting native model files lets the authority rerun the model and is the main safeguard against unreviewable black-box results.

3.17 Documentation

Every hydrologic submittal shall include:

1. **Form 3-1, Hydrologic Calculation Summary**, stating the method used and why (Table 3.1), the software and version, and the computation interval.
2. **Catchment maps** for the pre- and post-development conditions, showing sub-areas, flow paths used for Tc, POCs, and off-site run-on.
3. **Soils:** the HSG map and its source and scale (Clause 3.7), water-table data (Clause 3.10), and infiltration test results.
4. **Parameter tables** for each sub-area and condition: area, impervious area by type and connectivity, HSG, CN at AMC II and III (or C and Cf), Tc worksheets, and n values.
5. **Rainfall:** the Design Rainfall Sheet (Form 2-1) and the hyetographs used.
6. **Results:** hydrograph tables and plots for each storm; stage–storage and stage–discharge tables; routed outflows; and peak water levels.
7. **Form 3-2, Pre- vs Post-Development Comparison**, with Q_pre, Q_post (uncontrolled and routed) and the criterion for each POC, AEP and duration, and the sensitivity runs of Clause 3.14.11.
8. **Calibration report**, where Clause 3.15 applies.
9. **Model files** (Clause 3.16.4).
10. **A statement by the responsible engineer** that the methods and parameters of this chapter have been used, with any departures listed and justified.

References (chapter)

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Annexes and forms

Annex / Form	Content
3A	Chhattisgarh soil series to HSG lookup; seasonal high water table map
3B	Chhattisgarh CN calibration and supplementary land covers (cracked kanhar fallow, bunded paddy, talab and reservoir beds, bhata and laterite crust, overburden dumps and ash ponds, Bastar and northern forest and scrub)
3C	Benchmark test problems for software acceptance
3D	Peak rate factor study for flat coastal land
3E	CWC SUH parameters for subzones 3(d), 3(g), 3(f), 1(d) and 3(c), with worked example
Form 3-1	Hydrologic Calculation Summary
Form 3-2	Pre- vs Post-Development Comparison

CHAPTER 4

Site Runoff Control: Retention and Detention

4.1 Purpose and scope

4.1.1 This chapter sets the performance standard that every regulated development shall meet for runoff leaving the site. It covers:

- (a) the order in which runoff-control measures are to be applied (Clause 4.3);
- (b) retention of frequent rainfall, and how retained volume is credited against detention (Clause 4.4);
- (c) release-rate criteria for standard and flood-sensitive catchments (Clause 4.5);
- (d) the procedure for sizing detention storage and outlets (Clauses 4.6 to 4.9);
- (e) facility types, rainwater harvesting (RWH) accounting, and high-groundwater sites (Clauses 4.10 to 4.12);
- (f) regional detention and fee-in-lieu (Clause 4.13);
- (g) simplified methods for small sites: the Tier B tables and the fixed-coefficient route (Clause 4.14); and
- (h) public safety (Clause 4.15).

4.1.2 This chapter applies to land disturbance of **0.1 ha or more**, or new or replaced impervious area of **500 m² or more**, as set in Chapter 1. Single dwellings below that threshold, and road resurfacing that adds no impervious area, are exempt. The existing RWH mandate of the building rules (rain or roof water harvesting under rule 78(4) of the CGBVN 1984, as amended on 17 February 2009, on buildings on plots above a threshold reported as 500 m² (the amendment substitutes the figures "500" and "300" in the sub-rule, and the threshold is to be confirmed [TBC]), by the options of Appendix ढ (N), with a penalty of up to ₹1,000 a year for every 100 m² of built-up area where the system is not built and maintained; and, for the buildings of State departments and their undertakings, the General Administration Department order of 31 May 2016; the rules are discussed in the Commentary to Clause 4.11) continues to apply to every building it covers, whether or not this chapter applies.

4.1.3 Rainfall inputs come from Chapter 2. Hydrologic methods come from Chapter 3. Tailwater, downstream analysis and the outfall-closed check come from Chapter 5. Water quality volume (WQv) treatment comes from Chapter 8. Where this chapter and those chapters overlap, the more stringent requirement governs.

> **Commentary.** No Indian national rule or Chhattisgarh rule found during research for this Manual limits the rate of discharge from a site, and none requires on-site detention or "zero run-off". The building rules are the Chhattisgarh Bhumi Vikas Niyam 1984 (CGBVN 1984), the rules of 1984 that the State inherited from Madhya Pradesh in 2000, retitled and amended since by the Housing and Environment Department under section 85(1) read with section 24(3) of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973. Unlike Madhya Pradesh, which replaced its rules in 2012, Chhattisgarh has kept and amended the 1984 rules: among the amendments read are those of 17 December 2008 (No. एफ 7-26/32/2008, integrated townships), 17 February 2009 (No. 318/1193/32/2006, which amends the rainwater harvesting rule, Commentary to Clause 4.11), 6 October 2018 (No. F 7-27/2017/32) and 29 April 2020 (No. F 07-24/2019/32), and a draft of 30 March 2017 (No. F 7-15/2016/32) on green buildings; drafts of 12 September and 1 December 2025 and a reclassification of land uses in October 2026 are reported but were not read [TBC]. No consolidated Chhattisgarh text of the rules was found, so the rules on definitions, site suitability (Rule 50), drains (Rule 51), plinth and basements were not read in their Chhattisgarh form [TBC]. None of the amendments read touches drainage, site levels or water bodies, apart from the forms of 2018. The drainage provisions that can be traced protect the building, the drain and the site, not the catchment. Rule 50 is

still the State's rule on the unsuitability of a site: the T&CP Directorate's report format of 2011 asks whether a site is unsuitable under it, whether the land is a waterlogging area, and whether it lies beside a canal, nala or river and at what distance, and records the area left for nalas (letter No. 1680/TCR-2/नग्रानि./2011 of 23 June 2011). The inherited 1984 text of that rule bars building within 9 m of the highest water mark and, along a major watercourse, within "9 meters from average high flood mark or 15 meters from the defined boundary of water course, whichever is more" (r.50(b)), and on a site that is not properly drained or is incapable of being well drained (r.50(f)), while r.51 requires land within the curtilage to be "effectively drained by surface water drain or other means" if the Authority so requires; whether the Chhattisgarh text keeps these words and distances is to be confirmed [TBC]. The Directorate's checklist of 2011 asks, for land next to a nala, canal or river, for the no-objection of WRD or the Revenue Department and for "the high flood point" from WRD (No. 1569/स्टेनो/नग्रानि./2011), the only flood level found in the State's planning process. The amendment of 2018 added a plinth certificate (Rule 96), whose form checks location, setbacks and deviations but no level, and inspection forms that ask whether a "Nalah (Major Drain)" is adjacent to the plot and how far it is, "in rule" of the CGBVN 1984 with the rule number left blank, whether rainwater harvesting is installed "as per rule", and whether the drainage line is connected "up to final diposal" (Appendix Q). Integrated townships may not be sited on the bank of a perennial river or in an ecologically sensitive area (Appendix ण, 2008), and water sources are left out of their area (draft of 4 October 2018); and no unauthorised construction on the bed or edge of a reservoir or on a natural drain may be regularised (Chhattisgarh regularisation Act of 2002, section 7(1)(चार)). The draft green-building rule of 2017 would have asked for a reduction of hardscape on larger plots and an unpaved area of at least 20% of the recreational open space (draft rule 79-C); its final text was not found [TBC]. The Municipal Corporation Act 1956 and the Municipalities Act 1961, as adapted in Chhattisgarh, were not read in their Chhattisgarh form; their drainage and tank sections are cited from the Madhya Pradesh text of the same Acts (sections 82, 198, 203 and 204 of the 1956 Act), and their numbering in Chhattisgarh is to be confirmed [TBC]. None of these sets a design storm, a release rate, a storage volume or an overflow path, and no drainage design standard of UADD, T&CP or any Municipal Corporation stating a return period was found. The only explicit urban design storm in the State is in a city plan: the Naya Raipur Development Plan-2031 says that "5 years Maximum rainfall intensity should be taken for design of system", asks for river embankments "safe against 100 yrs frequency flood", road levels "at least 0.5 to 1.0 m above the observed high flood level" and road drainage to IRC: SP-50, and gives a chart of rainfall intensity against time of concentration for 2, 5, 10 and 50 years from IMD data for Raipur, with no equation or record length (§10.3, §10.3.1.1 and Chapters 7–9); it binds Nava Raipur only and is not a State standard. Raipur's drainage plan under AMRUT (2015) proposed ₹1,191 crore of works with no return period, intensity or runoff coefficient, and the CAG's audit of AMRUT in Chhattisgarh, for the years to March 2022, covers water supply, sewerage and parks and never mentions drains (Commentary to Clause 4.5). This Manual uses none of these values; its design storms come from Chapter 2. Basements are regulated, but not for flooding. The amendment of 2018 lets a basement be approved after a clear margin of 3 m from the plot boundary for a building up to 18 m high and 6 m above that (Rule 73(3)(क)), and in 2009 the Directorate required its own approval for basements below 10,000 sq ft because basement space was being misused; no Chhattisgarh rule restricts basements in low-lying or flood-prone land, sets a level for a basement ramp or opening, or keeps surface water out of a basement [TBC]. This Manual therefore adds its own requirements for every basement: no detention storage in a habitable basement (Clause 4.10.1); the crest of every basement ramp and the sill of every basement opening at least 0.5 m above the 1% AEP flood level, with basement drainage pumped to the surface drain through a non-return valve (Clauses 6.11.1 and 6.11.4); and no basement in Zone 2 (Clause 7.5.2). In the flood-sensitive catchments of Clause 4.5.3.2, in any catchment notified under Clause 4.5.4 and on the paddy lowlands, floodplains and backwater land of Clause 4.12.1, it recommends that no basement be permitted where the 1% AEP flood level stands above the surrounding ground; these are the Manual's proposals, not rules of the CGBVN 1984. Mine land is a drainage regime of its own. Mine drains are governed by the conditions of each environmental clearance and by DGMS, under whose Coal Mines Regulations 2017 the lowest point at which surface water could enter a mine is to stand at least 1.5 m above the highest flood level (regulation 149, as reported [TBC]); the coal-sector clearances size garland drains with "at least 50% safety margin over and above the peak sudden rainfall and maximum discharge", which is not a return period (standard coal-sector conditions, as read); mine dewatering round Korba draws the water table down over about 1.2 km from the Kusmunda and Gevra pits (as reported); and the ash dykes of the power stations at Korba have breached, at NTPC's Dhanras

dyke in July 2024, at CSPGCL's Korba West dyke on 19 April 2026, with one death, and at BALCO's ash dam, whose slurry reached a nala and the Hasdeo (as reported). For any development on or beside mine land, an overburden dump or an ash pond this Manual therefore requires that: no development drain or overflow path discharges into an active mine; runoff from an overburden dump, an ash pond or dyke or other mine-disturbed ground passes through a sediment basin (Chapter 8) and a stable toe drain at the foot of the dump or dyke before it reaches any urban drain or outfall; overburden dumps, mine-disturbed ground and ash ponds are HSG D with no infiltration credit (Clause 3.7.2); a mine void or pit lake is not used as an outfall or as storage unless the lessee and DGMS agree in writing and its water level, spill level and spill path are surveyed, and an ash pond only with the operator's consent and a check of the dyke (Clause 5.8.5(e)); and the path of an ash-dyke breach is a hazard to be mapped, not a design inflow. These are the Manual's requirements, not conditions of any clearance. This chapter is written so that it can supply, as a technical companion to the CGBVN 1984 and the Municipal Acts, the design storm, release rates and storage volumes that they leave open; it has not been adopted for either purpose. In practice most city nalas in Chhattisgarh carry sewage: in January 2021 the State's pollution control board reported that only Bilaspur had a sewerage system, while Raipur's new treatment plants were to take the flow of its nalas through diversion weirs (CECB, report to the NGT), Raipur's sewage is reported to reach the Kharun "through eight big canals", and the NGT has ordered the plants on Durg's Shankar and Pulgaon nalas finished within six months (16 January 2026, as reported) (Clause 8.9). Retention and detention apply in full on every site whatever the drain carries: every litre a site releases at the peak of a storm takes up capacity in a drain that is already part-filled with sewage. The CPHEEO (2019) manual describes limiting outflow to the pre-development peak as typical practice, but it uses no "shall" and names no storm frequencies (CPHEEO Part A §8.5.1). This chapter turns that description into a development-control requirement.

4.2 Definitions

Term	Meaning in this Manual
Retention	Permanent removal of runoff from the surface drainage system by infiltration, evapotranspiration or reuse, so that the water never reaches the outfall.
Retention depth	The 90th percentile wet-day 24-hour rainfall depth for the site's zone (Clause 2.10, Annex 2E).
Required retention volume (RRV)	The runoff volume from the retention depth over the post-development site (Clause 4.4.2).
Detention	Temporary storage of runoff, released through a controlled outlet at a rate set by Clause 4.5.
Retention credit	The reduction in required detention storage that results from retention practices, determined by routing under Clause 4.4.6. It is not the same as the retained volume.
Permissible site discharge (PSD)	The maximum rate at which a site may discharge for a stated AEP, in L/s or L/s/ha.
Site storage requirement (SSR)	The detention storage required per hectare of site under the simplified method of Clause 4.14.
Multi-stage outlet	An outlet structure with separate openings at different levels, each controlling a different AEP (Figure 4.2).
Drawdown time	Time from the peak stored volume until the stated fraction of that volume has been released.

Term	Meaning in this Manual
Emergency spillway	A secondary overflow that passes flows above the primary outlet capacity, or all flow when the primary outlet is blocked, without overtopping the embankment.
Freeboard	Vertical distance between the maximum design water level and the embankment crest or other critical level.
Flood-sensitive catchment	A catchment listed in Clause 4.5.3 or notified under Clause 4.5.4.
Authority	The approving authority under Chapter 1 (the Director of Town and Country Planning, or an officer authorised by the Director, who grants development permission for land in a planning area under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973, and the Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat that grants building permission under the CGBVN 1984; NRDA in Nava Raipur Atal Nagar, as a Special Area Development Authority under that Adhiniyam; or a development authority constituted under that Adhiniyam, where it grants that permission [TBC], as applicable).
Tier A / Tier B	Adoption tiers defined in Chapter 1. Tier A: Raipur Municipal Corporation (RMC), every approving authority within the planning area of Raipur under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973, including the Directorate of Town and Country Planning where it grants development permission, RDA in its schemes and the Nagar Palika Parishads and Nagar Panchayats in that area; NRDA in Nava Raipur Atal Nagar; every other Municipal Corporation constituted under the Municipal Corporation Act 1956 as adapted in Chhattisgarh, among them those of Bilaspur, Durg, Bhilai, Bhilai-Charoda, Risali, Birgaon, Rajnandgaon, Korba, Raigarh, Jagdalpur, Ambikapur, Dhamtari and Chirmiri; and the Directorate of Town and Country Planning where it grants development permission within the planning area of a Municipal Corporation. Tier B: Nagar Palika Parishads and Nagar Panchayats.

4.3 Order of measures

4.3.1 Every site runoff-control design shall apply measures in the following order, and shall document in the submittal why each earlier step could not do more:

1. **Preserve.** Keep existing waterbodies, talabs, natural drainage lines, wetlands, and permeable soils and vegetation. Avoid fill in flood storage (Chapter 7).
2. **Retain.** Retain the RRV on site by infiltration, reuse or evapotranspiration (Clause 4.4).
3. **Detain.** Detain the remaining runoff so that the release-rate criteria of Clause 4.5 are met for every design storm and duration (Clauses 4.6 to 4.9).
4. **Convey.** Convey the 1% AEP (100-yr) flow safely through the site to the receiving system on a defined overland path (Chapter 6), and check the receiving system downstream (Chapter 5).

4.3.2 Preservation and retention reduce the detention required (Clauses 4.4.6 and 4.4.7). They do not relieve the site of the release-rate criteria of Clause 4.5 or of the downstream analysis of Chapter 5.

> Commentary. This order follows the "layers of stormwater management" adopted in the Kansas City Metro APWA Section 5600 Version 2025.01: preserve or restore, retain, detain, convey, maintain (MARC factsheet; MARC Supplement, Nov 2025). Maintenance is the fifth layer and is set in Chapter 10. The 2025.01 manual credits retention and natural-area preservation against detention size (MARC Supplement, Nov 2025). Clause 4.4.6 adopts that credit, and defines it so that it cannot be claimed twice.

4.4 Retention

4.4.1 Requirement

The site shall retain the RRV. Retention shall be achieved by one or more of the following:

- infiltration practices (soakaways, infiltration trenches, permeable pavement with an infiltration base, bioretention);
- rainwater reuse that is drawn down by a demonstrated non-potable demand;
- green roofs and landscape areas that do not drain to the outfall in the retention-depth storm.

The limits of Clause 4.4.4 apply.

4.4.2 Computation of the RRV

> RRV (m³) = 10 × Q_R × A

where:

- Q_R (mm) is the runoff depth from the retention depth P_R (Annex 2E). It is computed with the SCS-CN method for the post-development cover of each sub-area, at AMC II, with Ia = 0.2S (Chapter 3); and
- A (ha) is the area of the site that drains to the outfall.

Q_R shall be computed for each sub-area separately and summed. An area-weighted CN shall not be used where impervious and pervious areas differ by more than 20 CN points.

The volumetric method RRV = 10 × P_R × Rv × A, with Rv = 0.05 + 0.009 × I (I = percent impervious), may be used for sites under 0.5 ha and under the Tier B method.

> **Commentary.** The Rv formula is the one used in the KC water quality volume (MARC BMP Manual 2012). It is simple to review for small sites. The SCS-CN form is required for larger sites so that the retention computation uses the same runoff model as the detention routing. That keeps the credit computation of Clause 4.4.6 consistent.

4.4.3 Verification

Retention by infiltration shall be supported by:

- (a) site infiltration tests at the location and depth of each practice, at the frequency set in Chapter 8;
- (b) a design infiltration rate of no more than **half** of the measured rate; and
- (c) groundwater evidence showing the seasonal high groundwater table at least **1.0 m** below the base of the practice (Chapter 8).

Retention by reuse shall be supported by a demand calculation showing that the RRV is drawn down within 72 hours by a continuous non-potable use, such as toilet flushing, cooling or irrigation.

> **Commentary.** The factor of 2 on measured infiltration rate has no direct precedent; this Manual sets it to allow for clogging and for the difference between a test pit and a full-scale practice. Chapter 8 may set a different factor; if

it does, Chapter 8 governs.

4.4.4 Limits on retention

4.4.4.1 Infiltration practices shall not be used:

- where the seasonal high groundwater table is less than 1.0 m below the base (Chapter 8);
- on contaminated land;
- within the setbacks from foundations, wells and septic systems set in Chapter 8; or
- on fill, unless tested after placement.

4.4.4.2 Where the RRV cannot be fully retained, the shortfall shall be stated in the submittal. The shortfall shall be treated as WQv under Chapter 8 and shall get no retention credit.

4.4.5 Drawdown of retention

Retention storage shall be emptied by infiltration or reuse within **72 hours** of the end of the retention-depth storm, verified by the tests of Clause 4.4.3.

4.4.6 Retention credit: definition and computation

4.4.6.1 **What the credit is.** The retention credit is the reduction in required detention storage that results when the retention practices are included in the post-development routing. It is computed. It is not assumed equal to the RRV.

4.4.6.2 **Tier A method (routing).** For each design storm and duration of Clause 4.6.3:

- (a) model each retention practice explicitly as a storage that is **empty at the start of the storm**, unless Clause 4.11 says otherwise;
- (b) let the practice fill from the front of the storm, and let it lose water during the storm only at the design infiltration or reuse rate of Clause 4.4.3;
- (c) once it is full, route its overflow to the detention facility; and
- (d) size the detention for the residual hydrograph.

The required detention storage is the largest storage over all storms and durations. The credit is the difference between that storage and the storage required with no retention. No further deduction shall be made.

4.4.6.3 **Tier B method (simplified).** The detention storage required by Clause 4.14 may be reduced by:

> $V_{credit} = C_R \times RRV_{provided}$

where C_R is the retention credit factor from Annex 4B for the zone, soil group and drawdown rate. Until Annex 4B is published, **$C_R = 0$**.

4.4.6.4 Retention that is not verified under Clause 4.4.3 gets no credit.

> Commentary. "Credited against detention" (APWA 5600 2025.01) can be read in two ways. One reading subtracts the retained volume from the detention volume, cubic metre for cubic metre. The other routes the storms with retention in place and sees what detention is still needed. The two readings give very different answers in

Chhattisgarh, because the mandatory 48- and 72-hour storms (Clause 2.5.2) fill any retention practice on the first day, long before the peak arrives on the last day (Clause 2.9.2). Unless the practice keeps infiltrating during the storm, it is full at the peak and does nothing to reduce the peak storage.

The illustrative example in Clause 4.6.7 shows the size of the effect. A 2 ha site with 552 m³ of retention (no infiltration during the storm) needs 138 m³ less detention for the 24-hour storm. That is a credit of about a quarter of the retained volume. For the 48- and 72-hour storms, which govern, the credit is close to zero. Subtracting the retained volume directly would have under-sized the detention by about 545 m³ (about 19%).

This Manual therefore defines the credit as the result of routing. Retention on fast-draining sandy soils, which keeps infiltrating through a multi-day storm, will earn a real credit. Retention that only fills will earn little. Both outcomes are physically correct. Setting C_R = 0 until Annex 4B exists is a safe default for Tier B, not a judgement that retention has no value: retention is still required for water quality and recharge (Chapter 8).

4.4.7 Preservation credit

Areas of the site that are permanently preserved in natural cover may be modelled at their pre-development CN in the post-development condition, provided that:

- they are protected by a registered covenant or dedication; and
- their runoff is not concentrated onto impervious surfaces.

This lowers both the RRV and the detention required. No other preservation credit applies.

4.5 Release-rate criteria

4.5.1 General

4.5.1.1 Peak discharge from the site shall be computed by a hydrograph method (Chapter 3) for the pre-development and post-development conditions with identical design storms (Clause 2.11.1). The Rational Method shall not be used for any part of storage sizing.

4.5.1.2 Pre-development is greenfield: open land in good condition for the mapped HSG. Existing impervious cover is not counted (Clause 2.11.5, Chapter 3). This applies to redevelopment as well, subject only to the constrained-redevelopment route of Clause 4.5.6.

4.5.1.3 The criteria apply at each point where runoff leaves the site. Where a site drains to more than one outfall, the criteria apply to each outfall for the area draining to it. Runoff shall not be diverted between catchments.

4.5.2 Standard catchments

For each of the 50%, 10% and 1% AEP (2-, 10- and 100-yr) storms, and for every duration in {1, 2, 3, 6, 12, 24, 48, 72 h}:

$$Q_{post}(T, d) \le 0.9 \times Q_{pre}(T, d)$$

4.5.3 Flood-sensitive catchments

4.5.3.1 In the catchments listed below, the **mixed-frequency rule** of Clause 2.11.3 applies, for every duration in Clause 2.5.2:

> Q_post(1% AEP, 100-yr) ≤ Q_pre(10% AEP, 10-yr), and Q_post(50% AEP, 2-yr) ≤ Q_pre(50% AEP, 2-yr).

Each cap is the pre-development peak at the duration that maximises it: the 10-yr peak for the 100-yr check, and the 2-yr peak for the 2-yr check. The cap applies to every post-development duration (Clause 2.11.3).

4.5.3.2 The flood-sensitive catchments are:

- (a) **Raipur's low colonies on the Kharun nalas**: the catchments of the nalas and drains of Raipur and Birgaon that discharge to the Kharun, with the talabs on them, and the low-lying colonies of the city that drain to them and flood in heavy rain, whose outfalls are held up when the Kharun is high;
- (b) **Bilaspur on the Arpa**: the land of Bilaspur that drains to the Arpa below the Bhaisajhar barrage, including Sarkanda and the other low colonies of the river's banks;
- (c) **Durg–Bhilai on the Seonath nalas**: the land of Durg, Bhilai, Bhilai-Charoda and Risali that drains to the Seonath (Shivnath) through the Shankar and Pulgaon nalas and the other nalas of the twin cities;
- (d) **Korba below Bango and Darri**: the land of Korba that drains to the Hasdeo below the Minimata Bango dam and the Hasdeo barrage at Darri, whose outfalls are held up when the dams release;
- (e) **Raigarh–Sarangarh in the Hirakud backwater**: the land of Raigarh and Sarangarh that drains to the Kelo, the Mand or the Mahanadi within reach of the backwater of the Hirakud reservoir;
- (f) **Jagdalpur on the Indravati**: the land of Jagdalpur that drains to the Indravati, whose outfalls are held up when the river is above its CWC warning level; and
- (g) any catchment notified under Clause 4.5.4.

The boundaries shall be those mapped in Annex 4A.

4.5.4 Notification of further flood-sensitive catchments

4.5.4.1 The Authority may propose that a catchment be notified as flood-sensitive where one or more of the following is documented:

- (a) buildings or arterial roads flooded in an event of 1% AEP or more frequent, on at least two occasions in the previous 20 years;
- (b) receiving drains or channels with capacity below the 10% AEP flow under existing conditions;
- (c) an outfall to a talab, lake, reservoir or barrage pond held at or above its full level, such as the talabs of Raipur and the lakes of Nava Raipur, or an outfall that is held up by a river in flood or by releases from dams and barrages upstream, such as the Kharun at Raipur, the Arpa below the Bhaisajhar barrage at Bilaspur, the Hasdeo below Bango and Darri at Korba, the Mahanadi below Gangrel, the Indravati at Jagdalpur or the Hirakud backwater at Raigarh and Sarangarh, for part of the SW monsoon (Chapter 5);
- (d) loss of more than a quarter of the mapped tank and talab storage in the catchment since the earliest available survey; or
- (e) a basin master plan that relies on limiting site discharge.

4.5.4.2 The procedure shall be:

1. The Authority prepares a technical note with the catchment boundary in GIS form and the evidence for 4.5.4.1.
2. The State-level technical committee constituted under Chapter 1 reviews the note.
3. The draft notification and map are published for public comment for at least 60 days.
4. The Government notifies the catchment and the map is added to Annex 4A.

5. The rule applies to applications received **6 months** or more after notification. Applications received earlier are assessed under Clause 4.5.2.

4.5.4.3 A notified catchment may be de-notified only by the same procedure, and only after a basin study shows that the downstream system meets Chapter 5 with all development under Clause 4.5.2.

Commentary. The 0.9 factor is derived from the Denver (MHFD) analysis. In that analysis, ten basins each releasing 100% of the historic 100-yr peak still produced 106% of the historic peak at the outlet; releasing 90% at each gave 97% (UDFCD Full Spectrum Detention Release Rate Memorandum, 2012). The mixed-frequency rule has no direct precedent; the nearest is the Malaysian DID (2008) checklist, which required post-development 2-, 10-, 50- and 100-yr peaks below the pre-development 2-yr peak. The specific "100-yr post ≤ 10-yr pre" form appears in a New Berlin, Wisconsin planning discussion (not an adopted rule). The Raipur entry follows the Kharun and the city's record. Raipur's nalas are linked to the Kharun "through eight big canals" (as reported), and the river set a new highest level of 281.68 m at Pathardihi on 29 August 2020, 2.47 m above its level of 1994, on the day the district had about 160 mm of rain (CWC, as transcribed by SANDRP; as reported); no warning or danger level is published for it. In the city's storm-drainage plan under AMRUT the drain network covered 49% of the city, about 480 km of drains were pucca, "Capacity of drain is not sufficient In rainy season most of the drains are overflowed", and 11 road stretches flooded two or three times a year; the plan proposed ₹1,191 crore of drainage works but stated no return period, intensity or runoff coefficient (Raipur Municipal Corporation, service level improvement plan, 4 November 2015), and the CAG's audit of AMRUT in the State, for the years to March 2022, covers water supply, sewerage and parks and never mentions drains. On 30 June 2007, when 297.0 mm fell at Raipur (IMD), more than 6,000 people were moved from low-lying parts of the city (as reported); on 4–5 July 2026 Adarsh Nagar (Mowa), DD Nagar, Shankar Nagar, Tatibandh and the Fafadih and Gudhiyari underpasses were under water and five ponds overflowed into the homes of Chaubey Colony (as reported); and in September 2026 the Kharun overtopped the road at Mahadev Ghat (as reported). The city's ponds have shrunk from about 130 to about 40 (as reported). The Bilaspur entry follows the Arpa. Bilaspur lies on the river below the Bhaisajhar barrage (full reservoir level 302.000 m, WRD); on 4 September 2025 the colonies from Sarkanda to Abhishek Vihar "turned into sprawling water bodies", and in July 2026 more than 400 houses were flooded after a fall reported as 28 cm in 24 hours by one paper and as 415.6 mm by another; the conflict is unresolved, and the city has no IMD station in the extremes table to test it [TBC]. The special area authority for the Arpa riverfront, whose development plan was approved in 2014, was largely sidelined after orders of the High Court in 2019, and no other line along the river was found. The Durg–Bhilai entry follows the Seonath nalas: the Shankar and Pulgaon nalas carry most of Durg's sewage to the Seonath, and the NGT ordered the treatment plants on them finished within six months (16 January 2026, as reported); in Bhilai, water hyacinth from the drains of the steel-plant township blocked the Junwani and Kosanala culverts in 2025 (as reported); and in the flood of September 2026 764 people were rescued in Durg and about 200 houses were inundated (as reported). The Korba entry follows the Hasdeo. The town lies below the Minimata Bango dam (spillway capacity 24,000 m³/s) and the Hasdeo barrage at Darri (19,820 m³/s) (CWC, National Register of Large Dams, 2009); when Bango released about 30,000 cusecs through six gates and Darri about 20,000 cusecs on 24–26 August 2024, houses were flooded in Motisagarpada, Seetamani and Imliduggu (as reported), and the Hasdeo set new highest levels at Katghora (313.58 m, 20 July 2026) and Bangodam (303.5 m, 17 August 2026) (as reported). Korba holds the record of 358.0 mm in 24 hours on 12 August 2004 (IMD), and its coalfield and ash dykes add sediment and breach hazards (Commentary to Clause 4.1). The Raigarh–Sarangarh entry follows the Hirakud backwater, which partly submerges 33 villages in Chhattisgarh and damaged villages on both sides of the border in 2011 (as reported); the Kelo overflowed in Raigarh on 14 August 2019, and the Mahanadi at Surajgarh and the Mand at Kurubhata set their highest levels, 201.06 m and 220.4 m, in August 2020 (as reported); Raigarh recorded 360.9 mm on 10 July 1958 (IMD). The backwater's extent within the State at Hirakud's full level has not been mapped [TBC]. The Jagdalpur entry follows the Indravati, the only river in the State with published CWC warning and danger levels found, 539.49 m and 540.80 m: it stood at 541.65 m, above danger, on 29 July–8 August 2019 (CWC), and set a new highest level of 544.891 m, 4.09 m above danger, on 26 September 2026 (as reported), while the floods of 25–28 August 2025 in the Bastar division killed eight people (as reported). Its upstream dams are in Odisha. Dhamtari, Rajim and Arang

below Gangrel, Rajnandgaon, Ambikapur and Kanker are candidates for notification under Clause 4.5.4. The notification triggers in 4.5.4.1 have no direct precedent and are set by this Manual. The 6-month lead time follows the update rule of Clause 2.14.2. Clause 4.16 explains why releases below pre-development are needed at all.

4.5.5 Minimum outlet and short-duration 2-yr checks

4.5.5.1 Where the PSD for any AEP and duration is less than the discharge of a **50 mm** orifice (Clause 4.8.2) at the corresponding stage, the 50 mm orifice is deemed to comply for that AEP and duration.

4.5.5.2 Where meeting the 2-yr criterion at a duration of 3 hours or less would require a low-flow orifice so small that the drawdown requirement of Clause 4.7 cannot be met, the designer shall:

- size the low-flow orifice as the smallest opening that meets Clause 4.7; and
- report the resulting 2-yr exceedance for those durations.

The Authority may accept the design if the exceedance is below the discharge of the 50 mm orifice at the 2-yr stage, or if the downstream analysis of Chapter 5 shows no adverse effect.

> Commentary. Under AMC II, a greenfield site produces almost no runoff in a 1- or 2-hour, 2-yr storm, because most of the rain is lost to initial abstraction. A pre-development peak of a few litres per second, compared duration by duration, then forces a very small low-flow orifice. That orifice cannot release the 10-yr volume within 48 hours. Checked as if it were outside a flood-sensitive catchment, the illustrative example of Clause 4.6.7 would need a low-flow orifice of about 75 mm to match the 1-hour, 2-yr pre-development peak. 90% drawdown of the 10-yr volume would then take about 84 hours even with a clean orifice, against the 48-hour limit. The 170 mm orifice that meets drawdown with 50% blockage exceeds the 1-hour, 2-yr pre-development peak by about 11 L/s. Clause 4.5.5.2 resolves that conflict in favour of drawdown, because a basin that is still partly full when the next SW monsoon storm arrives fails in the event that matters more. KC APWA 5600 (2006) used a 75 mm (3 in) minimum orifice with trash racks. This Manual sets a 50 mm minimum (Clause 4.8.2).

4.5.6 Constrained redevelopment sites

4.5.6.1 **Eligibility.** The relief route of this clause is available only for **redevelopment** where all of the following hold:

- (a) the existing site is at least **60% impervious**, as surveyed before demolition;
- (b) the site is **1 ha or less**; and
- (c) the designer shows, in a Constraint Statement submitted with Form 4-1, that the greenfield criteria of Clauses 4.5.2 or 4.5.3 cannot be met on the site. The statement must show that every storage option of Clause 4.10 has been assessed: rooftop and podium storage, tanks under parking or landscape, permeable paving with sub-base storage, and landscape depressions. It must also give the storage each option can provide.

New development on undeveloped land is never eligible.

4.5.6.2 **Minimum on-site performance.** An eligible site shall provide all of the following on site:

- (a) the full retention volume (Clause 4.4) and the WQv treatment (Chapter 8), with no relief;
- (b) post-development peak discharge no more than **50% of the existing-condition peak** for the 50%, 10% and 1% AEP storms, at each duration of Clause 2.5.2. The existing condition is modelled under Clause 3.3.4; and
- (c) no increase in the 1% AEP water level at any building or road downstream (Chapter 5).

4.5.6.3 **Shortfall.** The shortfall is the detention storage required to meet the greenfield criteria (Clause 4.6) minus the storage provided on site under 4.5.6.2. The applicant shall pay for the shortfall as a fee-in-lieu at the unit rate of Clause 4.13.3. The fee shall be:

- (a) paid into the ring-fenced account of the basin in which the site lies;
- (b) spent only on regional detention or on restoring talab or tank flood storage identified in the basin stormwater master plan (Chapter 11); and
- (c) recorded in the public ledger of Clause 4.13.4, which shall show the volume owed and the volume delivered in each basin.

Where no basin master plan has yet been approved, the fee shall be held for up to **5 years**. If no plan is approved in that time, it shall be spent on restoring tank flood storage in the same basin, with WRD agreement.

4.5.6.4 **Flood-sensitive catchments.** In the catchments of Clause 4.5.3, the route is available under the same conditions. The Authority may, however, refuse it where the downstream analysis of Chapter 5 shows existing flooding of buildings in the 10% AEP event within the analysis extent.

> Commentary. Greenfield is the target for all redevelopment, because a dense city only reduces its runoff as it rebuilds. On small, fully built plots in areas such as the old city of Raipur round Budha Talab, or the old cores of Bilaspur, Durg, Rajnandgaon and Raigarh, there is often no space for the full greenfield storage. A rule that cannot be met invites variances and non-compliance, which help no one downstream. This clause follows the approach of the UK Non-statutory Technical Standards for Sustainable Drainage (Defra, 2015, S2–S3). For previously developed sites, those standards require peak runoff "as close as reasonably practicable to the greenfield runoff rate" and never above the pre-redevelopment rate. The fixed 50% minimum reduction has no direct precedent; this Manual sets it to give reviewers a clear number in place of a "reasonably practicable" judgement. The fee-in-lieu turns the shortfall into regional storage in the same basin, so the basin still receives greenfield-equivalent control in total. Retention and water quality get no relief, because they are small volumes that almost every site can hold.

4.6 Detention sizing procedure

4.6.1 Method

Detention shall be sized by hydrograph routing through the stage–storage–discharge relationship of the facility, by level-pool (Modified Puls) or an equivalent numerical method (SWMM, HEC-HMS). Fixed-ratio and Rational-based storage formulae shall not be used. The simplified methods of Clause 4.14 (the Tier B tables and the fixed-coefficient route) are the only exceptions.

4.6.2 Steps

1. Delineate the site and sub-areas. Map HSG and confirm by testing (Chapter 3).
2. Extract design rainfall for 50%, 10% and 1% AEP and every duration in Clause 2.5.2, with the climate factor (Chapter 2, Form 2-1).
3. Build design hyetographs: alternating block for ≤ 24 h; the "peak day last" pattern for 48 and 72 h (Clause 2.9).
4. Compute pre-development hydrographs (greenfield CN; AMC III for storms of every duration from 1 July to 30 September on kanhar or dorsa paddy lowland, talab-draining and valley-floor sites, at all times in catchments draining to a reservoir or barrage pond, the Hirakud backwater, a mine void or a river at or above its CWC warning level, and at other sites and times after a 5-day total of more than 53 mm, AMC II otherwise).
5. Compute the release caps for each AEP and duration under Clause 4.5.2 or 4.5.3.

6. Compute post-development hydrographs with the same storms and AMC, including retention practices (Clause 4.4.6).
7. Choose a trial facility geometry (stage–storage) and a multi-stage outlet (Clause 4.6.5).
8. Route every storm and duration. Use a time step of no more than **5 minutes**, and no more than Tc/3 of the post-development catchment.
9. Check every release against its cap, and check drawdown (Clause 4.7). Adjust the outlet and storage and repeat.
10. Check the tailwater condition of Clause 4.6.6 and the downstream analysis of Chapter 5.
11. Size the emergency spillway and set freeboard (Clause 4.9).
12. Report the governing storm for storage, for each outlet stage, and for the spillway (Clause 4.17).

4.6.3 Storms to be routed

Every facility shall be routed for the full matrix:

Table 4.1 — Design storms to be routed for detention sizing

AEP	Durations (h)	AMC
50% (2-yr)	1, 2, 3, 6, 12, 24, 48, 72	II for ≤ 6 h; III for ≥ 12 h
10% (10-yr)	1, 2, 3, 6, 12, 24, 48, 72	II for ≤ 6 h; III for ≥ 12 h
1% (100-yr)	1, 2, 3, 6, 12, 24, 48, 72	II for ≤ 6 h; III for ≥ 12 h

The 24-, 48- and 72-hour runs are mandatory whatever the critical-duration result (Clause 2.5.2). Where Ia = 0.05S sensitivity is required by Chapter 3, it shall be run for the governing storage case.

4.6.4 Initial conditions

Each run shall start with:

- detention storage empty, down to the invert of the lowest outlet;
- any permanent pool full;
- retention practices empty, except as limited by Clause 4.11; and
- RWH storage full (Clause 4.11).

The 48- and 72-hour storms already contain their antecedent rain, so no further pre-filling is applied.

4.6.5 Multi-stage outlet design

4.6.5.1 The outlet shall have a separate control for each criterion, normally stacked as in Figure 4.2:

- **Stage 1**, at the invert: the low-flow orifice. It releases the WQv over 24–48 h (Chapter 8) and controls the 2-yr release.
- **Stage 2**: an orifice or slot set at or near the maximum 2-yr level. It controls the 10-yr release.
- **Stage 3**: a weir, riser crest or larger orifice set at or near the maximum 10-yr level. It controls the 100-yr release.
- **Emergency spillway**: separate from the primary outlet (Clause 4.9).

Stages may be combined where routing shows that every cap is met.

4.6.5.2 Orifices shall be computed with a discharge coefficient of 0.60 unless a rated device is used. Weirs shall use a coefficient appropriate to their shape. Submerged-outlet and tailwater effects shall be included (Clause 4.6.6).

4.6.5.3 Vortex flow controls and other proprietary devices may be used where the supplier provides a certified head–discharge curve and the minimum clear opening is at least 50 mm.

4.6.6 Tailwater on the outlet

The outlet shall be routed against the tailwater of the receiving system, not free outfall, wherever the receiving water level can rise above the outlet invert. The tailwater shall be:

- for a receiving drain: its water level in the same storm, from the Chapter 5 analysis or, for Tier B, the drain obvert;
- for tidal or backwater-affected outfalls: the levels and joint cases of Clause 2.12 and Chapter 5, including the outfall-closed check (Clause 5.10).

Where tailwater governs, a flap gate shall be fitted to the outlet (Clause 5.11). The storage below the tailwater level shall not be counted as available unless the routing includes the gate closure.

4.6.7 Illustrative example

Illustrative — not design values. This example shows the procedure and the sizes it produces. The inputs are invented to be plausible for a colony draining to the Kharun in Raipur. They are not NIH-DHAARA values and must not be used for design.

Inputs:

- Site: 2.0 ha, flood-sensitive catchment (Clause 4.5.3).
- Land cover: pre-development all pervious, CN 70. Post-development as two sub-areas (Clause 3.9.2): 1.57 ha of directly connected impervious surface at CN 98, never converted for AMC, and 0.43 ha of pervious open space at CN 70, on the same soil as pre-development; the area-weighted CN at AMC II is 92. For storms of 12 h and longer the pervious CN only is converted to AMC III: $CN_{III} = 23 \cdot CN / (10 + 0.13 \cdot CN) = 84.3$ (Clause 3.8.4).
- Time of concentration (Tc): 45 min pre-development, 15 min post-development (used for both sub-areas). SCS triangular unit hydrograph; 5-minute time step.
- 24-h depths: 110, 200 and 350 mm for 2-, 10- and 100-yr (the same 10- and 100-yr values as Clause 2.11).
- Shorter durations: $P(t) = P24 \cdot (t/24)^{0.40}$, an assumed ratio set and not Annex 2B.
- Longer durations: 48-h and 72-h depths taken as 1.25 and 1.40 × P24, laid out per Clause 2.9.2.
- Storage: vertical-walled, 2,500 m² plan area.
- Outlet, sized so that drawdown is met with the low-flow orifice 50% blocked (Clause 4.8.3). Annex 4C shows the trial outlet that failed this check.
    - Stage 1: 170 mm orifice at the invert.
    - Stage 2: 460 mm orifice with its invert 0.53 m above the floor.
    - Stage 3: none needed, because the 100-yr cap is met by Stages 1 and 2.

Results (discharges in L/s; storage in m³):

Table 4.2 — Illustrative example: peak discharges and storage used, by duration (Illustrative — not design values)

Duration (h)	2-yr pre	2-yr post, detained	10-yr pre	10-yr post, detained	100-yr post, uncontrolled	100-yr post, detained	100-yr storage used	10-yr drawdown, 50% blocked (h)
1	4	15	51	28	922	42	1,426	31.3
2	14	20	84	34	961	79	1,911	32.1
3	19	22	104	37	981	139	1,999	32.8
6	32	25	150	41	1,010	194	2,154	34.2
12	127	30	302	46	1,074	276	2,512	39.0
24	149	32	327	49	1,081	320	2,766	**45.3**

Duration (h)	2-yr pre	2-yr post, detained	10-yr pre	10-yr post, detained	100-yr post, uncontrolled	100-yr post, detained	100-yr storage used	10-yr drawdown, 50% blocked (h)
48	173	34	349	82	1,087	343	**2,913**	44.8
72	180	34	356	84	1,089	343	2,911	44.7

How to read it:

- **100-yr cap.** The cap is the largest pre-development 10-yr peak, 356 L/s (72 h). Every 100-yr detained peak is at or below it; the largest is 343 L/s.
- **2-yr cap.** The site is flood-sensitive, so the 2-yr cap is the largest pre-development 2-yr peak, 180 L/s (72 h; Clause 4.5.3.1). The largest 2-yr detained peak is 34 L/s. At 1, 2 and 3 hours the detained 2-yr peak is above the pre-development peak of the same duration (15 against 4 L/s, 20 against 14 L/s, and 22 against 19 L/s). That comparison is not required here. It would be for a site under Clause 4.5.2, and Clause 4.5.5.2 would then apply.
- **Governing storage.** Storage is governed by the **48- and 72-hour** storms: 2,913 m³, which is 0.146 m³ per m² of site, at a maximum depth of 1.17 m. A design that checked only the critical-duration peak storm (1–3 h here) would have provided about 2,000 m³, roughly two-thirds of what is needed.
- **Drawdown.** With the low-flow orifice 50% blocked (Clause 4.8.3), 90% of the 10-yr storage drains in at most 45.3 h (the 24-hour storm), within the 48 hours of Clause 4.7. A 120 mm orifice would drain it in about 42 h when clean but 54 to 75 h when half blocked, so it fails. Annex 4C shows that trial and the routing step by step.
- **Retention credit.** With an illustrative retention depth of 40 mm, RRV = 552 m³, computed for each sub-area (Clause 3.9.4). Modelled as filling from the start of the storm with no infiltration during it, the retention reduces the 24-hour storage by 138 m³ and the 48- and 72-hour storage by 7 m³ or less. The credit under Clause 4.4.6 is therefore effectively zero for this site.

The order of magnitude, about 0.15 m³ per m² of site, is consistent with the indication in Clause 2.11.

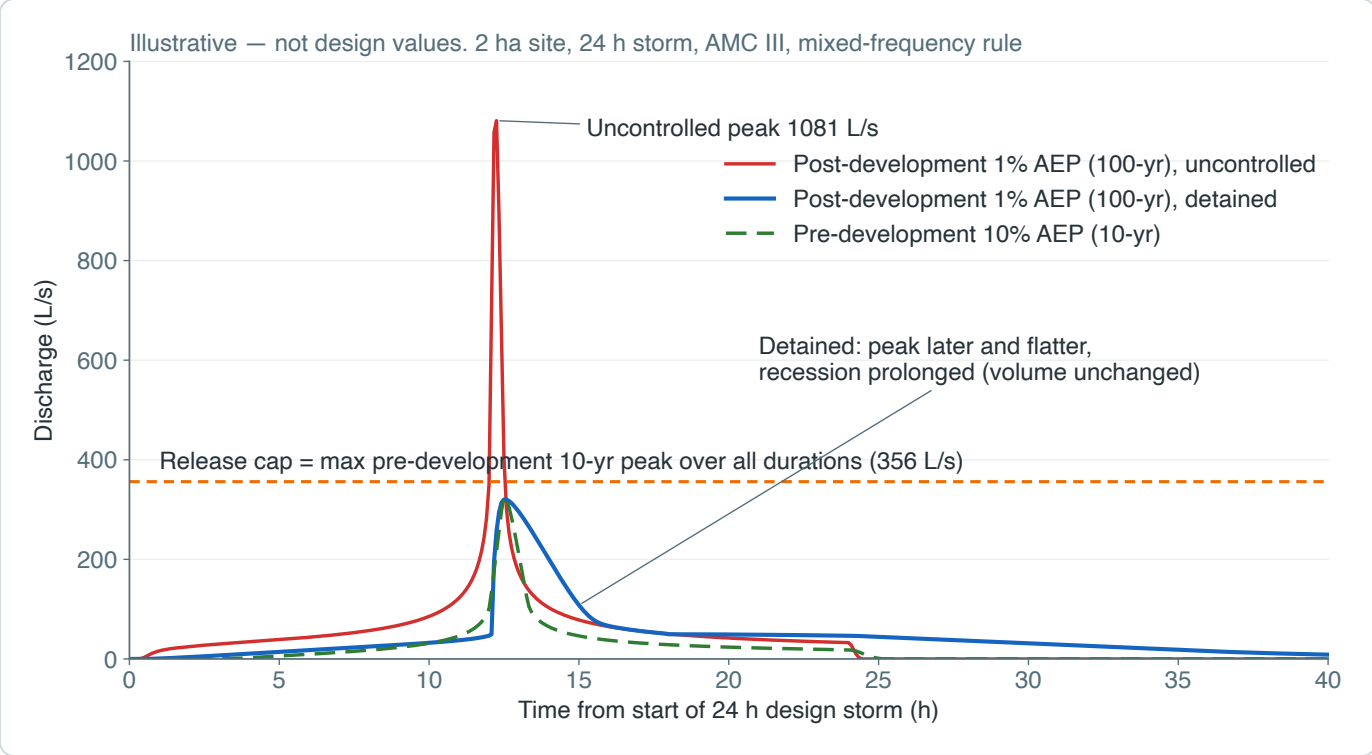


Figure 4.1 — Pre-development 10-yr, post-development 100-yr uncontrolled and detained hydrographs for the 24-hour storm of the illustrative example (Clause 4.6.7). Illustrative — not design values. Original drawing for this Manual, adapted from the pre/post-development hydrograph concept in MARC/KC APWA Section 5600 and the Georgia Stormwater Management Manual (2016).

> Commentary. Figure 4.1 shows what detention does and does not do. It cuts and delays the peak. It does not reduce the volume: the area under the detained curve equals the area under the uncontrolled curve, more than twice the pre-development 10-yr volume. The extra volume leaves the site over many hours after the storm. That prolonged release can coincide with the peak of the larger catchment downstream (Clause 4.16), and it must still drain when the outfall is tide-locked (Chapter 5).

4.7 Drawdown

4.7.1 **Detention.** At least **90%** of the maximum volume stored in the 10% AEP (10-yr) storm of each duration shall be released within **48 hours** of the time of maximum storage, by gravity, with the tailwater of Clause 4.6.6.

4.7.2 **Water quality volume.** The WQv shall be released over not less than 24 hours and not more than 48 hours (Chapter 8).

4.7.3 **Retention.** Retention storage shall be emptied within 72 hours by infiltration or reuse (Clause 4.4.5).

4.7.4 **Tidal outfalls.** Where gravity drawdown is prevented by tide for part of each cycle, the 48-hour limit applies with the tidal tailwater modelled. Where it cannot be met by gravity, pumped drawdown shall be provided under Clause 5.12.

> Commentary. KC APWA 5600 (2006) required dry basins to release at least 80% of storage within 24 hours of the inflow peak and to empty within 72 hours. Singapore PUB requires detention to be restored within 4 hours. The 48-hour, 90% rule is longer than PUB's because Chhattisgarh storage must hold multi-day volumes. It is still short enough that a basin filled by one SW monsoon depression will have most of its volume back before the next, which in late June and early July 2001 (365.0 mm at Jashpurnagar and 298.0 mm at Raigarh on 30 June, 281.8 mm at Narayanpur on 6 July and 497.8 mm at Charama on 9 July, IMD) followed within days.

4.8 Outlet structures

4.8.1 **General.** Primary outlets shall be:

- unattended, with no moving parts, valves or pumps, except where Clause 5.12 permits pumped drawdown;
- accessible for inspection and cleaning from the surface without confined-space entry, where practicable; and
- built of durable materials. In coastal areas these shall be corrosion-resistant (Clause 4.12.5).

4.8.2 **Minimum orifice.** No control orifice shall be smaller than **50 mm** in diameter, or 50 mm in least dimension for a slot.

4.8.3 **Trash racks and screens.** Every orifice and weir shall be protected by a trash rack or screen. The rack shall:

- have a clear bar spacing no greater than half the smallest opening it protects, and not less than 25 mm;
- have a clear open area at least **10 times** the area of the opening it protects;
- be inclined, or set as a cage around the riser, so that debris rises with the water and does not pin against the bars;
- be removable or hinged for cleaning; and
- for routing, be assumed **50% blocked** at the low-flow orifice when checking drawdown.

4.8.4 **Anti-vortex devices.** Riser crests and orifices that may run under more than 1.5 times their diameter of head shall have an anti-vortex plate or hood (Figure 4.2).

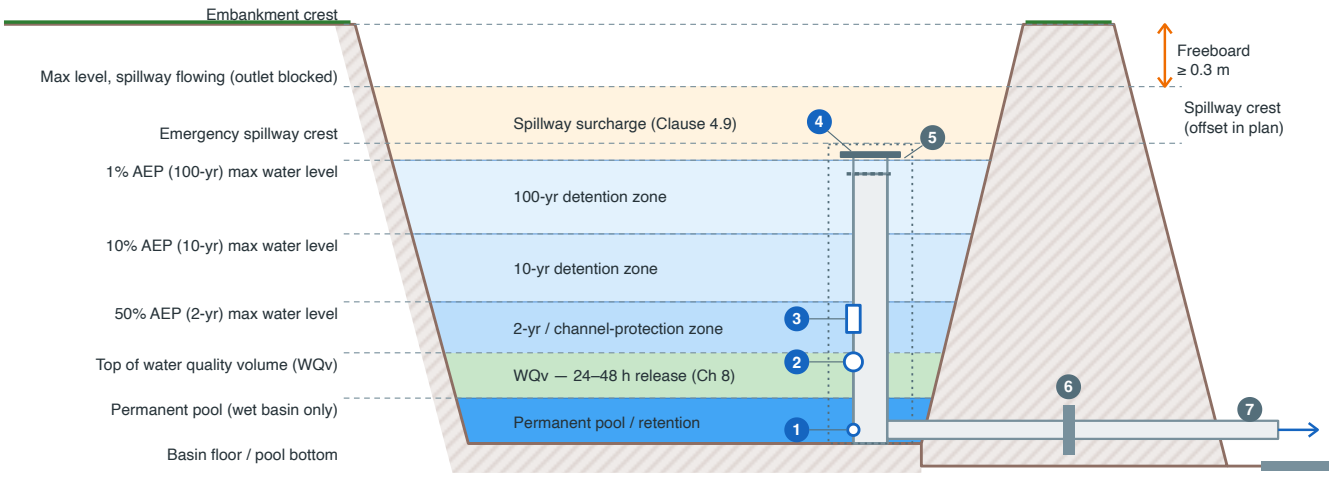
4.8.5 **Outlet barrels.** Outlet barrels through embankments shall:

- be at least 300 mm in diameter (Chapter 6);
- have watertight joints;
- have a filter diaphragm or seepage collars over the length within the embankment; and
- have an energy-dissipating outfall (Chapter 9).

4.8.6 **Isolation.** Facilities serving more than 1 ha shall have a means of isolating the outlet for maintenance, such as a stop-log slot or penstock. In normal service it shall be left fully open.

> **Commentary.** The 50 mm minimum is smaller than the 75 mm (3 in) of KC APWA 5600 (2006), so the trash-rack requirements are correspondingly stricter. The ratios in 4.8.3 (bar spacing at half the opening, open area 10 times the opening, 50% blockage for the drawdown check) have no direct precedent and are set by this Manual. The blockage factor matches the 50% grate clogging factor for sag inlets in Chapter 6. Indian urban drains carry heavy loads of solid waste, so a low-flow orifice without an effective rack will block within one season.

Storage stack and multi-stage outlet — schematic section, not to scale



Key

1 Stage 1 low-flow orifice (≥ 50 mm, Clause 4.8.2) — releases WQv over 24–48 h
2 Stage 1 second orifice — with 1, controls 50% AEP (2-yr) release
3 Stage 2 orifice or weir slot — controls 10% AEP (10-yr) release
4 Stage 3 riser crest / overflow grate with anti-vortex plate — controls 1% AEP (100-yr) release
5 Trash rack enclosing all openings (Clause 4.8.3)
6 Seepage collar or filter diaphragm on outlet barrel
7 Outlet barrel, energy dissipation and outfall apron
Levels and zone thicknesses are diagrammatic only.

Figure 4.2 — Detention storage stack (permanent pool, WQv, 2-yr, 10-yr and 100-yr zones, spillway surcharge and freeboard) and multi-stage riser outlet. Schematic, not to scale. Original drawing for this Manual, adapted from the multi-stage outlet and storage-zone concepts in MARC/KC APWA Section 5600 (2006, 2025.01) and the Mile High Flood District Urban Storm Drainage Criteria Manual, Volume 2 (full-spectrum detention).

4.9 Emergency spillway, freeboard and embankment safety

4.9.1 **Spillway capacity.** Every detention facility formed wholly or partly by an embankment, or whose overflow would reach buildings, shall have an emergency spillway that passes the **1% AEP (100-yr) peak inflow**:

- with the primary outlet assumed fully blocked;

- with the facility full to the spillway crest at the start of the storm; and
- with at least **0.3 m freeboard** from the maximum water level to the embankment crest.

4.9.2 **High-hazard facilities.** A facility is high-hazard if its failure or overflow could:

- endanger life;
- flood a habitable building or critical facility; or
- close an arterial road.

High-hazard facilities shall also be checked for the **0.2% AEP (500-yr)** peak inflow under the conditions of 4.9.1, with the embankment not overtopped.

4.9.3 **Spillway crest.** The spillway crest shall be at or above the 1% AEP maximum water level from routing with the primary outlet working. The recommended clearance is 0.15 m.

4.9.4 **Spillway location.** The spillway shall be:

- on natural ground or a reinforced section, not on fill, unless protected against erosion for the full design flow;
- discharging to the 1% AEP overland flow path of Chapter 6; and
- placed so that its flow does not reach a building.

4.9.5 **Underground and enclosed facilities.** Tanks, vaults and podium storage shall have an overflow route that operates when the outlet is blocked. The route shall carry the 1% AEP inflow to the overland path without surcharging any building, basement or lift pit.

4.9.6 **Embankments.**

- (a) Embankments more than **2 m** high (measured from the downstream toe to the crest) shall be designed by a qualified geotechnical engineer. The design shall include stability, seepage and settlement analysis.
- (b) Side slopes shall be no steeper than 3H:1V on any face that is mown or accessible to the public. Slopes shall be flatter where Clause 4.15 requires.
- (c) Embankments shall have:
  - a crest width of at least 3 m;
  - compacted, engineered fill with a core or cut-off where required by the seepage analysis; and
  - no trees or large shrubs.
- (d) An embankment structure that falls within the Dam Safety Act, 2021 is subject to that Act in addition to this Manual.

4.9.7 **Excavated facilities.** Facilities wholly below the surrounding ground, with no embankment, need not meet 4.9.6. They shall still meet 4.9.1, with the overflow route treated as the spillway.

> **Commentary.** The spillway criterion (1% inflow, outlet blocked, basin full, 0.3 m freeboard; 0.2% for high hazard) follows the KC APWA 5600 (2006) rule: the spillway passes the 1% storm with 1 ft (0.3 m) freeboard to the top of the dam, assuming zero storage and a clogged primary outlet, with the crest at least 0.5 ft (0.15 m) above the 1% maximum water surface. KC designed dams above 10 ft (3 m) to SCS TR-60 Class C. The 2 m embankment trigger, the 3H:1V slope (KC wet ponds), and the 3 m crest width have no direct precedent and are set by this Manual.

4.10 Facility types

4.10.1 **General.** Any storage that meets Clauses 4.5 to 4.9 may be used. Surface storage that is visible and can be inspected without confined-space entry is preferred. Storage shall not be placed in a habitable basement or in any space whose flooding would endanger life.

4.10.2 **Underground tanks and vaults.**

- (a) Gravity outlet is required where the receiving drain allows it. Pumped outlets are permitted only under Clause 5.12, with a registered maintenance declaration (Chapter 10).
- (b) Access openings shall be provided over the outlet chamber and over every inlet. In tanks over 500 m², they shall be no more than 30 m apart.
- (c) The floor shall fall at least 1% to a sump at the outlet, with a sediment pocket below the low-flow orifice invert.
- (d) The tank shall be ventilated.
- (e) Inlets shall pass through a silt trap or gross pollutant trap (Chapter 8).
- (f) Flotation and groundwater ingress shall be checked under Clause 4.12.

4.10.3 **Rooftop detention.**

- (a) It is permitted on flat roofs designed for the full ponded load with the outlet blocked. The ponding depth shall not exceed **150 mm** unless a structural engineer certifies a greater depth.
- (b) Each roof outlet shall have a flow-control ring or weir with a certified rating, and every roof area shall have at least one overflow scupper, independent of the controlled outlets, that passes the 1% AEP inflow.
- (c) Roofs used for RWH collection may also provide rooftop detention. The detained volume is released through the control, not into the RWH sump.

4.10.4 **Podium and landscape-deck detention.**

- (a) It is permitted above ground level or above a basement car park, provided the structure is designed for full saturation and ponding, and waterproofed.
- (b) The release, overflow and safety rules for rooftop detention (4.10.3) apply.

4.10.5 **Parking-lot detention.**

- (a) It is permitted only on surface parking. Ponding depth shall not exceed:
  - **200 mm** in the 10% AEP storm and **300 mm** in the 1% AEP storm at any parking bay;
  - zero on accessible routes, ramps and pedestrian crossings; and
  - zero in the 10% AEP storm on fire-tender access routes.
- (b) Signs shall warn that the area floods.
- (c) Ponding shall not reach a building wall unless the wall is waterproofed above the 1% level plus freeboard (Chapter 6).

4.10.6 **Multi-use detention (parks, playfields, open space reservation).**

- (a) Open space land may be used for detention, provided its primary use remains available in at least the 50% AEP storm.
- (b) The storage areas shall be graded to drain fully within the times of Clause 4.7.
- (c) A low-flow channel or underdrain shall carry dry-weather and frequent flows, so that the park is not waterlogged.
- (d) The depth and slope limits of Clause 4.15 apply.
- (e) Play equipment and seating shall be placed above the 10% AEP level.

4.10.7 **Ponds and wet basins.**

- (a) A permanent pool shall be at least 1.2 m deep over most of its area to discourage weed growth and mosquito breeding, with a vegetated safety bench at the edge (Clause 4.15).

- (b) Only storage above the permanent pool counts as detention.

> Commentary.
>
> - The rooftop depth (150 mm) and parking depths (200 and 300 mm) have no direct precedent. This Manual sets them so that a blocked outlet is noticed before it becomes dangerous.
> - The 1.2 m pool depth equals the 4 ft minimum permanent pool of KC APWA 5600 (2006). This Manual adopts it mainly for weed and mosquito control.
> - Pumped detention tanks need maintenance declarations in Singapore (PUB Addendum No. 3, April 2025). This Manual adopts the same approach through Chapter 10.
> - Using open space for multi-use detention fits the CGBVN 1984, under which layouts keep community open space (Rule 47; the Chhattisgarh percentage was not seen [TBC]) and integrated townships keep at least 10% of their area as gardens and playgrounds (Appendix ण, as amended in 2019), and the draft green-building rule of 2017 would have kept at least 20% of the recreational open space unpaved (draft rule 79-C, final text not found [TBC]). Nava Raipur Atal Nagar was planned with its parks as carriers of storm water to its lakes (as reported), and its development plan asks that rain water from the storm drains be stored in dug or natural lakes and depressions (Naya Raipur Development Plan-2031, §10.3.2). A detention area graded into the open space below the level of the road keeps it pervious and usable, which is what 4.10.6 requires. Four kinds of land are not open space for this purpose. A talab or dabri on the land is preserved under step 1 of Clause 4.3.1 and modelled as a water surface (Clause 3.3.2(e)) that starts at its full level (Clause 3.12.5), so only its flood storage above that level can count, and only as Chapter 7 allows; village tanks over which the community exercised rights of irrigation or nistar vest in the State (Chhattisgarh Land Revenue Code 1959, section 251), their encroachment is proceeded against as unauthorised occupation (section 248), as at the ponds of Baloda under the NGT's order of 14 November 2024 in O.A. 78/2024, and public tanks vest in the Municipal Corporation (section 82 of the Municipal Corporation Act 1956, as adapted in Chhattisgarh [TBC]). Land in a buffer is not spare land either: the line 50 m from the highest flood level of a wetland, which the State Wetland Authority has applied to halt works at Karbala Talab and to probe a project at Narhaiya Talab in Raipur, and to have NRDA withdraw tenders beside the Jhanjh wetland in Nava Raipur (2025, as reported); the Kopra Jalashay Ramsar site at Bilaspur; and the distances of Rule 50 from water bodies and watercourses, 9 m and 15 m in the inherited 1984 text, whose Chhattisgarh wording is to be confirmed [TBC]. Land in the hydraulic corridor of a nala, talab or river under Chapter 7 is corridor or flood land already: water reaches it first, so it is not counted as detention for the development beside it (Chapter 7). And a mine void, pit lake, ash pond or ash dyke on or beside the land is not open space or storage for the site unless the lessee and DGMS, or for an ash pond its operator, agree in writing and its levels are surveyed (Commentary to Clause 4.1).

4.11 Rainwater harvesting versus detention accounting

4.11.1 **No double counting.** RWH storage (sumps, tanks, recharge pits and wells provided under the RWH mandate) shall **not** be counted as detention.

4.11.2 **RWH sumps assumed full.** For all routing under this chapter, RWH sumps and tanks shall be assumed **full at the start of every storm**, in every season. Their overflow shall be routed to the detention facility, or directly to the outfall if it bypasses detention. In the latter case the bypass flow counts against the PSD.

4.11.3 **Combined tanks.** A single tank may provide both RWH and detention only if:

- (a) the detention zone lies wholly **above** the RWH overflow level;
- (b) the detention zone is drained by its own gravity control orifice set at the RWH overflow level, meeting Clauses 4.7 and 4.8;

- (c) no pump, valve or user action can hold water in the detention zone; and
- (d) the two zones and the orifice level are shown on the drawings and in the maintenance declaration.

Only the volume between the RWH overflow level and the detention overflow level counts as detention.

4.11.4 **Recharge structures.** Recharge pits, trenches, shafts and wells may count as **retention** (not detention) if they meet Clauses 4.4.3 to 4.4.5. In particular, the seasonal high groundwater table shall be at least 1.0 m below their base. In the routing they are treated under Clause 4.4.6 with the groundwater level for the SW monsoon season.

4.11.5 **RWH remains mandatory.** Nothing in this chapter reduces the RWH requirement of rule 78(4) of the CGBVN 1984 and its Appendix ढ (N), the General Administration Department order of 31 May 2016 for government buildings, the rainwater harvesting required in integrated townships (Appendix ण of the CGBVN 1984) and in Nava Raipur Atal Nagar (Naya Raipur Development Plan-2031, §10.3.2), any area notified under the Chhattisgarh Ground Water (Management and Regulation) Act 2022 once it is in force, any condition of a development or building permission, or any rainwater harvesting rule or direction of an approving authority.

> Commentary. RWH in Chhattisgarh rests on rule 78(4) of the CGBVN 1984, which sets out the planning standards for rainwater harvesting (Housing and Environment Department, letter No. 2940/1445/32/2009 of 23 November 2009) and was amended on 17 February 2009 (notification No. 318/1193/32/2006, Gazette p. 124(10)). The amendment substitutes the figures "500" and "300" in sub-rule (4), without saying for what ("नियम 78 के उपनियम (4) में अंक '500' तथा अंक "300" प्रतिस्थापित किया जावे", as printed), and adds clause (क): if the owner does not build and maintain the rainwater harvesting system as provided, the Authority may impose a penalty of up to ₹1,000 a year for every 100 m² of built-up area. The Chhattisgarh text of the sub-rule before or after 2009 was not read, so its threshold is not known: a Rajya Sabha answer of 8 July 2019 reports that rule 78(4) makes rain or roof water harvesting mandatory "for all types of buildings having plot size more than 500 sq. mt." in towns with planning areas notified under the 1973 Adhiniyam, which fits "500" as the plot threshold, but what "300" refers to is unknown [TBC]. The options are set out in Appendix ढ (Appendix N). The 2009 amendment replaced the pebbles of ढ-2 with a filter of fine stone over 50% of the depth at the bottom and fine sand over 20% in the middle, and added ढ-4: an underground storage tank, dug as needed in the marginal open space of the house, into which rain water is also led, with a tap to draw the water for washing clothes, cleaning floors and the garden, and, in premises of 100 dwelling units, a tank under a plant that treats the used water; options ढ-1 and ढ-3 were not read [TBC]. Integrated townships must provide rainwater harvesting (Appendix ण, 2008), Nava Raipur makes it "mandatory at individual development level" (Naya Raipur Development Plan-2031, §10.3.2), and the draft green-building rule of 2017 would have required it on every plot of more than 100 m² (draft rule 79-C, 30 March 2017), but whether that draft was made final was not found [TBC]. For government buildings, the General Administration Department ordered on 31 May 2016 that every building of a department or its undertakings under construction with a roof of more than 1,500 sq ft (about 139 m²) shall have rainwater harvesting, and every existing building with a pucca roof of more than 5,000 sq ft (about 465 m²) shall be given it, from the department's own budget (order No. F 9-22/2016/1/5). The Chhattisgarh Ground Water (Management and Regulation) Act 2022 lets the State enforce rainwater harvesting and recharge in notified over-exploited and critical blocks and stressed urban areas (section 7), but no notification bringing it into force or notifying an area was found [TBC]. Raipur Municipal Corporation is reported to require at least 50 rainwater harvesting pits in each township, and the district acted in 2016 against 18 builders and 7 industrial units that had not built them (as reported). None of these rules sets a storage volume, an infiltration test or a depth to the water table, requires a tank to be empty before a storm, or restricts recharge on clay, karst, mine ground or contaminated aquifers. This Manual credits these structures against its own retention requirement (Clause 4.4) as follows:
>
> - An RWH storage tank or sump, including the underground tank of Appendix ढ-4 and the tanks built under the order of 31 May 2016, is RWH storage. It is not detention (Clause 4.11.1), and it is assumed full at the start of every storm (Clause 4.11.2), because after the July rains it is full. A tank may also provide detention only if it is built as a combined tank under Clause 4.11.3, with a detention zone above its RWH overflow level that drains through its own orifice; only that zone counts.

- An RWH tank counts toward the **retention** requirement of Clause 4.4 only to the extent that it is emptied within 72 hours by a continuous non-potable use, such as toilet flushing or washing, demonstrated under Clause 4.4.3; watering a garden is not a continuous use in the monsoon. The tank is then modelled under Clause 4.4.6 like any other retention practice.
- The pits, filter beds and recharge structures of Appendix ढ, and the pits that Raipur requires in townships, count as retention only where they meet Clause 4.11.4: they are credited only to the extent that they drain within 72 hours at half the rate measured in a site test (Clauses 4.4.3 and 4.4.5), with their base at least 1.0 m above the seasonal high water table, and the credit is found by routing under Clause 4.4.6, not by deducting their volume.
- The community open space of a layout (Rule 47), the open space of an integrated township and the unpaved area of the draft green-building rule are pervious area. They enter the post-development run as pervious land under Clause 3.9.2 and get no further credit as retention or detention unless they are built and tested as an infiltration practice.
- **Recharge first where the aquifer is stressed.** Groundwater use is moderate statewide, at a stage of extraction of 47.32% in the 2024 assessment, with 120 of the 146 blocks safe, 21 semi-critical, 5 critical and none over-exploited, but the critical blocks lie in the Seonath plain and include Dharsiwa, round Raipur, at 93.51%, with Gurur, Nawagarh, Bemetara and Berla, and the blocks of Durg and Dhamdha are semi-critical (CGWB, *Dynamic Ground Water Resources of Chhattisgarh 2024*; a press report that no block was critical in 2026 is unverified [TBC]). In these urban blocks, retention shall be provided by recharge before reuse or detention wherever site tests allow, through a silt trap, a filter and a first-flush diverter ahead of the recharge structure, and with a hydrogeologist's check of the target zone.
- **No infiltration practice on kanhar.** No pit, trench, filter bed, basin or recharge structure shall be founded on or in kanhar, which takes water through its cracks only while it is dry and almost none once they close (Clause 3.7.2). The RWH of rule 78(4) may still be met on such a site by a storage tank under Appendix ढ-4, which is credited only as the second bullet allows.
- **A cavity check in the limestone belt.** The Chandi and Charmuria limestones of the Raipur–Durg–Rajnandgaon belt, which supply Raipur's groundwater, are officially described as "Karstified and Cavernous", with field permeabilities of 4–65 m/day and solution cavities filled with clay (CGWB, 2024; CGWB, Rajnandgaon brochure); sinkholes and borehole cavities near Raipur are reported but not confirmed by the Geological Survey [TBC]. In this belt, an infiltration or recharge practice is credited only after a cavity check below its base, by probe holes or a geophysical survey, and only with the pre-treatment above, because runoff that reaches a cavity enters the aquifer with almost no filtering and can wash out the clay that fills it.
- **No recharge credit on wet or disturbed ground.** On the kanhar and dorsa paddy lowlands, the floodplains, the talab and reservoir margins, the Hirakud backwater and the other land of Clause 4.12.1, and wherever the seasonal high water table is within the 1.0 m separation of Clause 4.4.3, no recharge or infiltration practice earns retention credit; detention, conveyance and reuse govern there. No recharge or infiltration practice shall be built on an overburden dump or other mine-disturbed ground, on or beside an ash pond or ash dyke, or within the dewatering zone of a working mine, such as the drawdown of about 1.2 km round the Kusmunda and Gevra pits at Korba (as reported), where the water would be pumped out again or would wet unstable ground, and none within 50 m of the edge of a mine void without a geotechnical check.
- **Fluoride, arsenic and nitrate.** Fluoride above 1.5 mg/l was found in 14 of 783 samples, in Janjgir-Champa, Kanker, Korba, Koriya, Mahasamund, Raigarh, Rajnandgaon and Surajpur districts; arsenic in samples from Korba, Koriya and Raigarh and in the wells of Kaudikasa in the Ambagarh Chowki block, formerly of Rajnandgaon district; and nitrate above 45 mg/l in 11.5% of samples, in 20 districts including Raipur, Durg and Bilaspur (CGWB factsheet, December 2024; CGWB studies of Ambagarh Chowki). In these districts the hydrogeologist's check shall report the fluoride, arsenic and nitrate of the target aquifer before a recharge bore or shaft is credited, and untreated runoff shall not be put into a bore or shaft there; retention is then provided by surface infiltration through a soil or filter layer, or by detention and reuse (Chapter 8).
- **Everywhere,** road and parking runoff shall be recharged only through a surface infiltration practice with a filter, such as a vegetated basin or trench, never directly into a bore or well, and runoff from a nala that carries sewage, as most of Raipur's and Durg's nalas do, shall not be recharged at all.

> On kanhar and the paddy lowlands a recharge pit drains slowly or not at all in the SW monsoon, and in the Seonath plain a sump is full after the first heavy rain. Treating RWH as detention would therefore credit storage that does not exist when it is needed. That is why this Manual assumes RWH sumps are full at the start of every storm. Applying the assumption to every season keeps one simple rule. The combined-tank provision (4.11.3) lets an owner build one structure, as long as the detention part is empty when the storm arrives. The recharge-first rule for the stressed urban blocks, the bar on infiltration practices on kanhar, the cavity check in the limestone belt, the bars on mine-disturbed ground, ash ponds and dewatering zones, the 50 m distance from a mine void, the fluoride, arsenic and nitrate checks and the pre-treatment and road-runoff rules are set by this Manual; no Chhattisgarh rule on where recharge may or may not be credited was found.

4.12 High-groundwater and sandy coastal sites

4.12.1 **Design groundwater level.** Each site in the kanhar and dorsa paddy lowlands of the Chhattisgarh plain, where the water table rises close to the surface in the monsoon, the floodplains of the Kharun, the Seonath, the Arpa, the Hasdeo, the Kelo, the Mahanadi and the Indravati and their tributaries, the beds and margins of the talabs, reservoirs and barrage ponds, among them the talabs of Raipur and the lakes of Nava Raipur, and the Hirakud backwater at Raigarh and Sarangarh, or any area where the groundwater table lies within 3 m of the surface at any time of year shall establish a design groundwater level (DGWL) for the facility. The DGWL is the seasonal high level during or after the SW monsoon, from:

- (a) site piezometer readings over at least one full June–November season; or
- (b) where (a) is not available, the higher of the CGWB or Water Resources Department record for the nearest observation well, plus **0.5 m**, and the design flood level of any adjacent river, nala, talab, reservoir or mine void.

For the flotation check of 4.12.3 at sites in the kanhar and dorsa paddy lowlands, the talab and river floodplain margins and the Hirakud backwater, where (a) is not available, the DGWL shall be taken as existing ground level.

4.12.2 **Lined storage.** Where the DGWL is less than 1.0 m below the base of a storage facility:

- (a) the facility shall be watertight (lined or of watertight concrete), and no infiltration credit applies (Clause 4.4.4); or
- (b) if unlined, the volume below the DGWL shall not be counted as detention, and groundwater inflow shall be added to the routed inflow.

4.12.3 **Uplift and flotation.** Every below-ground tank, vault, lined pond or chamber shall be checked for flotation:

- with the facility empty;
- with groundwater at the DGWL; and
- for coastal sites, with groundwater also at the major-system tailwater level of Chapter 5, where that is higher.

The factor of safety against uplift, counting only permanent dead loads and ignoring wall friction unless supported by geotechnical design, shall be at least **1.2**. Anchoring by ballast, toe extensions or tension piles may be used.

4.12.4 **Shallow and above-ground storage preferred.** On sites where the DGWL is within 1.5 m of ground, storage should be provided above ground where possible: rooftop, podium, landscape or surface storage. Deep tanks should be avoided.

4.12.5 **Durability.** Concrete and metal parts in contact with saline or brackish groundwater or tidal water shall be designed for the exposure class appropriate to coastal conditions under IS 456:2000. Outlet fittings, trash racks and gates shall be of stainless steel, HDPE, FRP or another corrosion-resistant material.

4.12.6 **Sandy soils with deep groundwater.** Where the DGWL is 1.0 m or more below the base, infiltration retention in sandy soils is encouraged. The design infiltration rate of Clause 4.4.3 applies, and the Chapter 8 requirements on pollution of shallow aquifers shall be met.

> **Commentary.** Chhattisgarh is landlocked, so the coastal provisions of this clause, the tidal water levels of 4.12.1 and 4.12.3, do not apply in the State, and saline groundwater is rare (electrical conductivity above 3,000 µS/cm in 2 of 783 samples; CGWB factsheet, December 2024), so the exposure provisions of 4.12.5 apply only where a test at the site shows it, or where the facility is in contact with mine water or ash-pond water whose chemistry has not been tested. The water table of the plain is several metres down before the monsoon but rises close to the surface after it: in 2023, 52% of the State's observation wells stood at 5–10 m before the monsoon, and about 76% within 5 m after it, 17.6% of them within 2 m (CGWB, *Dynamic Ground Water Resources of Chhattisgarh 2024*); in Raipur district the post-monsoon depth was 0.87–7.05 m (CGWB district brochure, about 2007). The kanhar and dorsa paddy lowlands, the floodplains of the Kharun, the Seonath, the Arpa, the Hasdeo, the Kelo, the Mahanadi and the Indravati, the margins of the talabs, lakes, reservoirs and barrage ponds, and the Hirakud backwater at Raigarh and Sarangarh are the wet cases. There an unlined tank fills with groundwater before the storm, and an empty lined tank can float, especially when a river in flood, a reservoir or barrage pond at its full level, a talab or a flooded mine void raises the water table beside it; in the Gondwana coalfields groundwater can also stand under flowing (artesian) conditions (CGWB, 2024). The design groundwater level of 4.12.1(b) therefore takes the design flood level of the adjacent river, nala, talab, reservoir or mine void where that is higher than the well record, and on the paddy lowlands, the talab and river floodplain margins and in the Hirakud backwater it is taken as existing ground level for the flotation check unless piezometers show otherwise. The June–November monitoring season of 4.12.1(a) is set by this Manual to cover the SW monsoon and the post-monsoon high that the CGWB measures in November: a reading taken in May or June, when most wells are at 5–10 m, understates the design level. No monsoon water-table depths for individual town sites in Raipur, Durg–Bhilai, Bilaspur, Korba or Raigarh were obtained [TBC]. The factor of 1.2, the 0.5 m allowance and the "ground level" default for the lowlands, the floodplain margins and the Hirakud backwater have no direct precedent; this Manual sets them because no published site-scale seasonal groundwater depths for Chhattisgarh were found during research.

4.13 Regional detention and fee-in-lieu

4.13.1 **Where permitted.** In a basin covered by a Basin Stormwater Master Plan approved under Chapter 11, a development may meet all or part of its detention requirement through:

- (a) a regional detention facility identified in the plan; or
- (b) a fee-in-lieu paid toward restoration of talab or tank storage identified in the plan.

4.13.2 **Conditions.** Regional detention or fee-in-lieu shall be accepted only where:

- (a) the regional facility or restored tank is operational, or is funded and scheduled to be operational within **3 years**. Until then, interim on-site detention is required.
- (b) the conveyance from the site to the facility carries the 1% AEP post-development flow without increasing water levels at buildings or roads between them (Chapter 5);
- (c) the storage credited at a tank is the **flood storage** only. This is the volume between full tank level (FTL) and the maximum water level the bund and surplus weir can safely hold, plus any volume below FTL that a notified operating rule commits to empty before the SW monsoon. Storage below FTL that is held for irrigation or supply through June to September shall not be credited.
- (d) the tank meets Chapter 7: surveyed FTL, a documented operating rule, and a maintained surplus weir;
- (e) the site still provides **retention** (Clause 4.4) and **WQv** treatment (Chapter 8) on site; and
- (f) in flood-sensitive catchments, the regional facility meets the mixed-frequency rule for its whole contributing area.

4.13.3 **Fee.** The fee-in-lieu shall be computed as:

> Fee = (detention volume required on site under Clause 4.6, m³) × (unit rate, ₹/m³, notified by the Authority)

The unit rate shall include land, construction and 10 years of maintenance. Fees shall be held in a ring-fenced basin account and spent only on the facilities named in the plan.

4.13.4 **Ledger.** The Authority shall keep a public ledger of:

- detention volume credited to each regional facility;
- volume delivered; and
- fees received and spent.

> Commentary. The Malaysian DID (2008) checklist favoured community ponds over on-site detention for sites above 0.1 ha. Studies of on-site detention show that its benefit depends on location and fades with distance downstream (Colorado State University thesis; McEnroe for Kansas City), and that a pond low in the catchment can synchronise with the main-stem peak (*Water* 2016). Regional storage therefore has to be sited by basin modelling, not simply aggregated.
>
> Chhattisgarh has no tank or lake Act, and no talab, lake or reservoir in its cities was found to be operated for flood control under a documented rule [TBC]. Village tanks over which the community exercised rights of irrigation or nistar have vested in the State since 6 April 1959 (Chhattisgarh Land Revenue Code 1959, section 251), and encroachment on them is proceeded against as unauthorised occupation (section 248): after the NGT ordered the Collector to remove encroachments from ponds recorded in the revenue record at Baloda (order of 14 November 2024 in O.A. 78/2024), the Tahsildar began proceedings under that section. Public tanks vest in the Municipal Corporation under the Municipal Corporation Act 1956, but the same Act, in its Madhya Pradesh text, also lets the Commissioner require a private tank to be filled or drained (sections 82 and 202, as adapted in Chhattisgarh [TBC]), so a private talab can be lost by order, and this Manual prefers desilting and inlet screening to filling. Raipur is a city of talabs: about 130 ponds were reported in 2001 and about 37–40 remain, over about 69 ha, all of them reported in 2015 to be "polluted and heavily silted" (as reported). The State Wetland Authority ordered all the water bodies of Raipur district surveyed on 8 May 2023, but only Karbala Talab is reported to have been surveyed (as reported). Nava Raipur Atal Nagar was planned round a network of natural reservoirs, among them the Sendh, Jhanjh and Khandawa lakes, fed by storm-water channels and parks, and its development plan asks that storm water be stored in dug or natural lakes and depressions (Naya Raipur Development Plan-2031, §10.3.2); yet the State Wetland Authority had to direct NRDA to withdraw tenders for plots and residences beside the Jhanjh wetland (3 March 2025, as reported). The State's only Ramsar site is Kopra Jalashay near Bilaspur, 210 ha, a reservoir of WRD built in 1993 whose overflow runs by the Ghonga to the Maniyari and the Seonath, designated on 8 August 2025 according to the Ramsar Information Sheet and announced in December 2025 (as reported). The State's regional storage is of five kinds: the talabs and dabris of the towns and villages, such as Budha Talab and Telibandha in Raipur; the lakes and reservoirs of Nava Raipur; the dams, reservoirs and barrages of WRD, among them Ravishankar Sagar (Gangrel), Minimata Bango and the Hasdeo barrage at Darri, Kharang, Maniyari, Kelo, Kumhari and Pendrawan, the Arpa Bhaisajhar and Mongra barrages and the barrages on the Mahanadi; the voids and pit lakes of the coalfields and the ash ponds of the power stations; and the river floodplains. Four points follow:
>
> - A talab on the record may not be a working talab. No inventory of the talabs within RMC or the other Municipal Corporations, with their beds, bunds and outlets, was found [TBC]. A talab is credited only after a survey shows its bed, bund and outlet, and only for the storage above its full level.
> - The reservoirs and barrages are not spare capacity for a site. They are held for irrigation, supply and industry through the monsoon, so their storage below full level is not credited (Clause 4.13.2(c)); their releases are set by WRD for the whole basin, as when Gangrel opened all 14 gates in September 2026 (as

reported); and the Hirakud reservoir, whose backwater reaches Raigarh and Sarangarh, is operated by Odisha, and was opened at Chhattisgarh's request in August 2020 and September 2024 (as reported). Storm water spilled to a reservoir or barrage pond, or to a river below one, uses capacity that is operated for the whole basin; it can be relied on only with the written agreement of WRD for a stated flow and level.

- A mine void, pit lake or ash pond is not regional storage. A void can attenuate runoff, but water raised against an unstable high wall or into old workings is a hazard, and the lessee and DGMS control it; an ash pond is held behind a dyke whose breaches at Korba in 2024–2026 sent ash slurry onto fields and into the Hasdeo (as reported). Either is used only with the written consent of the lessee and DGMS, or of the operator for an ash pond, and with its water level, spill level and spill path surveyed (Clause 5.8.5(e)), and it is never assumed to have free storage.
- No rule dividing natural nalas from municipal drains was found [TBC]. Until one is made, this Manual treats rivers, dams, reservoirs, barrages and canals as WRD's; the nalas, drains and public tanks within the limits of a Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat as that body's; the roads, drains and lakes of Nava Raipur as NRDA's; the drains of the Bhilai Steel Plant township as SAIL's; village tanks vested under section 251 as the Collector's; mine drains, voids and pit lakes as the lessee's and ash ponds and dykes as the power company's; and Kopra Jalashay and any wetland the State Wetland Authority treats as protected as within that Authority's remit (Commentary to Clause 6.9). This is the Manual's working assignment, not a statutory one.

The first point is the basis of condition 4.13.2(c) in Chhattisgarh. A talab gives flood storage only above its full level, unless a notified operating rule commits it to be lowered before the SW monsoon. The 3-year operational limit and the 10-year maintenance allowance have no direct precedent and are set by this Manual.

4.14 Simplified methods for small sites

4.14.1 **Applicability.** Tier B local bodies may permit the simplified method for sites of **2 ha or less** outside flood-sensitive catchments. In flood-sensitive catchments, it may be used for sites of **0.5 ha or less**. All other sites shall use Clause 4.6. Until Table 4.3 (Annex 4B) is published, all Tier B sites shall also use Clause 4.6.

4.14.2 **Method.**

1. Read the PSD (L/s/ha) and SSR (m³/ha) for the site's zone, HSG and post-development imperviousness from **Table 4.3** (Annex 4B).
2. Required storage: V = SSR × A − V_credit (Clause 4.4.6.3).
3. Permissible discharges: Q_PSD,T = PSD_T × A for the 2-yr and 100-yr stages.
4. Select the low-flow orifice and upper control from the standard outlet table in Annex 4B (orifice sizes for the PSD at the stated depths), subject to the 50 mm minimum.
5. Provide a spillway or overflow under Clause 4.9, the RWH accounting of Clause 4.11, and the safety provisions of Clause 4.15.
6. Complete Form 4-2 (Tier B site sheet).

Table 4.3 — Tier B permissible site discharge and site storage requirement (template)

Zone (Annex 2A)	HSG	Imperviousness	PSD 2-yr (L/s/ha)	PSD 100-yr (L/s/ha), standard	PSD 100-yr (L/s/ha), flood-sensitive	SSR (m³/ha), standard	SSR (m³/ha), flood-sensitive
CG-Z1 Mahanadi plain: CG-A Raipur–Durg–Bhilai–Rajnandgaon plain (Seonath–Kharun, limestone)	B / C / D	50% / 70% / 90%	[TBC]	[TBC]	[TBC]	[TBC]	[TBC]

Zone (Annex 2A)	HSG	Imperviousness	PSD 2-yr (L/s/ha)	PSD 100-yr (L/s/ha), standard	PSD 100-yr (L/s/ha), flood-sensitive	SSR (m³/ha), standard	SSR (m³/ha), flood-sensitive
CG-Z1 Mahanadi plain: CG-B Bilaspur–Mungeli–Kawardha (Arpa–Maniari–Hamp)	B / C / D	50% / 70% / 90%	[TBC]	[TBC]	[TBC]	[TBC]	[TBC]
CG-Z1 Mahanadi plain: CG-C Korba–Janjgir–Champa–Raigarh (Hasdeo–Mand–Kelo coalfields)	B / C / D	50% / 70% / 90%	[TBC]	[TBC]	[TBC]	[TBC]	[TBC]
CG-Z1 Mahanadi plain: CG-D South-east plain (Dhamtari–Mahasamund–Gariaband–Rajim)	B / C / D	50% / 70% / 90%	[TBC]	[TBC]	[TBC]	[TBC]	[TBC]
CG-Z2 Bastar plateau: CG-E Bastar plateau and Godavari frontage	B / C / D	50% / 70% / 90%	[TBC]	[TBC]	[TBC]	[TBC]	[TBC]
CG-Z3 Northern hills: CG-F Northern hills (Surguja–Koriya–Jashpur)	B / C / D	50% / 70% / 90%	[TBC]	[TBC]	[TBC]	[TBC]	[TBC]

Worked example (illustrative — not design values).

A 0.4 ha residential layout in a Tier B Municipality, in a flood-sensitive catchment, with about 80% directly connected impervious area (area-weighted post-development CN about 92). Suppose Table 4.3 had been computed and gave the per-hectare equivalent of the Clause 4.6.7 example:

- PSD 100-yr = 356 / 2.0 = 178 L/s/ha
- SSR = 2,913 / 2.0 ≈ 1,460 m³/ha
- PSD 2-yr ≈ 17 L/s/ha

Then:

- Storage: V = 1,460 × 0.4 = **584 m³**. C_R = 0, so there is no retention credit.
- 100-yr release: 178 × 0.4 = **71 L/s**.
- 2-yr release: 17 × 0.4 = **7 L/s**. At about 1 m head this needs a low-flow orifice of about 60 mm, above the 50 mm minimum (Clause 4.8.2).
- Layout options:
  - a 400 m² underground tank 1.5 m deep; or
  - 250 m² of podium storage at 0.3 m (75 m³) plus a 350 m² tank 1.5 m deep (525 m³).
  - Rooftop storage at 150 mm over 1,200 m² of roof (180 m³) could replace part of the tank.
- RWH sumps are additional, and are assumed full (Clause 4.11).

> **Commentary.** Australian councils use permissible site discharge and site storage requirement tables in the same way (Sydney Water OSD guide; the Upper Parramatta River Catchment Trust handbook listed SSR of about 455 m³/ha). The illustrative SSR here, about 1,460 m³/ha, is much larger. That is the expected effect of combining a mixed-frequency cap with mandatory 48–72-hour storms at AMC III. MSMA 2 in Malaysia offers a similar simplified OSD method for sites under 5 ha. The MARC policy supplement offers smaller Kansas City municipalities adoptable alternatives within the regional standard; Clause 4.14 follows that model for Tier B.

4.14.3 **Fixed-coefficient route.** As an alternative to Clauses 4.6 and 4.14.2, the designer of any site of **1 ha or less**, under any Approving Authority and in any catchment, may size detention by Clauses 4.14.4 to 4.14.7. The route shall not be used where:

- (a) the site counts tank or waterbody storage in its design (Chapter 7);
- (b) any part of the site lies within a waterbody setback (Chapter 7); or
- (c) the site drains to a tank cascade whose storage is counted in a downstream design.

The route sets the release caps by fixed coefficients and does not need Table 4.3. It may therefore be used before Annex 4B is published. Retention (Clause 4.4), drawdown (Clause 4.7), outlets (Clause 4.8), the spillway (Clause 4.9), the downstream and tailwater checks (Chapter 5) and water quality (Chapter 8) still apply.

4.14.4 **Release caps.** Two caps apply at each point where runoff leaves the site:

$Q_{cap,10} = 2.78 \times C_{10} \times i_{10} \times A$ and $Q_{cap,2} = 2.78 \times C_2 \times i_2 \times A$

where Q_{cap} is in L/s; A is the site area (ha); i_{10} and i_2 are the 10% AEP (10-yr) and 50% AEP (2-yr) rainfall intensities (mm/h) for a **15-minute** duration at the site, from the sources of Chapter 2, with the climate factor of Clause 2.8; and C_{10} and C_2 are the fixed coefficients of Table 4.3A for the site's hydrologic soil group (Clause 3.7). For a site with more than one soil group, the coefficients are area-weighted.

Table 4.3A — Fixed release coefficients (provisional)

Hydrologic soil group	C_{10}	C_2
A (sandy, well drained)	0.05	0.02
B	0.08	0.03
C	0.15	0.08
D (clay, poorly drained)	0.25	0.15

$Q_{cap,10}$ is the maximum release for **every storm up to and including the 1% AEP (100-yr) storm**, in every catchment. $Q_{cap,2}$ is the maximum release for the 50% AEP (2-yr) storm. Where a cap is smaller than the discharge of a 50 mm orifice at the corresponding stage, the 50 mm orifice is deemed to comply (Clause 4.5.5.1).

4.14.5 **Storage.** The detention storage V (m³) is the larger of V_{100} and V_2, each the largest over the durations d of Clause 2.5.2 (1 to 72 hours):

$V_{100} = \max_d [\, 10 \times C_v \times P_{100}(d) \times A - 1.8 \times Q_{cap,10} \times d \,]$

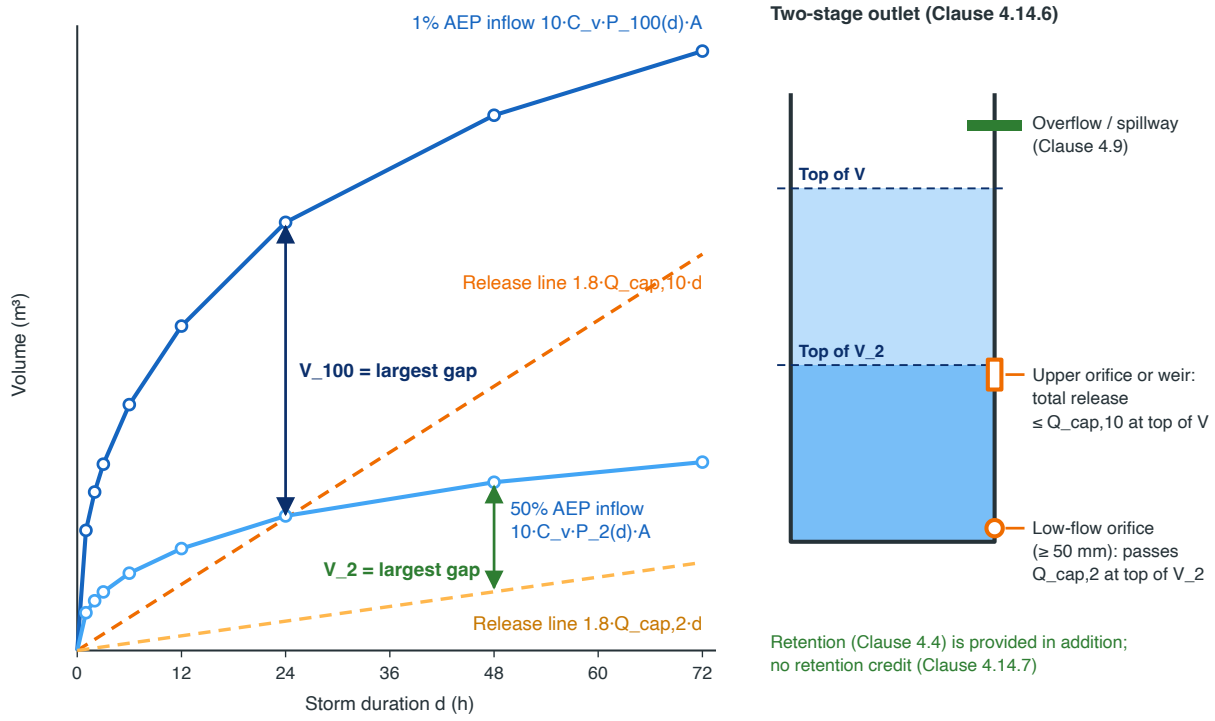
$V_2 = \max_d [\, 10 \times C_v \times P_2(d) \times A - 1.8 \times Q_{cap,2} \times d \,]$

where $P_{100}(d)$ and $P_2(d)$ are the 1% and 50% AEP rainfall depths (mm) for duration d (hours), with the climate factor; and C_v is the post-development volumetric coefficient: the composite C of Table 3.2 (upper half of each range), multiplied by Cf of Table 3.3 for the 1% AEP. For durations of 12 hours or more, C for every pervious surface shall be taken as not less than **0.50**. The factor 1.8 assumes an average release of half the cap over the storm (3.6 × 0.5).

4.14.6 **Outlet.** The outlet shall be a two-stage control: a low-flow orifice that passes $Q_{cap,2}$ when the storage holding V_2 is full, and an upper orifice or weir that limits the total release to $Q_{cap,10}$ at the top of V. The outlet table of Annex 4B may be used once it is published.

4.14.7 **Retention.** The retention volume of Clause 4.4 is provided in addition to V. No retention credit (Clause 4.4.6) is taken under this route.

Fixed-coefficient route for sites of 1 ha or less (Clauses 4.14.3–4.14.7) — schematic, not to scale



Dots: durations of Clause 2.5.2 (1, 2, 3, 6, 12, 24, 48, 72 h). Pervious C ≥ 0.50 for d ≥ 12 h.

$Q_{cap,10} = 2.78 \times C_{10} \times i_{10} \times A$; $Q_{cap,2} = 2.78 \times C_2 \times i_2 \times A$; (L/s; A in ha; i in mm/h)

C_{10}, C_2 from Table 4.3A by soil group; i_{10}, i_2 for a 15-minute duration with the climate factor (Clause 2.8)

$V = \max(V_{100}, V_2)$; $V_T = \max$ over d of $[\ 10 \times C_v \times P_T(d) \times A - 1.8 \times Q_{cap} \times d\ ]$ (m^3) ; 1.8 = 3.6 × 0.5 (average release of half the cap)

Figure 4.3 — Fixed-coefficient route for sites of 1 ha or less (Clauses 4.14.3 to 4.14.7): the release caps Q_cap,10 and Q_cap,2 from Table 4.3A; the storage V as the largest gap, over the durations of Clause 2.5.2, between the cumulative inflow 10·C_v·P(d)·A and the release line 1.8·Q_cap·d, for the 1% and 50% AEP storms; and the two-stage outlet that passes Q_cap,2 at the top of V_2 and Q_cap,10 at the top of V. Schematic, not to scale. Original drawing for this Manual, adapted from the rainfall mass-curve (inflow minus outflow) storage concept and the single-coefficient peak cap of the PUB Code of Practice on Surface Water Drainage, 7th ed. (cl. 7.1.5).

Commentary. The fixed-coefficient route follows Singapore, which caps the peak runoff from every development of 0.2 ha or more with one runoff coefficient, C = 0.55, applied to the 10-yr storm (PUB Code of Practice on Surface Water Drainage, 7th ed., cl. 7.1.5, Addendum No. 3, 2025). It is quick to apply and quick to check: the cap needs only the site area, the soil group and two intensities. PUB's 0.55 is not used here, because Singapore's baseline is the existing urban drainage system, while this Manual's baseline is greenfield land (Clause 4.5.1.2). The coefficients of Table 4.3A are set by this Manual from the greenfield curve numbers of Clause 3.3 (CN 39, 61, 74 and 80 for soil groups A to D). They are the ratios of runoff to rainfall that those curve numbers give for 1-hour depths typical of the Chhattisgarh plain, rounded down. They are therefore **provisional**. The State Stormwater Technical Committee shall confirm or revise them against NIH-DHAARA values for each zone when it issues Annex 4B.

The route is deliberately **stricter** than the full method, so that simplicity costs the developer storage rather than costing the people downstream. The 100-yr release is held to about the greenfield 10-yr peak everywhere, which is the flood-sensitive rule of Clause 4.5.3; the storage assumes the outlet releases only half the cap on average; pervious ground is assumed wet in long storms; and no retention credit is taken. A designer who wants a smaller tank can use the full method of Clause 4.6. The storage formula is the rainfall-mass (inflow minus outflow)

method. Clause 3.4.2 otherwise forbids rainfall-based peak methods for storage, because they ignore the shape of the hydrograph; this route is a stated exception because its conservative assumptions cover that error for small sites, and the 24- to 72-hour durations are always checked.

4.15 Safety, fencing and multi-use

4.15.1 **Depth limits for publicly accessible storage.** Where the public has access to a storage area (parks, playfields, open space, school grounds):

- (a) maximum depth in the 10% AEP storm: **0.45 m**;
- (b) maximum depth in the 1% AEP storm: **1.2 m**, except in a fenced zone under 4.15.3;
- (c) side slopes: no steeper than **4H:1V**, with at least a quarter of the perimeter at **6H:1V** or flatter for escape; and
- (d) no vertical drop greater than **0.3 m** within the storage area or at its edge.

4.15.2 **Wet basins.** Permanent pools shall have:

- a safety bench at least 3 m wide, no more than 0.3 m below the normal water level; and
- an emergent planting fringe that deters entry.

4.15.3 **Fencing.** A fence at least **1.2 m** high, of a non-climbable design with a lockable gate, is required around any part of a storage that:

- has a vertical wall or headwall drop greater than **0.6 m**;
- exceeds 1.2 m depth in the 1% AEP storm with slopes steeper than 4H:1V; or
- contains an outlet structure with an opening larger than 300 mm that is not grated.

4.15.4 **Outlets.** Every inlet and outlet opening larger than 300 mm accessible to the public shall be fitted with a grate or rack. The grate shall be designed so that a person pinned against it can escape: low approach velocity, inclined, with a large open area.

4.15.5 **Signs and depth markers.** Every publicly accessible storage shall have signs in Hindi and English stating that the area floods during rain, and a depth marker at the deepest point.

4.15.6 **Confined spaces.** Underground tanks shall have lockable covers. They shall be marked as confined spaces, and the maintenance declaration (Chapter 10) shall state the entry procedure.

4.15.7 **Vector control.** Dry storage shall drain completely (Clause 4.7) and have no low points that pond for more than 72 hours. Wet basins shall meet Clause 4.10.7.

> **Commentary.** The numeric limits in Clause 4.15 have no single direct precedent. They are drawn from common international practice, for example KC APWA 5600 (2006), which required at least a quarter of a dry-pond perimeter at 5:1 or flatter, a 20 ft (6 m) clearance from property lines and a 15 ft (4.6 m) maintenance strip. They are made somewhat stricter here because multi-use storage on open space land and in schools will be used by children during the monsoon. Standing water in dry basins is a mosquito-breeding risk, so complete drainage (Clause 4.15.7) is a public-health requirement, not an amenity.

4.16 Basis: why peak matching alone fails

Commentary. This section has no requirements. It records why this chapter requires release **below** pre-development rates, retention, long-duration checks and a downstream analysis, rather than simple peak matching.

1. **Timing.** Detention lowers a site's peak but stretches its hydrograph (Figure 4.1). Downstream, the stretched releases from many sites overlap with each other and with the main-stem peak. In Denver's study, ten basins each releasing 100% of the historic 100-yr peak produced 106% of the historic peak at the outlet. Only releasing 90% at each site brought it to 97% (UDFCD memo, 2012). A Colorado State University thesis found that location-blind on-site detention "can actually increase peak flows above post-development peaks". An optimisation study found that a downstream pond performed worse than upstream ponds because its outflow synchronised with the main stem (Water 2016, 8(7):290).
2. **Volume.** Detention does not remove volume. In the illustrative example, the post-development 100-yr storm produces about three times the runoff volume of the pre-development 10-yr storm (Clause 2.11). Only retention, or storage that empties to somewhere other than the receiving drain, reduces volume.
3. **Scale.** Kansas City modelling (McEnroe, 2005) found that uniform 3 cfs/ac control reduced Q100 by 36% in a 200-ac watershed. With the same control everywhere, Q100 still rose by 40% at 14 sq mi, and the benefit faded beyond about 1–2 sq mi. The KC committee concluded that "'match existing' can harm streams" (MARC Evolution deck). Georgia makes a ten-percent downstream analysis mandatory for the same reason: "Due to peak flow timing and runoff volume effects, some structural practices fail to reduce discharge peaks to pre-development levels downstream" (GSMM 2016).
4. **Design-storm dependence.** An Australian study of 20 real on-site detention systems found that designs using even ten temporal patterns "cannot reliably produce an OSD system design that will always be effective" under real rainfall (Water Science and Engineering, 2019). This is why Clause 4.6.3 routes the full duration matrix rather than one critical storm.
5. **Closed outfalls.** In Chhattisgarh, the governing failure is volume with no outlet when a river, reservoir or talab downstream is high, and on the plain it lasts. Raipur drains to the Kharun, which on 29 August 2020 set a new highest level of 281.68 m at Pathardihi and stayed above its previous highest level for about 46 hours, while the Mahanadi at Kalma stayed above its previous highest level for about 110 hours (CWC, as transcribed by SANDRP); and in Chaubey Colony five ponds overflowed into homes on 4–5 July 2026 (as reported). The cities below dams and barrages drain to rivers whose levels are set by releases: Korba to the Hasdeo, when Bango released about 30,000 cusecs through six gates and the Darri barrage about 20,000 cusecs on 24–26 August 2024 and houses in the town were flooded (as reported); Dhamtari, Rajim and the villages along the Mahanadi to the river below Gangrel, which released about 55,000 cusecs through eight gates in early October 2025 and opened all 14 gates on 24–27 September 2026 (as reported); and Bilaspur to the Arpa below the Bhaisajhar barrage. Jagdalpur drains to the Indravati, which stood above its danger level of 540.80 m from 29 July to 8 August 2019 (CWC) and reached 544.891 m on 26 September 2026 (as reported), with dams upstream in Odisha. The low outfalls of Raigarh and Sarangarh lie within reach of the backwater of the Hirakud reservoir in Odisha, whose gates were opened at Chhattisgarh's request in August 2020 and September 2024 (as reported). No hourly record of how long a river, reservoir or talab in Chhattisgarh stays above a city's outfalls was obtained [TBC], and no document was found that sets a design river level for an outfall; Chapter 5 sets the design level and the closure to be assumed. A perfectly orificed basin behind a closed outfall simply fills. No release-rate rule can fix that; it is handled by the outfall-closed check of Chapter 5.

The response in this Manual is layered:

- release below pre-development (Clause 4.5);
- retain the frequent storms (Clause 4.4);
- route long SW monsoon storms (Clause 4.6.3);
- prefer regional storage in the talab system (Clause 4.13); and
- check downstream and at the outfall (Chapter 5).

> Kansas City's own history shows the price. On a test site, peak matching needed 3.0 ac-ft of storage and the 2011 comprehensive criteria needed 4.9 ac-ft (MARC Evolution deck). Over-detention costs more storage. That is the cost of protecting the catchment rather than only the site boundary.

4.17 Submittal requirements

Every site runoff-control submittal shall include a **Site Runoff Control Report** (Form 4-1) containing:

1. the Design Rainfall Sheet (Form 2-1) and the hydrology inputs (Chapter 3);
2. the evidence for the order of measures under Clause 4.3.1;
3. the RRV computation, infiltration tests, groundwater evidence and drawdown calculation (Clause 4.4);
4. the retention credit computation (Clause 4.4.6);
5. the release-rate criterion applied, standard or flood-sensitive, with the Annex 4A map extract (Clause 4.5);
6. a results table in the format of Clause 4.6.7 for every AEP and duration, with the governing case highlighted;
7. stage–storage and stage–discharge tables and plots, and inflow/outflow hydrographs for the governing cases;
8. the drawdown calculation (Clause 4.7);
9. outlet details, including orifice sizes and levels, trash racks and anti-vortex devices (Clause 4.8);
10. the spillway or overflow calculation, freeboard and hazard classification, and the geotechnical report where required (Clause 4.9);
11. the RWH and detention accounting, with tank sections showing zone levels (Clause 4.11);
12. the design groundwater level and flotation check (Clause 4.12);
13. the downstream and tailwater report (Chapter 5, Form 5-1); and
14. the safety provisions (Clause 4.15) and the draft maintenance declaration (Chapter 10).

Tier B sites using Clause 4.14.2 shall submit Form 4-2 in place of items 6 and 7. Sites using the fixed-coefficient route (Clause 4.14.3) shall submit Form 4-3 in place of items 6 and 7.

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Annexes and forms

Annex / Form	Content
4A	Flood-sensitive catchment boundaries (GIS); shallow-groundwater zone map
4B	Tier B tables: PSD and SSR (Table 4.3), retention credit factor C_R, standard outlet sizes; confirmed fixed release coefficients (Table 4.3A)
4C	Worked example (Tier A) with full routing tables
Form 4-1	Site Runoff Control Report
Form 4-2	Tier B Site Sheet
Form 4-3	Fixed-coefficient Route Sheet (Clauses 4.14.3 to 4.14.7)

CHAPTER 5

Downstream and Tailwater Analysis

5.1 Purpose and scope

5.1.1 This chapter sets two checks that every regulated development and every public drainage project must pass.

- **The downstream analysis** (Clauses 5.3 to 5.5) confirms that the project does not make flooding worse below its outfall.
- **The tailwater analysis** (Clauses 5.6 to 5.13) sets the water level the system discharges into, including tide, storm surge, sea-level rise, river backwater and sand-bar closure. It also requires the system to cope when its outfall is closed.

5.1.2 This chapter applies to:

- (a) every site to which Chapter 4 applies;
- (b) every minor- and major-system drainage project under Chapter 6; and
- (c) every regional detention or tank restoration project under Clause 4.13 and Chapter 7.

5.1.3 Rainfall and joint-occurrence inputs come from Chapter 2, in particular Clauses 2.5.3 and 2.12. Hydrologic methods come from Chapter 3. Site release criteria come from Chapter 4. Hydraulic design of gates, culverts and pumps follows Chapter 9.

5.1.4 **No tidal outfalls in Chhattisgarh.** Chhattisgarh is landlocked: it has no coast, no tide and no Coastal Regulation Zone. A Bay of Bengal depression or cyclone reaches the State only as rainfall, and that rainfall is dealt with in Chapter 2, not as a tidal or surge level. No outfall in the State discharges to the sea, a tidal creek or an estuary, so the provisions of this chapter that depend on tide, storm surge, sea-level rise or a river mouth do not apply in Chhattisgarh: Clause 5.7 and Annex 5A, cases J1 to J3 of Table 5.2, the tidal lock windows of Clause 5.10.3, the seawater requirement of Clause 5.11.3(a) (gates and seats shall still be of corrosion-resistant materials), and Clause 5.13. They are kept so that the clause numbers of this Manual match its other State editions. In Chhattisgarh the receiving water is one of four kinds: a river, reached directly, through a gate in a bank or embankment or through a pumping station, whose level is often set by the releases of a dam or barrage upstream, as on the Mahanadi below Gangrel, the Hasdeo at Korba below the Minimata Bango dam and the Darri barrage, and the Arpa at Bilaspur below the Bhaisajhar barrage, or by a flood from a catchment that lies partly in Odisha, as on the Indravati at Jagdalpur; a reservoir, barrage pond, lake or talab held at its full level, such as the talabs of Raipur, the lakes and reservoirs of Nava Raipur Atal Nagar, the Kelo dam above Raigarh and the barrage ponds of the Mahanadi and the Seonath; the backwater of the Hirakud reservoir in Odisha, which reaches the low land of Raigarh and Sarangarh-Bilaigarh districts; or, in the coalfields and around the power plants of Korba, Raigarh and Surguja, a mine void, pit lake or ash pond, which is not a design outfall at all unless its operator, and for a mine DGMS, agree in writing and its level and spill are surveyed; and:

- (a) its tailwater is set by Clauses 5.6 and 5.8, including Clause 5.8.5;
- (b) where Clauses 5.10 to 5.12, Form 5-1 or Chapter 6 refer to the J1 tailwater or the J1 tidal curve, the minor-system tailwater of Clause 5.6.2 or 5.8.5 applies, as a time series where the receiving level varies; where they refer to the J2 tailwater or to HAT + surge + SLA, the major-system tailwater applies: the design-AEP river level of Clause 5.8.4(b), or the major-system tailwater of Clause 5.8.5, whichever is higher at the outfall; and

- (c) the outfall-closed check of Clause 5.10 applies where an outfall is closed by a river, a reservoir, barrage pond or talab, the Hirakud backwater, a mine void or ash pond or a receiving drain, with the river lock of Clause 5.8.5(a) and the closures of Clause 5.8.5(b), (d), (e) and (g) in place of the tidal lock windows.

Commentary. The drains of Chhattisgarh's towns fail in places that site design does not see, and in four different ways. The first is the river, often below a dam. Raipur drains to the Kharun, which set its highest recorded level at Pathardihi, 281.68 m, on 29 August 2020, 2.47 m above the record of 1994 (CWC data, as copied by SANDRP), and cut the Raipur–Bhilai road at Mahadev Ghat in September 2024 and overflowed there again in September 2026 (as reported). Dhamtari, Rajim and the towns down the Mahanadi lie below the Gangrel dam, all 14 of whose gates were opened in September 2026 (as reported); Korba lies on the Hasdeo below the Minimata Bango dam and the Darri barrage, whose releases of August 2024 were followed by flooding in the low parts of the town (as reported), and the Hasdeo set new highest levels at Katghora and Bangodam in July and August 2026; Bilaspur lies on the Arpa below the Bhaisajhar barrage; and Jagdalpur lies on the Indravati, which rises in Odisha and set a new highest level at Jagdalpur, 544.891 m, on 26 September 2026, 0.21 m above that of 1973 (CWC data, as copied by SANDRP). The second is the reservoir, barrage pond or talab. Raipur grew around its talabs, of which about 130 are reported to have existed and about 40 to remain (as reported), and on 5–6 July 2026 five ponds in Chaubey Colony overflowed into homes (as reported); Nava Raipur Atal Nagar was planned around a network of lakes and reservoirs, reported as 55, into which its parks and channels carry storm water (as reported); and a reservoir, barrage pond or talab at its full level holds every drain that ends in it. The third is the Hirakud reservoir in Odisha, whose submergence partly takes in villages in Chhattisgarh, 33 of 174 by one count (as reported), and whose backwater sets the level of the low outfalls of Raigarh and Sarangarh-Bilaigarh districts. The fourth is the coalfield and the power plant. In Korba and Raigarh a drain can end in a mine void or run towards a working mine, and the ash dykes of the power plants have breached: at NTPC's Dhanras dyke in July 2024, and at the Korba West dyke of CSPGCL near Jhabu on 19 April 2026, where one person died (as reported). The outfalls also belong to one body while the drains above them belong to others: the rivers, dams and barrages are held by WRD and Hirakud by the Government of Odisha, the drains by the Municipal Corporations, RDA, NRDA, the Bhilai Steel Plant and the coal and power companies, and no rule divides them (Chapter 6, Clause 6.9). No document found during research sets a design level for any river, reservoir, talab, void or ash-pond outfall in the State, and the Indravati at Jagdalpur is the only gauge in the State for which a CWC warning and danger level were found. A detention basin that meets every release rate in Chapter 4 still fails if the river it drains to stands above its outfall for days, if the reservoir or talab is full, or if the void it ends in has no outlet. This chapter makes these failure modes part of the design. Chhattisgarh has no tidal outfall, so Clause 5.1.4 says which of the chapter's coastal provisions fall away and what takes their place.

5.2 Definitions

Term	Meaning in this Manual
Analysis point	A location on the downstream system where pre- and post-project conditions are compared (Clause 5.3.3).
Contributing catchment	The whole area draining to an analysis point, on and off site.
Major drain	A channel listed as a major drain in the basin plan or Annex 5B, or any river, WRD canal, the Kharun at Raipur and the nalas of the city that run to it, among them the Goverdhan nala at Mahadev Ghat, the streams and channels that link the lakes and reservoirs of Nava Raipur Atal Nagar, the Seonath and the Shankar and Pulgaon nalas of Durg, the nalas of Bhilai and of its steel-plant township, the Arpa at Bilaspur and the nalas that run to it, the Hasdeo and the nalas of Korba, the Kelo at Raigarh, the Indravati at Jagdalpur, any drain or canal of WRD, any drain built or held by a Municipal Corporation, a Nagar Palika Parishad, a Nagar Panchayat, RDA, NRDA, the Chhattisgarh Housing Board or the Bhilai Steel Plant with an internal width of 1.0 m or more, the garland drains and diversion channels of a mine, the drains of an ash pond, any irrigation canal that receives storm water, or a tidal creek.

Term	Meaning in this Manual
Chart datum (CD)	The datum of the published tide tables and nautical charts, to which tidal heights are referred.
Mean sea level (MSL)	Average sea level over a long period at a tide gauge.
MHWS	Mean high water springs: the average of high waters at spring tides.
HAT	Highest astronomical tide: the highest level predicted under average meteorological conditions over a full tidal cycle (18.6 years).
Storm surge	The rise in sea level above the astronomical tide caused by wind and low pressure, mainly during cyclones.
Still-water level	Astronomical tide plus surge, excluding individual waves.
Sea-level allowance (SLA)	The allowance added to present-day tidal levels for sea-level rise over the design life (Clause 5.7.4).
Tailwater	The water level at the downstream end of a drain, outlet or storage.
Tide-lock	The condition in which the tailwater is at or above the upstream water level, so a gravity outfall cannot discharge.
Lock window	A period during which the outfall-closed check of Clause 5.10 sets gravity discharge to zero.
Sand bar	A bar of beach sand that partly or fully closes a river mouth or creek inlet.
Project datum	The levelling datum used on the project drawings. It must be tied to a Survey of India benchmark (Clause 5.7.2).

5.3 Extent of the downstream analysis

5.3.1 The downstream analysis shall start at each site or project outfall. It shall continue downstream to the **first** of:

- (a) the point at which the project area is **10% or less** of the contributing catchment;
- (b) the next **tank, talab** or other waterbody with a surplus weir or controlled outlet; or
- (c) the next **major drain**, or the sea.

The end point itself shall be analysed (Figure 5.1).

5.3.2 Where the analysis ends at a tank or talab under 5.3.1(b), it shall report:

- the change in the tank's maximum water level; and
- the change in surplus-weir discharge, for the AEPs and durations of Clause 5.4.3.

If the maximum water level rises, the analysis shall continue below the surplus weir to the next end point.

5.3.3 Analysis points shall be set at least at:

- (a) each site outfall;
- (b) each confluence with a drain or channel whose contributing catchment is at least half that of the receiving drain at that point;
- (c) each culvert, bridge or road crossing;
- (d) each location recorded as flood-prone by the local body, RMC, CGSDMA or WRD; and
- (e) each building or road segment lying within the 1% AEP flood extent along the route. Representative points may be used where buildings are closely spaced.

5.3.4 **Small sites.** For sites of less than **0.5 ha**, the Authority may accept a simplified downstream statement (Form 5-2) in place of the full analysis, where all of the following hold:

- the receiving public drain has documented capacity for its minor-system design flow (Chapter 6);
- no flood-prone location is recorded within 500 m downstream; and
- the site does not drain to a tidal or backwater-affected outfall.

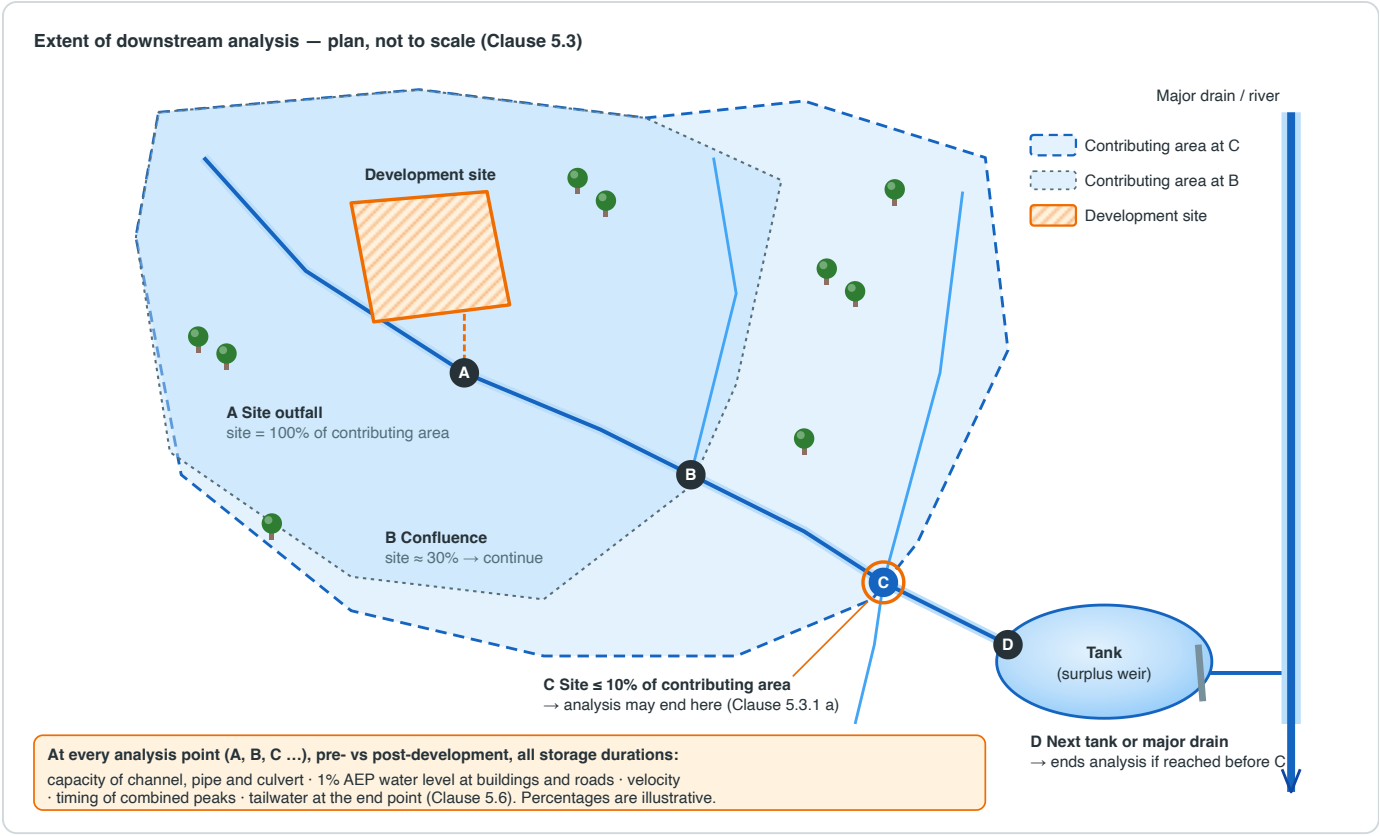


Figure 5.1 — Extent of downstream analysis: analysis points A–D, the contributing area at each point, the 10% point, and the next talab or major drain as an alternative end point. Schematic; percentages are illustrative. Original drawing for this Manual, adapted from the ten-percent rule in the Georgia Stormwater Management Manual (2016, Vol. 2 §3.1.9) and KC APWA Section 5600 (2006, 5601.5).

Commentary. The 10% rule has established precedent. KC APWA 5600 (2006) required the downstream system to be analysed "to the point the development's land mass is less than 10% of the total watershed". The Georgia manual makes the ten-percent analysis a mandatory standard (GSMM 2016, Standard #8). The tank or major-drain end point has no direct precedent; this Manual adds it for Chhattisgarh. In Raipur, where the nalas of the city run past and through its talabs to the Kharun, to which the city's sewage network is reported to be linked through eight big canals (as reported), and the Kharun runs on below the city to the Seonath, and in Nava Raipur Atal Nagar, where four major streams divide the area into four catchments and link a network of lakes and reservoirs into which the parks and channels carry storm water (NRDA, Naya Raipur Development Plan-2031; as reported), the 10% point can lie many kilometres downstream, across several tanks. A tank with a working surplus weir changes the hydrograph completely, so the analysis is better ended there and the tank level reported. Clause 5.3.2 prevents that end point from hiding a rise in the tank level. The small-site statement (5.3.4) and its 500 m distance have no direct precedent and are set by this Manual.

5.4 What the analysis must check

5.4.1 **Scenarios.** Each analysis shall compare:

- (a) **Existing**: the present downstream system and present land use of the whole contributing catchment, with the project site at its pre-project state; and
- (b) **With project**: the same, with the project and all its controls (Chapter 4) in place.

Where the Authority's basin plan defines future land use, the Authority may also require:

- (c) **Future**: the catchment fully developed under the plan, all to Chapter 4 standards, with and without the project.

5.4.2 **Quantities to be checked at every analysis point:**

- (a) **Capacity**: peak flow against the capacity of every channel, pipe, culvert and crossing, for the minor-system AEP of that element (Chapter 6) and for 1% AEP;
- (b) **Flood levels**: 1% AEP maximum water level at buildings (at the lowest opening) and on roads (at the crown and at the deepest point);
- (c) **Velocities**: peak velocity for 50%, 10% and 1% AEP;
- (d) **Timing of combined peaks**: the hydrograph at each confluence, showing the time to peak of the project outflow and of the receiving drain; and
- (e) **Duration of inundation**: on roads and at buildings, the time above the Chapter 6 depth limits.

5.4.3 **Storms.** The analysis shall use:

- 50%, 10% and 1% AEP;
- the duration set of Clause 2.5.2, at least the durations that give the highest level at each analysis point (these will usually be longer than at the site); and
- the 24-, 48- and 72-hour storms in every case.

The AMC rule of Clause 2.11.4 applies. Areal reduction follows Clause 2.7, applied to the contributing catchment at each analysis point and never to the site storage.

5.4.4 **Method.** The analysis shall:

- use a hydrograph method that combines hydrographs at confluences (SWMM, HEC-HMS, or an equivalent);
- include a hydraulic model with backwater (SWMM dynamic wave, HEC-RAS, or equivalent) wherever tailwater or backwater affects any analysis point; and
- not use the Rational Method at any analysis point, because it cannot represent timing.

Off-site areas shall be modelled at their present land use. Upstream detention shall be included only where it is built, maintained and documented.

5.4.5 **Tailwater.** The downstream end of the model shall use the tailwater of Clauses 5.6 to 5.9 and the sand-bar condition of Clause 5.13 where applicable.

> **Commentary.** Clause 5.4.2(d) exists because detention changes when the site's water arrives, not only how much arrives (Clause 4.16). A Colorado State University thesis and a *Water* (2016) optimisation study both found that delayed site releases can coincide with the main-stem peak and raise it. Georgia's manual states the result directly: some structural practices "fail to reduce discharge peaks to pre-development levels downstream" because of "peak flow timing and runoff volume effects" (GSMM 2016). The duration-of-inundation check (5.4.2(e)) matters in the river towns of Chhattisgarh, whose outfalls lie below rivers held up for days by floods and by the releases of dams and barrages: on 28–29 August 2020 the Mahanadi at Kalma rose above its previous highest level and stayed above it for about 110 hours, and the Kharun at Pathardihi for about 46 hours (CWC data, as copied by SANDRP); from 24 to 27 September 2026 all 14 gates of the Gangrel dam were open, the Kharun

overflowed at Mahadev Ghat and the Indravati set a new highest level at Jagdalpur (as reported); and in Raipur on 5–6 July 2026 five ponds in Chaubey Colony overflowed into homes and sewage entered houses (as reported). Detention that prolongs releases can lengthen that time even when it lowers the peak.

5.5 Acceptance criteria

5.5.1 The project is acceptable if, at every analysis point and for every AEP and duration of Clause 5.4.3, comparing "with project" to "existing":

- (a) the 1% AEP maximum water level does not rise by more than **10 mm** at any habitable building, at any building's lowest opening, or on any road carriageway;
- (b) no building that is not flooded in the existing case is flooded with the project, and no building or road that meets the Chapter 6 freeboard or depth limits in the existing case fails them;
- (c) peak flow does not exceed the existing peak at the minor-system AEP of each element, or at 1% AEP;
- (d) peak velocity in lined channels stays within the permissible velocity for the lining (Chapter 6: 3.0 m/s maximum); and
- (e) in unlined or natural channels, peak velocity for 50% and 10% AEP does not increase by more than **10%**, unless protection is provided under Chapter 9.

5.5.2 The duration of inundation under 5.4.2(e) shall be reported. The Authority may refuse a project that increases the duration of road closure on an arterial or emergency route.

5.5.3 **Where the criteria are not met**, the project shall do one or more of the following:

- (a) reduce release rates or increase storage (Chapter 4);
- (b) build downstream improvements, with the consent of the owner of the downstream system. These shall not transfer the problem further downstream, and shall be re-analysed under this chapter;
- (c) contribute to a regional facility under Clause 4.13; or
- (d) under Clause 5.5.4, change the release strategy.

5.5.4 **Timing exception.** Where the analysis shows that detention raises downstream levels because of timing, the Authority may vary the site release-rate criteria of Clause 4.5. This applies only where the site discharges within the lower tenth of the contributing catchment, directly to a major drain, tidal creek or the sea, and only on the following conditions:

- (a) the varied strategy meets 5.5.1;
- (b) the retention requirement (Clause 4.4) and WQv treatment (Chapter 8) are met in full; and
- (c) the outfall-closed check (Clause 5.10) is met.

Commentary. The 10 mm tolerance (5.5.1(a)) has no direct precedent. It is meant to be at or below the numerical precision of a typical one-dimensional model, so that "no increase" can be tested without arguing over rounding. The 10% velocity tolerance is likewise set by this Manual. The timing exception in 5.5.4 follows the Georgia manual, where the downstream analysis can show "that peak controls are not needed" and allow them to be waived (GSMM 2016, Standard #8). It is limited here to sites low in the catchment, which is where synchronisation with the main-stem peak is most likely (*Water* 2016, 8(7):290). It is not available above that, because the evidence on scale (Clause 4.16) shows that early release from many upstream sites raises peaks.

5.6 Tailwater: general and non-tidal outfalls

5.6.1 Every drain, outlet and storage shall be designed against a stated tailwater. A free outfall shall be assumed only where the outlet invert is above the maximum receiving water level for the case being checked.

5.6.2 **Minor-system tailwater, non-tidal.** The tailwater is the receiving system's water level at the time of the minor-system peak, from the downstream model. Where no model exists, use:

- for a receiving pipe, its obvert; or
- for a receiving open drain, its full-bank level less freeboard (Chapter 6).

5.6.3 **Major-system tailwater, non-tidal.** The tailwater is the receiving system's 1% AEP water level, unless Clause 5.8 allows a coincident lower level.

5.6.4 **Tidal and backwater outfalls.** Clauses 5.7 to 5.13 apply in addition.

5.6.5 **Storage elements.** Detention, tanks and pumped sumps shall be routed with a time-varying tailwater (Clause 4.6.6), not a constant level, wherever the tailwater varies with tide or with the receiving system's hydrograph.

5.7 Tidal tailwater: datums, surge and sea-level allowance

5.7.1 **Design tailwater levels.** For outfalls to the sea, tidal creeks, estuaries, backwaters and tidal reaches of rivers and canals:

Table 5.1 — Design tailwater levels for tidal outfalls

Design case	Tailwater
Minor-system design (Chapter 6)	**MHWS + SLA**
Major-system design and storage design	**HAT + surge + SLA**, with the surge term defined in Clause 5.7.3
Joint case (Clause 2.12(b))	1% AEP still-water level + SLA

The SLA is **+0.3 m minimum** (Clause 5.7.4). The cases to be run are listed in Clause 5.9.

5.7.2 **Sources and datums.**

- (a) **Tidal levels.** MHWS, HAT and MSL shall be taken from the current edition of the Indian tide tables published by the National Hydrographic Office (NHO) or by the Survey of India, for the nearest standard or secondary port, or from the port authority's published tidal levels. The edition and port shall be stated.
- (b) **Datum conversion.** Tide-table heights are referred to chart datum. They shall be converted to the project datum through a documented connection between chart datum, the tide gauge zero and a Survey of India benchmark. The offset and its source shall be stated on Form 5-1.
- (c) **Estuaries, creeks and canals.** For outfalls inside estuaries, creeks, backwaters or tidal canals (for example the estuaries of the coastal States; there is none in Chhattisgarh, Clause 5.1.4), tidal levels may differ from the open coast. Open-coast levels shall be used unless measured levels over at least one spring–neap cycle, or a calibrated hydrodynamic model, justify a different value. No reduction below open-coast levels shall be taken without such evidence.
- (d) **Interim.** Until Annex 5A is published, the designer shall obtain the values directly from the sources in (a) and record them.

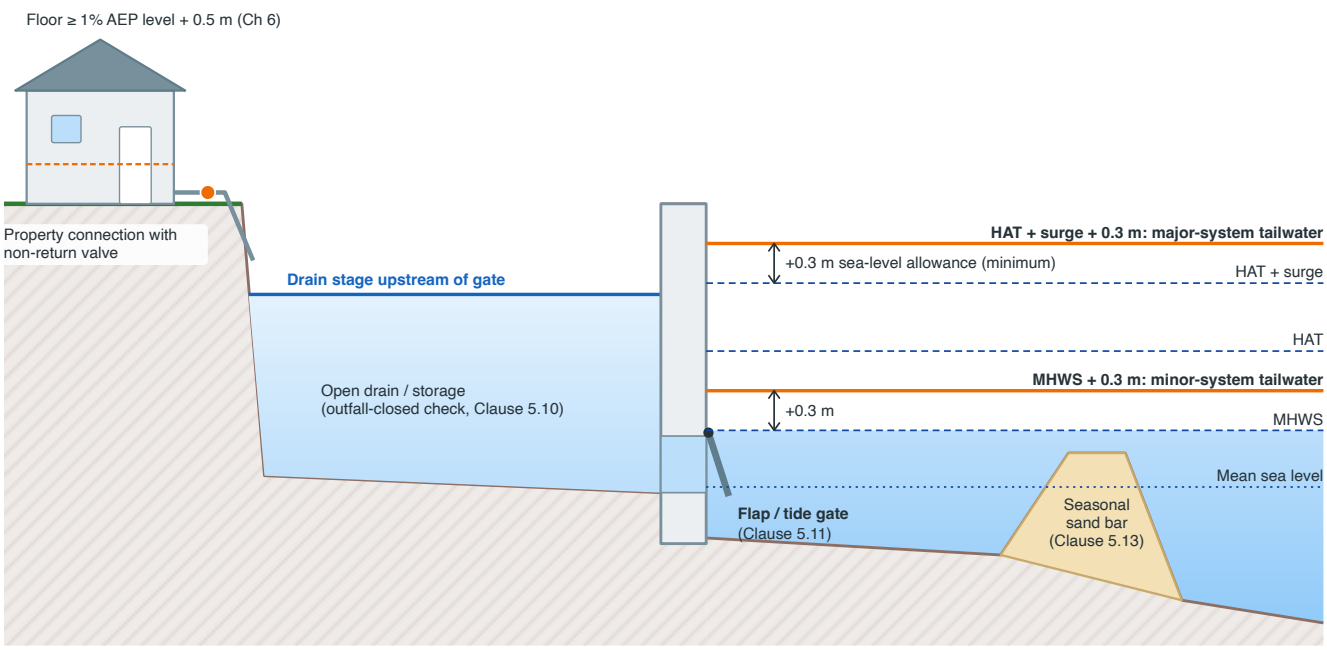
5.7.3 **Storm surge.**

- (a) **Major-system and storage design.** The surge term in "HAT + surge" is the **10% AEP (10-yr) storm surge** for the coastal segment, given in Annex 5A. The 1% AEP surge is used only in the joint case of Clause 2.12(b), with the 10-yr 48-hour storm. That keeps the two cases symmetric, so that a rare rainfall is never paired with a rare surge.
- (b) **Joint case.** The 1% AEP still-water level for Clause 2.12(b) is taken from Annex 5A.
- (c) Until Annex 5A is published, both values shall be taken from a site-specific study using INCOIS, IMD or NCCR surge hindcasts or return-level analyses. The study shall state its source, method and record length.
- (d) Wave set-up shall be added where the outfall faces the open sea without a protected entrance.

5.7.4 **Sea-level allowance.** The SLA shall be:

- (a) **+0.3 m** minimum for every element;
- (b) greater than 0.3 m where the State, RDA or the Authority adopts a higher projection for the planning area; and
- (c) assessed for the design life of the element. For elements with a design life of more than 50 years, the designer shall report the effect of a sea-level rise of twice the SLA as a sensitivity run.

Tidal outfall — schematic section, not to scale (Clauses 5.6–5.13)



Notes
Levels are diagrammatic. Datums from the current NHO / Survey of India tide tables, reduced to the project datum (Clause 5.7).
Joint case (Clause 2.12 b): 10-yr 48 h storm with the 1% AEP still-water level + 0.3 m. Riverine reaches: see Clause 5.8.
Outfall invert below the 1% still-water level → flap or tide gate required; property connections need non-return valves.

Figure 5.2 — Tidal outfall: flap gate, property non-return valve, and the design tailwater levels (MHWS + 0.3 m for the minor system; HAT + surge + 0.3 m for the major system), with a seasonal sand bar at the mouth. Schematic, not to scale. Original drawing for this Manual, adapted from the tidal tailwater table in MoHUA/CPHEEO Manual on Storm Water Drainage Systems (2019), Part A, Table 6.2.

Commentary. CPHEEO (2019) Table 6.2 sets the design tailwater for coastal outfalls as:

- MHWN to MHWS for a minor storm (more frequent than 1 in 5 years);
- MHWS to HAT for a major storm; and
- "Effect of climate change: additional 0.3 m".

> CPHEEO also says the joint probability of storm, tide and surge "should" be considered, with the state maritime board consulted. This Manual takes the upper end of each CPHEEO range, adds surge for the major system, and makes the joint case explicit (Clause 2.12). CPHEEO does not require surge to be added; this Manual adds it because the surge records of the coastal States showed why it is needed there; Chhattisgarh has no coast, and the surge term does not arise in the State, which a Bay of Bengal depression or cyclone reaches only as rainfall (Chapter 2).
>
> Indian tide tables have been published by the Survey of India since the nineteenth century (Survey of India, Tidal Predictions). Current Indian tidal predictions also appear in the NHO's tide tables. One port document describes the current edition as covering 95 Indian ports, based on NHO observations and on data from civil ports, NIO and INCOIS (Adani Ports, Dahej tide table 2025, citing NHO INP-12). INCOIS runs coastal tide gauges and issues storm-surge and swell-surge warnings. None of these is used for design in Chhattisgarh, which has no coast, and Annex 5A is not prepared for the State (Clause 5.1.4).

5.8 Riverine backwater

5.8.1 **Where it applies.** Outfalls to rivers and canals whose water level in a storm can exceed the outfall invert shall be designed with riverine backwater. This includes:

- the **Kharun** at **Raipur**, past **Mahadev Ghat** and the **Pathardihi** gauge, with the nalas of the city that end in it, and on to its confluence with the **Seonath** (Clause 5.8.5(a));
- the **Seonath** at **Rajnandgaon**, **Durg** and **Bhilai** below the **Mongra** barrage, with the **Shankar** and **Pulgaon** nalas of Durg, and the **Hamp** at **Andhiyar Khore** (Clauses 5.8.3 and 5.8.5(a) and (c));
- the **Arpa** at **Bilaspur** below the **Arpa Bhaisajhar** barrage, and the **Maniari** at **Maniyari** below the **Maniyari** dam (Clauses 5.8.3 and 5.8.5(a) and (c));
- the **Hasdeo** at **Korba** below the **Minimata Bango** dam and the **Hasdeo barrage** at **Darri**, with the **Katghora** and **Bangodam** gauges and the nalas of Korba (Clauses 5.8.3 and 5.8.5(a) and (c));
- the **Mahanadi** below the **Gangrel** dam at **Dhamtari** and **Rajim**, with the **Pairi** at **Malgaon**, past the barrages from **Samoda** to **Kalma** and the gauges at **Kalma** and **Surajgarh**, and the **Mand** at **Kurubhata**, to the head of the **Hirakud** reservoir (Clauses 5.8.3 and 5.8.5(a), (c) and (d));
- the **Kelo** at **Raigarh** below the **Kelo** dam (Clauses 5.8.3 and 5.8.5(a) and (c));
- the **Indravati** at **Jagdalpur** and **Chindnar**, below the dams in Odisha, with the **Narangi** at **Sonarpal**, and the **Sabari** at **Talnar** and **Konta** (Clauses 5.8.3 and 5.8.5(a) and (c));
- the backwater of the **Hirakud** reservoir in **Raigarh** and **Sarangarh-Bilaigarh** districts (Clause 5.8.5(d));
- the **Gangrel**, **Minimata Bango**, **Kharang**, **Maniyari**, **Kelo** and **Kopra** reservoirs, the ponds of the **Mongra**, **Arpa Bhaisajhar** and **Darri** barrages and of the barrages on the Mahanadi, the lakes and reservoirs of **Nava Raipur Atal Nagar** (among them **Sendh**, **Jhanjh** and **Khandawa**), the talabs of **Raipur** (among them **Budha Talab**, **Telibandha**, **Kankali**, **Maharajbandh**, **Karbala** and **Narhaiya**) and the talabs of the other towns that receive their drains (Clause 5.8.5(b));
- the mine voids, pit lakes and working mines of the **Korba**, **Mand–Raigarh** and **Surguja** coalfields, and the ash ponds and ash dykes of the power plants of **Korba** and **Raigarh** (Clause 5.8.5(e));
- any irrigation canal of **WRD**, and any drain or channel of another body that receives storm water (Clause 5.8.5(g)); and
- elsewhere: any river or WRD channel listed in Annex 5B.

5.8.2 **River levels.** River water levels shall be taken from:

- (a) WRD design flood levels or recorded highest flood levels for the reach, where published; or
- (b) a hydraulic model (HEC-RAS or equivalent) of the river reach. The model shall use:
  - the river design flood from a hydrograph method (Chapter 3), with continuous simulation where Chapter 3 requires it;

    - the tidal boundary of Clause 5.7 at the mouth; and
    - the sand-bar condition of Clause 5.13.

5.8.3 **Reservoir and tank releases.** Where a reservoir or tank upstream can release to the river (for example the Ravishankar Sagar (Gangrel) dam on the Mahanadi above Dhamtari and Rajim, the Minimata Bango dam and the Hasdeo barrage at Darri above Korba, the Arpa Bhaisajhar barrage above Bilaspur, the Mongra barrage on the Seonath above Durg, the Kelo dam above Raigarh, the Maniyari dam on the Maniari, the barrages on the Mahanadi from Samoda to Kalma, and the dams in Odisha on the Indravati above Jagdalpur), the river level shall include the release given by the reservoir's operating rule for the storm being checked. Where there is no published rule, it shall include the largest recorded release.

5.8.4 **Coincidence of site storm and river flood.** Unless a joint-probability study is accepted by the Authority, the more severe of the following governs:

- (a) the site design storm (AEP as required), with the river at its **10% AEP (10-yr)** level; and
- (b) the site **10% AEP** storm, with the river at its design-AEP level (1% AEP for the major system).

The two storms in each pair shall have the same duration and timing where they are generated by the same rainfall event.

5.8.5 **River-locked outfalls, reservoir, barrage and talab outfalls, rivers below dams and barrages, the Hirakud backwater, mine voids and ash ponds, WRD consent, and discharges to canals and the drains of other bodies.**

- (a) **River-locked outfalls.** For an outfall to a river directly, through a gate in a bank or embankment or through a pumping station, or into a drain that itself discharges in one of those ways, the designer shall identify the body that operates each gate and pumping station, and obtain the sill of each gate, the crest of the bank or embankment, the installed pumping capacity and the operating levels, including the river level at which each gate is closed. Every level shall state its datum and its connection to the gauge datum of the nearest CWC or WRD gauge.
    - **Design river level.** The river level of Clause 5.8.2 at the outfall shall be, for the major-system cases (Clause 5.8.4(b)), the **1% AEP river level at the outfall**: the 100-year level of a flood-frequency study or floodplain delineation that gives a level at the outfall, or a level from a hydraulic model under Clause 5.8.2(b), with the dam and barrage releases of item (c). No floodplain of any river in the State has been demarcated, and no flood line of WRD was found for any river [TBC]. The level shall in no case be less than the **highest recorded level** at the nearest CWC gauge on the same river, transferred to the outfall. The highest levels found during research (Table 5B.1), from SANDRP's copies of CWC data and extracts of CWC advisories, none read in a CWC site sheet [TBC], are: the Indravati at Jagdalpur **544.891 m** (26 September 2026; 544.68 m, of 9 July 1973, before it) and at Chindnar **340.49 m** (26 September 2026); the Kharun at Pathardihi **281.68 m** (29 August 2020); the Mahanadi at Kalma **202.43 m** and at Surajgarh **201.06 m** (both 29 August 2020); the Mand at Kurubhata **220.4 m** (28 August 2020); the Hasdeo at Katghora **313.58 m** (20 July 2026) and at Bangodam **303.5 m** (17 August 2026); the Maniari at Maniyari **236.23 m** (13 August 2022); the Hamp at Andhiyar Khore **260.2 m** (12 August 2022); the Pairi at Malgaon **329.33 m** (30 July 2026); the Narangi at Sonarpal **543.43 m** (26 September 2026); and the Sabari at Talnar **213.81 m** (26 September 2026) and at Odinaguda (Konta) **71.64 m** (10 September 2024). Where a later flood has exceeded the listed level, the later level governs; seven of these levels were set in 2026. The recorded level shall be transferred along the river with the water-surface slope of the flood that set it, from the CWC and WRD gauge records, or from the model. Until either is available, the recorded level of the nearest gauge upstream applies unchanged at the outfall, except that where the outfall lies between two gauges on the same river, with no tributary joining between them, the level may be interpolated linearly by river distance between the highest recorded levels of the two gauges (set by this Manual).
    - **Rivers with no gauge level.** No CWC level was found for the Seonath at Simga or at any point near Rajnandgaon, Durg or Bhilai, the Arpa at Bilaspur, the Kelo at Raigarh, the Mahanadi at Rajim, Seorinarayan or Basantpur, or the nalas of Raipur, Durg–Bhilai, Bilaspur and Korba [TBC]. On such a reach, and for an outfall upstream of every gauge on its reach, the river level shall be the higher of the level from a model

under Clause 5.8.2(b) and the highest flood level surveyed at the outfall from flood marks, including those of the Kharun at Mahadev Ghat in September 2024 and September 2026 and of the Arpa at Bilaspur in July 2026, and from the records of WRD, the local body or the Collector, with its source stated. Where a level is known only as a gauge reading, the designer shall tie the gauge zero to the project datum and a Survey of India benchmark by levelling. No outfall shall be designed on such a reach without one of these levels (set by this Manual).

- **The local gauge at Jagdalpur.** The readings reported in the press for the Indravati at Jagdalpur, such as 12.56 m, "4.26 metres above the danger mark", on 26 September 2026 (as reported), belong to a gauge whose location, zero and datum have not been reconciled with those of the CWC gauge, whose danger level is 540.80 m [TBC]. They are alert levels for operations only. They shall not be used as a design level, compared with a CWC level or transferred to an outfall, until the zeros of both gauges are tied by levelling to a Survey of India benchmark and the tie is confirmed by CWC or WRD (set by this Manual).
- **10% AEP level.** For the cases with the river at its 10% AEP level (Clause 5.8.4(a)), the level shall be taken from the same model or a frequency analysis of the gauge record, and not less than the **danger level** at the nearest CWC gauge, transferred in the same way: **540.80 m** at Jagdalpur (warning level 539.49 m; 539.50 m in one advisory), from extracts of CWC advisories of 2019 [TBC]. No danger level was found for any other gauge in the State; there, the 10% AEP case shall use the design river level of this item until a frequency analysis is accepted (all set by this Manual). The datum of every level is to be confirmed for each gauge [TBC].
- **Tailwater.** The tailwater at the outfall shall be the higher of that river level beyond the gate, bank or embankment, and the maximum level in the drain behind it, computed by routing the drain as a closed storage for as long as the river stands above the level behind it, with the inflow of the whole catchment that drains to it, its storage as surveyed in its present condition, net of silt, encroachment, solid waste and weed, and an outflow of no more than the pumping capacity that the operator confirms in writing is available in a storm, and in no case more than the installed capacity of the permanent pumps less the largest pump (set by this Manual), with gravity discharge through the gate only while the river is lower than the level behind it. Pumps shall deliver against the design river level of this item, and every pump, panel and control shall remain operable with the river at that level (Clause 5.12.5).
- **River lock.** Portable dewatering pumps brought in for an event, and capacity that has been sanctioned or proposed but not commissioned, shall not be counted. The outfall-closed check of Clause 5.10 applies with a single lock that lasts for as long as the river stays above the outfall invert, or above the level at which its gate is closed if that is lower, taken from the river model or from the stage record of the nearest CWC or WRD gauge in the flood that set the highest recorded level or in a comparable flood, with the releases of item (c), and not less than: **120 hours (5 days)** on the Mahanadi from Rajim to the Hirakud backwater, on the Mand below Kurubhata and on the low ground that drains to them; and elsewhere **72 hours**, or, on a river below a dam or barrage, the time for which the largest recorded release of the dams and barrages above stayed above the flow at which the outfall is closed, from WRD's records, if that is longer (set by this Manual). Either lock covers the whole of the 48- and 72-hour storms of Clause 5.10.2, so that storage and pumping handle the full storm volume (Clause 5.9.4) and the drawdown of Clause 5.10.5 is met by pumping until the lock ends. A shorter lock may be used only where the outfall invert is above the 1% AEP river level of this item, or where a gauge analysis under Clause 5.10.6, including the releases of the dams and barrages above, shows it and the Authority approves it.
- **Gate failure and breach.** Floor levels and pumped outfalls behind a gate, bank or embankment shall also be checked for the failure of the gate and for a breach of the bank or embankment, with the land and drains behind it filled to the river level beyond it. Below a dam or barrage they shall also be checked for the release that results from a gate failing open.

- (b) **Reservoir, barrage-pond and talab outfalls.** For an outfall to a reservoir, barrage pond, lake, talab or pond, directly or through a nala that ends in it, such as the talabs of Raipur, the lakes and reservoirs of Nava Raipur Atal Nagar, the Gangrel, Minimata Bango, Kharang, Maniyari, Kelo and Kopra reservoirs, and the ponds of the Mongra, Arpa Bhaisajhar and Darri barrages and of the barrages on the Mahanadi:
    - **Consent.** Storm water shall not be discharged to a reservoir, barrage pond, lake, talab or pond without the written consent of its custodian, the body that holds and operates it: WRD for its reservoirs and barrages; NRDA for the lakes and reservoirs of Nava Raipur Atal Nagar; the Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat for a tank vested in it (Municipal Corporation Act 1956 and Municipalities Act 1961, as adapted; the Chhattisgarh texts were not read [TBC]); the Collector for a tank vested in the State under s.251 of the Chhattisgarh Land Revenue Code 1959; or another custodian that the State names (the custodian of each talab of Raipur and of each lake of Nava Raipur was not confirmed during research [TBC]). Where the waterbody is a Ramsar site (Kopra Jalashay, designated on 8 August 2025) or a wetland to which the State Wetland Authority applies the Wetlands (Conservation and Management) Rules 2017, as it has to the Karbala and Narhaiya talabs in Raipur and the Jhanjh wetland in Nava Raipur (as reported), any permission that the Authority requires shall also be obtained (Chapter 7). An outfall that carries dry-weather flow is dealt with under Clause 8.9.
    - **Design level.** The tailwater for the major-system and storage cases shall be the waterbody's **maximum water level (MWL)** as WRD or the custodian publishes it, or, where only the full tank or reservoir level (FTL) is published, **FTL plus the surcharge in its design flood**, computed from the rating of its spillway, weir and gates with the gates operated only as the custodian's written rule provides. Where neither the MWL nor the surcharge has been published, or where the published MWL is not above the FTL, the tailwater is **FTL + 0.5 m** (set by this Manual) [TBC]. For a talab or pond with no published level, the tailwater is the level at which it spills to the next drain or onto land, from its overflow sill or, where it has none, the lowest point of its rim, plus the head over that point needed to pass the 1% AEP inflow (set by this Manual). For a barrage pond, the tailwater is the higher of that level and the river level of item (a) at the barrage with its gates as operated. In every case it shall be not less than the highest level recorded at the waterbody. The minor-system tailwater is the FTL or the spill level, or the level at which the custodian's rule opens the gates if that is higher. The project's discharge shall be included in the level (Clauses 5.3.2 and 5.12.3).
    - **The named reservoirs and barrages.** The full reservoir levels of WRD's daily tank gauge report of 10 October 2024, which gives no MWL, give the following tailwaters (FTL + 0.5 m) until WRD confirms the MWL or the design-flood surcharge and the datum (set by this Manual) [TBC]: **Ravishankar Sagar (Gangrel)**, FRL 348.700 m, **349.20 m**; **Minimata Bango**, FRL 359.660 m, **360.16 m**; **Kharang**, FRL 294.110 m, **294.61 m**; **Maniyari**, FRL 358.140 m, **358.64 m**; **Kelo**, FRL 233.000 m, **233.50 m**; the **Arpa Bhaisajhar** barrage, FRL 302.000 m, **302.50 m**; and the **Mongra** barrage on the Seonath, FRL 337.150 m, **337.65 m**. No level on a stated datum was found for the Hasdeo barrage at Darri, the barrages on the Mahanadi, Kopra Jalashay (whose Ramsar information sheet gives an elevation of 287–295 m), the lakes and reservoirs of Nava Raipur Atal Nagar or any talab of Raipur [TBC]; no outfall to any of them shall be designed without the custodian's FTL and spillway or weir rating in writing, or, until the custodian gives them, a survey of the spillway, weir or overflow crest tied to a Survey of India benchmark, taken as the FTL (set by this Manual).
    - **Closed outfall.** Where the outfall invert is below the design level of this item, the outfall shall be treated as closed for the whole storm (Clause 5.9.4), with storage and pumping handling the full storm volume, unless the custodian's stage record shows the waterbody below the invert at the start of comparable storms and the Authority accepts it. Every reservoir, barrage pond, lake, talab and pond starts every design storm at its FTL or spill level (Clause 7.7.3). No lowering of a reservoir, barrage pond or talab before a storm and no early or additional gate opening shall be assumed unless the custodian's written operating rule and records show it, and no storage shall be credited below its spill level in a lake or talab that a drainage scheme uses as storage, such as the lakes of Nava Raipur Atal Nagar that its parks and channels feed, unless the custodian's records show that storage available at the start of comparable storms.
- (c) **Rivers below dams and barrages.** For an outfall to a river below a dam or barrage, such as the Mahanadi below the Gangrel dam and its barrages, the Hasdeo below the Minimata Bango dam and the Darri barrage, the Arpa below the Bhaisajhar barrage, the Seonath below the Mongra barrage, the Kelo below the Kelo dam, the

Maniari below the Maniyari dam and the Indravati below the dams in Odisha:

  - in addition to the cases of Clause 5.8.4, the site design storm (each AEP required) shall be run with the river at the level that results from the largest recorded release of the dams and barrages above, routed to the outfall, with the flow of the catchment below them as it occurred in the same event (set by this Manual), until WRD publishes an operating rule that bounds the release [TBC]. As reported, all 14 gates of Gangrel were open from 24 to 27 September 2026 and it released about 55,000 cusec through eight gates on about 3 October 2025, and on 26 August 2024 Bango released about 30,000 cusec through six gates and the Darri barrage about 20,000 cusec. The design spillway capacities, 17,230 m³/s at Gangrel, 24,000 m³/s at Bango, 19,820 m³/s at Darri and 994 m³/s at Kharang (CWC, National Register of Large Dams 2009), bound what each structure can pass, not what it has passed. The largest recorded release of each dam and barrage and its duration shall be obtained from WRD and stated [TBC];
  - a release, or a release rule, alone shall not be taken to bound the river level: the level shall also be checked for the largest recorded event at the nearest gauge, with the local inflows as they occurred;
  - the river level shall not be reduced on the ground that the flood comes from rain that falls outside the town or outside the State, as on the Indravati, which rises in Odisha, or that the site storm is small, and the release shall be placed before, during and after the site storm;
  - no dam or barrage gate shall be assumed to open earlier or wider than the operator's records show; the release rules and records of the dams on the Indravati in Odisha shall be obtained through WRD from the Government of Odisha [TBC], and until they are, an outfall to the Indravati at Jagdalpur shall take the level of item (a), not less than 544.891 m at Jagdalpur transferred to the outfall; and
  - an outfall in Korba to the Hasdeo, or to a nala or drain that ends in it, shall be designed with the releases of Bango and Darri placed together with the site storm, and an outfall to the Mahanadi between Rajim and the Hirakud backwater with the releases of Gangrel and of the barrages on the river.

- (d) **The Hirakud backwater.** For an outfall in the low land of Raigarh and Sarangarh-Bilaigarh districts that drains to the Mahanadi, the Mand or another river within the reach of the Hirakud reservoir in Odisha, directly or through a nala:
  - the tailwater for the major-system and storage cases shall be the higher of the river level of item (a) at the outfall, not less than 201.06 m at Surajgarh and 220.4 m at Kurubhata transferred to the outfall, and the level of the reservoir under item (b): its MWL, or its FRL plus the surcharge in its design flood, or, where neither is published, its FRL + 0.5 m (set by this Manual), carried up the backwater profile from the dam to the outfall with the inflow of the Mahanadi and its tributaries in the same event;
  - Hirakud is held by the Government of Odisha. Its FRL, its MWL, its operating rule and its backwater levels in Chhattisgarh were not obtained during research [TBC]; the reservoir is reported to have stood at 628.5 ft (about 191.6 m) when its gates were opened on 9 September 2011 (as reported), a level whose datum was not found. They shall be obtained through WRD from the Government of Odisha, with the levels and extent of the land acquired or submerged in Chhattisgarh. Until they are, no outfall in the reach shall be designed with its invert below the higher of 628.5 ft (about 191.6 m) and the highest flood level surveyed at the outfall from flood marks and from the records of WRD and the Collector (set by this Manual) [TBC];
  - an outfall below the design level of this item shall be treated as closed for the whole storm (Clause 5.9.4); the reservoir starts every design storm at its FRL, and no drawdown or gate opening at Hirakud shall be credited; and
  - no outfall structure, gate or pumping station shall be built in the reach without the consent of WRD, which shall obtain any consent that the Government of Odisha requires [TBC].
- (e) **Mine voids, ash ponds and working mines.** For a site in or near a coalfield or a thermal power plant, such as the Korba, Mand–Raigarh and Surguja coalfields and the power plants of Korba and Raigarh, whose drainage could reach a mine void, a pit lake, a working mine, an ash pond or an ash dyke:
  - **Not a design outfall.** A mine void or pit lake, working or abandoned, is not a design outfall or storage unless the lessee and DGMS consent in writing and its water level, spill level and spill path are surveyed and tied to a Survey of India benchmark; where the lease has ended, the body that holds the land shall consent in place of the lessee [TBC]. An ash pond is not a design outfall or storage unless its operator

consents in writing, its water level, dyke crest, spillway and spill path are surveyed and tied to a Survey of India benchmark, and the consent states the stability of the dyke with the design inflow, as assessed for the operator. Neither is ever assumed to have free storage (set by this Manual).

    - **No drainage into a working mine.** No development shall drain, directly or through a drain, nala or overland flow path, into a working mine, its workings, its pit or the area inside its garland drain. The Coal Mines Regulations 2017 require the lowest point at which surface water could enter a mine to stand at least 1.5 m above the highest flood level at that point (reg. 149, as reported; the Gazette text was not read [TBC]), and a drain that raises the flood level at a mine opening works against that.
    - **Where consent is given.** The tailwater is the surveyed spill level plus the head over the spill needed to pass the 1% AEP inflow from all the land that drains to the void or pond, and not less than the highest level recorded in it, from the operator's records; the void or pond starts every design storm full to its spill level; an outfall below that level is treated as closed for the whole storm (Clause 5.9.4); and the downstream analysis continues along the surveyed spill path to the next end point of Clause 5.3.1. The consent shall state the stability of the void's sides and benches, or of the pond's dyke, with the design inflow, and the water-quality conditions (Chapter 8).
    - **Ash-dyke breach.** The breach of an ash dyke is a hazard to map, not a design inflow. A site below or beside an ash pond shall show on its plans the path and extent of a breach flow, from the operator's breach study where one exists and otherwise from the ground levels, and shall keep buildings, basements, pumping stations and outfalls out of it, or set their floor and opening levels above it, as the Authority directs (set by this Manual). No storm drain shall discharge into an ash pond or the drains of an ash pond, and no storm drain shall be used to carry ash-pond water.
    - **Mine-disturbed ground.** Mine-disturbed ground is overburden dumps, backfilled or reclaimed mine land, compacted mine benches and haul roads, and ground filled with ash. Runoff from it is treated as high in volume and in sediment (Chapter 3), and runoff from an overburden dump or ash fill shall pass a sediment basin and stable toe (garland) drains before it reaches any town drain or outfall (Chapter 8). No outfall, drain or storage shall be placed on a dump or dyke slope or in ground declared unstable. The levels of every outfall, drain and floor on mine-disturbed ground shall come from a level survey made for the design.
- (f) **WRD consent; plan, wetland and court lines.** No outfall structure, gate, pumping station or drain shall be built in the bed, bank or floodplain of a river, or in a reservoir, barrage, dam, embankment or canal of WRD, without the consent of WRD; the procedure for that consent was not found during research [TBC]. The designer shall also check the outfall against the lines that the building rules, the wetland rules, the revenue record and the courts have set, as they stand at the time of design (Chapter 7): the distances from water bodies and watercourses of the Chhattisgarh Bhumi Vikas Niyam 1984, which in the inherited 1984 text are 9 m from the highest water mark and, for a major watercourse, 9 m from the average high flood mark or 15 m from the defined boundary, whichever is more (r.50; Chhattisgarh text to be confirmed [TBC]), and the high flood point that the T&CP Directorate's checklist of 2011 requires from WRD, with the no-objection of WRD or the Revenue Department, for land next to a nala, canal or river; the bar on regularising construction on the bed or edge of a reservoir or on a natural drain (Chhattisgarh regularisation Act of 2002, s.7(1)(चार)); the 50 m from the highest flood level of a wetland that the State Wetland Authority applies under the Wetlands (Conservation and Management) Rules 2017 (as reported); the tanks vested in the State under s.251 of the Chhattisgarh Land Revenue Code 1959 and the land recorded as a pond in the revenue record, from which encroachments have been removed under s.248 after orders of the NGT (O.A. 78/2024); and Kopra Jalashay, a Ramsar site. The State has no floodplain zoning law and no notified flood zone. Whether a drain outfall is permitted within each line is to be confirmed with the authority concerned before design [TBC].
- (g) **WRD channels, irrigation canals and the drains of other bodies.** Storm water shall not be discharged to a drain or canal of WRD, or to another irrigation canal, without the consent of WRD or the canal's owner, or to a drain of another body, such as a drain of a Municipal Corporation or other urban local body, RDA, NRDA, the Chhattisgarh Housing Board, the township of the Bhilai Steel Plant, an industrial area, a mine or a power plant, without the written consent of that body. Where consent is given:
    - the discharge shall not exceed the rate the consent allows;

- the tailwater is the higher of the channel's design water level as its owner states it in writing and its level in the coincident case of Clause 5.8.4, computed from all the storm water it receives, including the drains of other bodies, and with its downstream boundary at the river, reservoir, talab or backwater level of items (a), (b) and (d); until a design water level is published for a channel, the computed level governs [TBC];
- the channel shall be taken in its present condition, net of silt, encroachment, solid waste, weed and obstruction by crossing services and culverts, and with its narrowest section as surveyed, and with any weir that intercepts its dry-weather flow (Clause 8.9) at its crest level; a widening, lining, remodelling or diversion that has been proposed but not completed shall not be counted, and a channel designed for a stated return period shall not be taken to carry more than its surveyed section carries; and
- the outfall shall have a flap gate (Clause 5.11) where the design water level or the computed level is above the outfall invert.

No pump, gate, sluice, barrage or dam operation that helps drainage, such as pumping a drain down before a storm, lowering a reservoir, barrage pond or talab before a storm, closing a gate late or opening dam or barrage gates early, shall be assumed unless the operator's written records show it. Where the outfall invert is below the tailwater of this clause, the outfall shall have its own flap gate (Clause 5.11) and, where the outfall-closed check requires it, pumping (Clause 5.12), with the pumped volume included in the routing of the drain, the waterbody or the river (Clause 5.12.3). Pumping shall not be used to move water into a mine void, a working mine, an ash pond or another catchment.

> Commentary. In large continental river basins, a site storm and a river flood are usually treated as roughly independent. That is the basis of tables of coincident frequencies such as FHWA HEC-22 Table 7.3. Chhattisgarh's rivers break that assumption, because the monsoon depressions from the Bay of Bengal that fill them cross the State and fall on its towns at the same time. On 4 August 1910 Raipur recorded 370.3 mm in a day, its highest, with 381.0 mm at Kurud and 318.0 mm at Arang in the same storm (IMD); on 28–29 August 2020, when Akaltara recorded 282.7 mm (IMD), the Kharun at Pathardihi, the Mahanadi at Kalma and Surajgarh and the Mand at Kurubhata all set new highest levels (CWC data, as copied by SANDRP); and between 24 and 27 September 2026 the Indravati set a new highest level at Jagdalpur in the same days in which all 14 gates of the Gangrel dam were open and the Kharun overflowed at Mahadev Ghat in Raipur (as reported). The two cannot be treated as independent, and a local storm can fall before, during or after the river peak. Clause 5.8.4 therefore pairs a design event on one side with a 10-yr event on the other, in the same way as Clause 2.12(b). This pairing has no direct precedent. It is more severe than an independence assumption would give.
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> Clause 5.8.3 reflects the dams and barrages above the State's towns, all of which WRD holds except those in Odisha. The Ravishankar Sagar (Gangrel) dam on the Mahanadi has a spillway designed for 17,230 m³/s, the Minimata Bango dam on the Hasdeo 24,000 m³/s, the Hasdeo barrage at Darri 19,820 m³/s and the Kharang dam 994 m³/s (CWC, National Register of Large Dams 2009). Gangrel was reported to release about 10,000 cusec through all 14 gates in 2022, 25,000 cusec through nine in 2024 and 55,000 cusec through eight on about 3 October 2025, when it stood at 348.68 m, 99.49% full, and 72 riverside villages were alerted; all 14 gates were open again from 24 to 27 September 2026. Bango released about 30,000 cusec through six gates and the Darri barrage about 20,000 cusec on 26 August 2024 (all as reported). WRD's daily tank gauge report gives the full reservoir level of each reservoir and barrage and the day's release over its spillway (WRD, 10 October 2024). Below Gangrel the Mahanadi passes the barrages at Samoda, Seorinarayan, Basantpur, Mirauni, Saradiha and Kalma, whose crest and pond levels were not found [TBC]; in August 2022 Odisha said that they had been opened before it was told, and that the flow into Hirakud then rose suddenly (as reported). The Indravati comes down to Jagdalpur from Odisha, and the dams above it there are operated by that State. No operating rule or release record for any of these dams and barrages, and no hourly stage record for any gauge in the State, was obtained during research [TBC]. Until they are, the largest recorded release applies, with the flow of the catchment below the dam as it occurred in the same event (Clause 5.8.5(c)). The river levels used in this Manual come from SANDRP's copies of CWC data and from extracts of CWC advisories, which could not be downloaded during research, and their datum and gauge zero are to be confirmed with CWC [TBC].

Clause 5.8.5 is set by this Manual. In Chhattisgarh the tailwater at a town outfall is set not by the tide but by a river held high by a flood or by the releases of the dams and barrages above it, by a reservoir, barrage pond or talab at its full level, by the backwater of a reservoir in another State, or, in the coalfields, by a mine void or ash pond whose level no one has surveyed, and no document found during research gives a design level for any of them. The State's municipal law was not read in its Chhattisgarh form: the Municipal Corporation Act 1956, as it stands in Madhya Pradesh, lets the drains of the Corporation empty "into any place, whether within or without the limits of the Corporation" (s.191), but does not say against which level [TBC]. The State has no floodplain zoning law and no notified flood zone (Chapter 7).

The first is the river. Most of the State's towns stand on rivers of the Mahanadi basin below a dam or barrage, and a dam spills when it is full and goes on spilling while the inflow lasts. The records found show how long the rivers stay up: in August 2020 the Mahanadi at Kalma stood above its previous highest level for about 110 hours and the Kharun at Pathardihi for about 46 hours, and in September 2026 the Indravati stood above its level of 1973 at Jagdalpur for about 15 hours (CWC data, as copied by SANDRP), while all 14 gates of Gangrel were open from 24 to 27 September (as reported). No hourly stage record for any gauge in the State was obtained during research [TBC]. The single lock of at least 72 hours therefore covers the whole of the longest design storm of Clause 2.5.2, so that the site does not rely on the river falling during the storm, and below a dam or barrage it is lengthened to the duration of the largest recorded release, which WRD's records show and research did not find [TBC]. On the Mahanadi below Rajim, where the river collects the releases of Gangrel and its barrages and the flows of the Seonath, the Hasdeo and the Mand, and where it stood above a previous record at Kalma for about 110 hours, the lock is set at 120 hours, the next whole day above that record. The levels listed in item (a) come from SANDRP's copies of CWC data and from extracts of CWC advisories, which could not be downloaded; their datum and gauge zero are to be confirmed [TBC]. Seven of them were set in 2026, which is why a later flood that exceeds a listed level governs. The danger level is used as the floor of the 10% AEP level at Jagdalpur because the river has crossed it in recent floods, to 541.65 m between 29 July and 8 August 2019 (CWC advisories, as seen in extracts) and to a new highest level in 2026, so a level reached about once in ten years is unlikely to lie below it; it is to be replaced by a frequency analysis of the CWC record [TBC]. Elsewhere no danger level was found, and the design level is used instead. At Jagdalpur the press reports a local gauge, on which the river stood "4.26 metres above the danger mark" on 26 September 2026 (as reported), while CWC's danger level is 540.80 m; the two have never been reconciled in any source found, and item (a) keeps them apart, because a design level built from one and a reading from the other could be wrong by metres in either direction. Item (a) counts only the permanent pumps less the largest one, for the same reason that Clause 5.12.2(b) requires a standby pump.

The second is the reservoir, barrage pond and talab. A reservoir at its full level does not fall for the rest of the season, but passes its inflow over its spillway, and its level rises above FTL by whatever head the flood needs: Gangrel stood at 348.68 m, 99.49% full, on about 3 October 2025 (as reported), against an FRL of 348.700 m (WRD, 2024). Item (b) therefore takes the MWL, or FTL plus the design-flood surcharge, as the tailwater, and treats an outfall below it as closed for the whole storm, which is what Clause 5.9.4 already requires when the tailwater stays above the outfall. WRD's tank gauge report gives an FRL for each reservoir and barrage but no MWL, so FTL + 0.5 m is set as the interim value: it is the freeboard that this Manual uses for floor levels (Clause 6.11.1), and it is a floor, not an estimate of the surcharge, which depends on the spillway and the flood [TBC]. A barrage pond is held at its level by gates that open in a flood, after which the pond follows the river, so item (b) takes the higher of the two. The talabs of Raipur and the lakes of Nava Raipur Atal Nagar have no published level at all, so item (b) uses their spill level and the head over it, and does not count the storage of a lake that a drainage plan already uses. The custodian's consent is required because a storm drain added to a reservoir or talab raises its level for every other outfall, and because the talabs also receive the city's sewage, which is the subject of Clause 8.9; the State Wetland Authority has applied the 50 m line of the Wetlands Rules to Raipur's talabs and halted works at Karbala Talab in September 2025 (as reported).

The third is the coincidence of the release and the storm. Item (c) runs the design storm on the town with the river at the largest recorded release because the rain that fills a reservoir and the rain on the town below come from the same monsoon spell or the same Bay depression (see the Commentary to Clause 5.8.4): in September 2026 Gangrel was releasing through all 14 gates while the Kharun overflowed at Raipur (as reported). The spillway

capacities of the National Register of Large Dams are listed only to show the size of the structures; they are not releases. The Indravati's dams lie in Odisha, so their releases are decided outside the State; item (c) asks WRD to obtain their rules and records and, until then, uses the river's highest recorded level.

The fourth is the Hirakud reservoir. The Mahanadi leaves the State into the reservoir of the Hirakud dam in Odisha, and its submergence takes in part of 33 villages of Chhattisgarh (as reported). An outfall in that reach drains not to a river that falls after a flood but to a reservoir that is held up through the later monsoon, at a level set by another State. In August 2022 the two States disagreed over the timing of the releases from the barrages on the Mahanadi (as reported). Item (d) therefore takes the higher of the river level and the reservoir level, treats an outfall below it as closed, and requires Odisha's levels through WRD. Until they are obtained, the 628.5 ft at which the reservoir stood when its gates were opened in September 2011, as reported and recorded in the Odisha edition of this Manual, is used as a floor, because a level that the reservoir has reached cannot be below its design level [TBC].

The fifth is the mine void and the ash pond. A void or pit lake looks like free storage, but its level, spill point and spill path are not surveyed for drainage, its sides can fail, it may be connected to workings, and its water may be acid or laden with sediment. Around Korba the mines are dewatered, with inflows of about 7,500 to 13,700 m³ a day across six mines and the water table drawn down for about 1.2 km around Kusmunda and Gevra (Singh et al., 2023, as summarised), and the Hasdeo has been found to be connected to mine water through a fault (2018, as summarised). The standard condition of the environmental clearances for coal mines sizes the garland drains "keeping at least 50% safety margin over and above the peak sudden rainfall", which is not a return period (environmental clearance of 13 August 2024 for the Gare Palma Sector II mine), and the Coal Mines Regulations 2017 require mine openings to stand at least 1.5 m above the highest flood level (reg. 149, as reported). Ash ponds are storages behind dykes that have failed: NTPC's Dhanras dyke at Korba in July 2024, damaging about 16 ha of paddy; the Korba West dyke of CSPGCL near Jhabu and Nawagaon on 19 April 2026, where one person died; and slurry from BALCO's ash dam into the Belgari (Rukbahari) nala and the Hasdeo (as reported; to be confirmed with CECB). Item (e) therefore treats a void as an outfall only with the lessee's and DGMS's written consent and a survey, and an ash pond only with its operator's consent and a stability statement; it never lets a town drain into a working mine or an ash pond, and it treats a breach as a hazard to keep out of, because a breach flow is a slurry released by a structural failure, not a flood that drainage can be sized for.

The sixth is the body that owns the receiving water. The rivers, dams and barrages of the State are WRD's; Hirakud and the dams on the Indravati are Odisha's; the public drains and tanks vest in the urban local bodies or, for village tanks, in the State (Land Revenue Code s.251); the lakes of Nava Raipur Atal Nagar are planned and held by NRDA; the voids and ash ponds are the coal and power companies'; and no rule dividing the natural nalas from the municipal drains was found (Chapter 6, Clause 6.9). Items (b), (f) and (g) ask for the owner's consent and design level because a design that discharges to a channel, a reservoir, a talab or a void cannot be checked without them. The lines of item (f) decide where an outfall can be built, not its level; their status is set out in Chapter 7.

5.9 Joint-probability cases

5.9.1 Every element draining to a tidal or backwater-affected outfall shall be checked for the cases below. The most severe governs each design quantity: storage, water level, pipe size and pump capacity.

Table 5.2 — Joint-probability cases for tidal and riverine outfalls

Case	Rainfall	Tailwater	Applies to	Basis
J1	Minor-system design AEP, Tc to 24 h	MHWS + SLA (tidal curve)	Minor system	Clause 2.12(a); Clause 5.7.1

Case	Rainfall	Tailwater	Applies to	Basis
J2	1% AEP, all durations of Clause 2.5.2	HAT + surge + SLA	Major system, detention, tanks, pumped systems	Clause 5.7.1
J3	10% AEP, **48 h**	1% AEP still-water level + SLA	Major system, detention, tanks, pumped systems	Clause 2.12(b)
J4	Design AEP, **48 h** (72 h reported)	Tidal curve per J1 with two **6 h lock windows** (Clause 5.10)	All storage and every gated or tide-affected outfall	Clause 5.10
R1	Site design AEP	River at 10% AEP level	Riverine outfalls	Clause 5.8.4(a)
R2	Site 10% AEP	River at design-AEP level	Riverine outfalls	Clause 5.8.4(b)

5.9.2 For storage elements, the 48- and 72-hour storms shall be run with the J2 and J3 tailwaters (Clause 2.5.3).

5.9.3 For tidally influenced riverine outfalls, R1 and R2 shall be run with the river model's downstream boundary at the J1 tidal curve. The Authority may also require them at J2.

5.9.4 Where tailwater is constant at a level above the outfall invert for the whole storm, the case is treated as a fully closed outfall. Storage and pumps shall then handle the full storm volume.

> Commentary. Clause 2.12 sets two joint cases. Case (a) is design rainfall with MHWS plus the sea-level allowance. Case (b) is the 10-yr, 48-hour storm with the 1% still-water level. This Manual adds HAT plus surge for major-system design (Clause 5.7.1). Case J2 combines these. J4 is the outfall-closed check, which tests the timing of tide against the storm rather than a single level. Rainfall and surge share a cyclone cause, so they cannot be treated as independent (Clause 2.12 commentary). These cases are simple surrogates, to be replaced when a statistical joint analysis of IMD rainfall and tide-gauge records is adopted.

5.10 Outfall-closed (tide-locked) storage check

5.10.1 **Where it applies.** The check applies to:

- every storage facility;
- every minor- and major-system element; and
- every pumped system

whose outfall invert is below HAT + surge + SLA, or which drains to a reach subject to tide-lock or sand-bar closure (Clause 5.13).

5.10.2 **Storm.** Use the 48-hour design storm built per Clause 2.9.2 (peak day last), at:

- the AEP of the element: 1% AEP for detention, tanks and the major system; the minor-system AEP for minor drains; and
- AMC III.

The 72-hour storm shall also be run and reported.

5.10.3 **Lock windows.** Unless a site study under 5.10.6 gives otherwise:

- (a) the outfall is closed for **two consecutive lock windows of 6 hours each**, centred on two consecutive high tides of the local tidal curve. For a semi-diurnal tide, the window starts are therefore about 12.4 hours apart.
- (b) During each lock window, gravity discharge is **zero**.

- (c) Between lock windows, gravity discharge is computed against the J1 tidal curve, including the head loss of the flap gate and any non-return valve.
- (d) The first lock window shall be placed so that it is centred on the peak 1-hour block of the final 24 hours of the storm. A second run with the first window **starting** at that peak block shall also be made. The more severe governs.

5.10.4 **Requirement.** Storage plus pumping (where provided) shall hold the inflow so that, throughout the check:

- (a) the maximum stage in any detention facility stays below its emergency spillway crest, so the spillway does not operate. Where the outfall-closed stage is higher than the 1% AEP maximum water level of the free-outfall design, it becomes the facility's 1% AEP maximum water level for Clause 4.9.3, and the spillway crest, freeboard and wall height are set from it;
- (b) the maximum stage in the minor and major systems meets the Chapter 6 limits: building floor levels, and street depths of no more than 150 mm at the crown and no more than 350 mm at the deepest point; and
- (c) no property connection surcharges above the lowest opening it serves, or a non-return valve is fitted under Clause 5.11.

In volume terms, over each lock window and the full storm:

> $V_{inflow} \le V_{storage\ available} + V_{pumped} + V_{gravity\ (between\ locks)}$

This shall be demonstrated by routing, not by a single volume balance. The release from each facility after a lock window ends may exceed the release caps of Chapter 4, because the facility is emptying into a drain that has just reopened. That release shall be included in the downstream analysis (Clauses 5.4 and 5.9).

5.10.5 **Drawdown after the storm.** After the second lock window, the storage shall drain under the J1 tidal curve. The drawdown requirement of Clause 4.7 shall be met, with pumping where gravity cannot meet it (Clause 5.12).

5.10.6 **Site study.** A longer or shorter lock may be used where a site study gives a time series of still-water level for the design case. The study may use a calibrated hydrodynamic model, gauge records of past events, or a surge hindcast. Locks shorter than the default require the Authority's approval. A longer lock shall be used where:

- sand-bar closure (Clause 5.13) or river backwater (Clause 5.8) keeps the outfall closed for longer; or
- the gauge record shows tailwater above the outfall invert for more than 6 hours per tide in comparable past events.

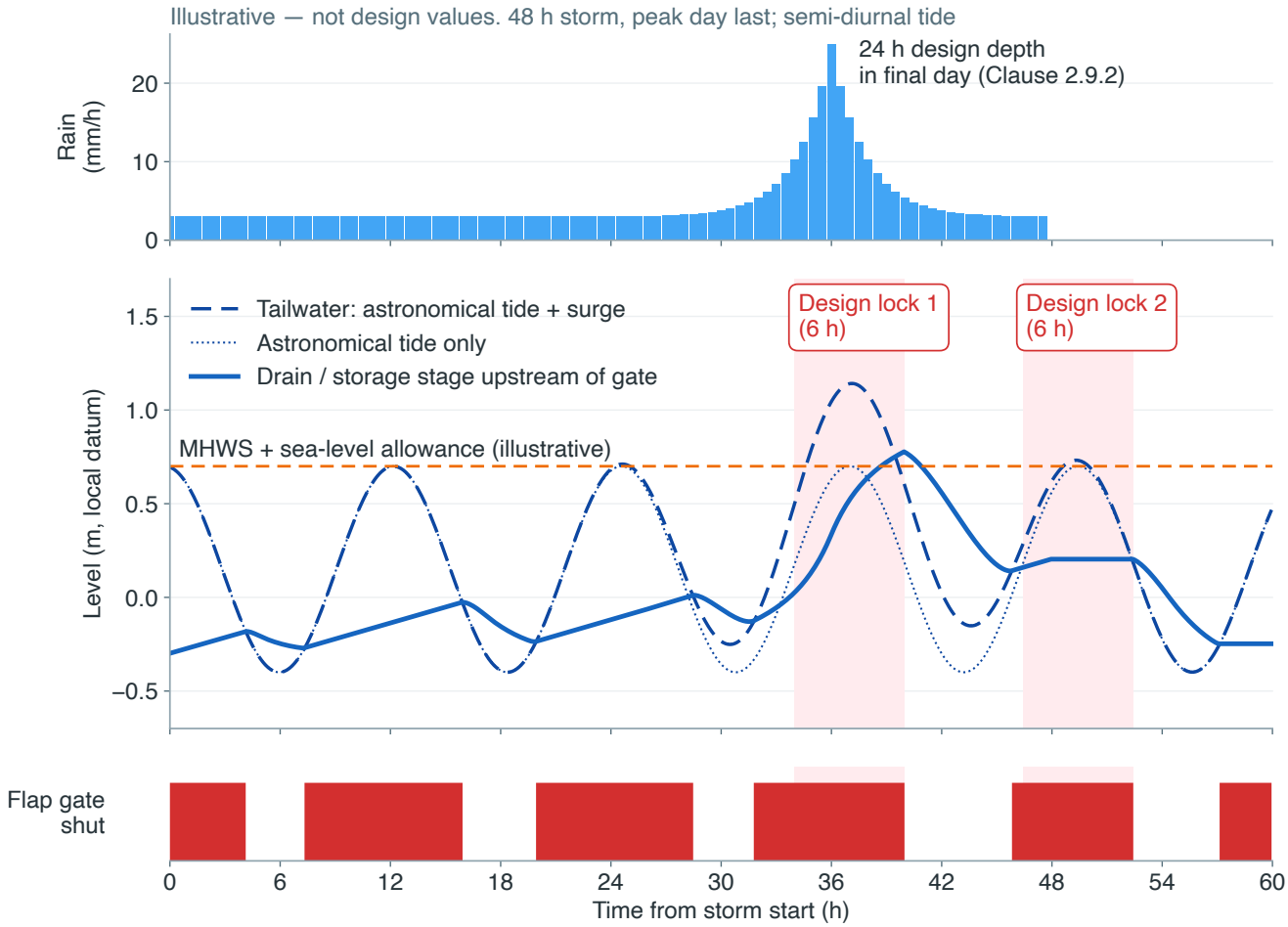

Figure 5.3 — Outfall-closed check: 48-hour storm with the peak day last (Clause 2.9.2), tailwater of astronomical tide plus surge, two 6-hour design lock windows, and the resulting stage upstream of the flap gate. Illustrative — not design values. Original drawing for this Manual, adapted from the tidal tailwater and joint-probability concepts of MoHUA/CPHEEO Manual on Storm Water Drainage Systems (2019), Part A, §6.3.6.

Commentary. The outfall-closed check is the distinctive Chhattisgarh requirement in this Manual. In Chhattisgarh it applies where an outfall is shut not by the tide but by a river held high by a flood or by the releases of a dam or barrage, by a reservoir, barrage pond or talab at its full level, by the backwater of Hirakud, or by a mine void or ash pond with no outlet. In August 2020 the Mahanadi at Kalma stood above its previous highest level for about 110 hours and the Kharun at Pathardihi for about 46 hours (CWC data, as copied by SANDRP), and all 14 gates of the Gangrel dam were open from 24 to 27 September 2026 (as reported). A river below a spilling dam stays high for as long as the dam spills, a reservoir or talab at its full level stays there for the rest of the monsoon, and a void with no outlet does not drain at all, which is why Clause 5.8.5 replaces the two 6-hour windows described below with a single river lock of at least 72 hours, or 120 hours on the Mahanadi, and a closure for the whole storm at a reservoir, talab, Hirakud backwater, void or ash-pond outfall below its design level. A rate rule cannot help when there is no outlet: an orificed basin behind a closed gate simply fills.

The default of two consecutive 6-hour locks has no direct precedent. It represents gravity outfalls failing to discharge for about half of each tidal cycle around the two high tides nearest the storm peak. Figure 5.3 shows why both windows matter. The first lock catches the storm peak. The second arrives before the storage has drained, so the stage rises again.

> Placing the first window on the rainfall peak is deliberately unfavourable. Cyclone rainfall and surge share a cause, and Clause 2.12 already treats them as dependent. The check replaces neither the J2 and J3 level checks nor the downstream analysis; it adds a test of timing and volume.

5.11 Flap gates, tide gates and non-return valves

5.11.1 **Where required.** A flap gate, tide gate or equivalent non-return device is required on every outfall whose invert is below the **1% AEP still-water level** (Clause 5.7.3). It is also required on outfalls into irrigation channels under Clause 5.8.5.

5.11.2 **Hydraulics.** The design shall include:

- the gate's head loss and opening head, from the manufacturer's certified head–discharge curve; and
- the reduction in effective outfall capacity when the gate is partly open.

Gates shall not be assumed to seal perfectly in the outfall-closed check unless they are tested.

5.11.3 **Construction.**

- (a) Gates, hinges and seats shall be of corrosion-resistant materials suitable for seawater (Clause 4.12.5).
- (b) The gate shall be set so that sand, silt or debris on the downstream side cannot hold it shut or open. Where the bed is mobile, the gate invert shall be raised above the bed, or the approach shall be protected.
- (c) A trash screen shall be provided upstream where the gate could be jammed by debris.
- (d) A stop-log slot or penstock shall be provided upstream for maintenance isolation on outfalls larger than 600 mm.
- (e) The gate chamber shall be accessible at all tides, with its access above HAT + surge + SLA.

5.11.4 **Non-return valves on property connections.** A non-return valve, or a backwater valve with an accessible cleaning eye, is required on every property drainage connection where the lowest opening served is below:

- the major-system tailwater (J2) at the point of connection; or
- the maximum stage in the outfall-closed check (Clause 5.10).

Basement drainage shall in addition be pumped to a level above both, with its own non-return valve.

5.11.5 **Inspection.** Gates and valves shall be inspected and operated:

- before **10 June** each year;
- after every event in which they closed against a storm flow; and
- as set in the maintenance plan (Chapter 10).

> **Commentary.** The requirement for gates below the 1% still-water level is set by this Manual. The construction details follow common coastal-drainage practice. A gate that is jammed open by debris lets the tide into the drain. A gate held shut by a sand bank turns a gravity outfall into a dead end. In Chhattisgarh the outfalls do not cross a beach, but many of them end in a river whose level is set by the gates of a dam or barrage upstream, or in a reservoir, barrage pond or talab held at its full level. All 14 gates of the Gangrel dam were opened in September 2026; on 26 August 2024 the Minimata Bango dam released about 30,000 cusec through six gates and the Darri barrage about 20,000 cusec, and houses in the low parts of Korba were flooded; and in August 2022 the State's barrages on the Mahanadi were opened ahead of a sudden rise at Hirakud in Odisha, which said it was told only afterwards (all as reported). A gate opened in a hurry raises the river below; a gate held shut raises the reservoir and every outfall that drains to it. The dams and barrages of the State are held by WRD, while Hirakud and the dams on the Indravati above Jagdalpur lie in Odisha, and the drains that reach the rivers belong to the Municipal

> Corporations, NRDA, the Bhilai Steel Plant and the coal and power companies; no rule linking the operation of any of these gates to the levels of the drains was found during research [TBC], so a gate and the drains behind it can belong to different bodies. The 10 June inspection date is set ahead of the SW monsoon.

5.12 Pumped outfalls

5.12.1 **When required.** Pumping shall be provided where gravity discharge cannot meet:

- the outfall-closed check of Clause 5.10;
- the drawdown requirement of Clause 4.7; or
- the J2 and J3 cases.

Pumping shall not be used where a gravity solution meets these requirements.

5.12.2 **Capacity.**

- (a) The duty pumps together, with the storage provided, shall pass every case of Clause 5.9 (including J4), demonstrated by routing.
- (b) **Standby** capacity at least equal to the largest duty pump shall be installed (duty + 1 standby as a minimum).
- (c) Pump capacity shall be stated at the static head for the **J2** tailwater, not at mean tide.

5.12.3 **Effect on the receiving water.** Pumped discharge to a river, canal or creek shall be included in the river or creek level analysis of Clause 5.8. Pumping that raises the receiving water level above the acceptance criteria of Clause 5.5 elsewhere is not permitted.

5.12.4 **Power supply.** Pumping stations shall have either:

- two independent power supplies; or
- standby generation for 100% of the duty capacity, with on-site fuel for at least **72 hours** of duty operation.

Changeover shall be automatic.

5.12.5 **Flood immunity.** Electrical panels, control equipment, generators and fuel storage shall be set above the higher of:

- the 1% AEP flood level + 0.5 m; and
- HAT + surge + SLA + 0.5 m.

Access to the station shall remain usable in the 1% AEP event.

5.12.6 **Discharge arrangement.** Rising mains shall discharge:

- over a level above the J2 tailwater; or
- through a non-return valve and a flap gate.

The arrangement shall prevent back-siphoning.

5.12.7 **Controls and monitoring.** Each station shall have:

- automatic level control;
- remote alarms for high level, pump failure and power failure, sent to the operator; and
- a level and run-time log kept for at least 5 years.

Wet wells shall be sized to keep pump starts within the manufacturer's limits.

5.12.8 **Screens.** Inlet screens shall be provided with a cleaning method that works during a storm (a mechanical rake, or duplicate screens).

5.12.9 **Testing.** Every pumped outfall shall be test-run at full duty, including on standby power, before **10 June** each year. Private pumped systems shall be covered by a registered maintenance declaration (Chapter 10).

> Commentary. Duty + standby, 72 hours of fuel and flood-immune electrics have no single direct precedent and are set by this Manual. The 72-hour fuel figure matches the longest mandatory design storm (Clause 2.5.2). A pump station that depends only on grid power can stop exactly when it is needed, because supply to flooded areas is often switched off for safety. Singapore's April 2025 amendments added annual maintenance declarations for pumped detention tanks (PUB Addendum No. 3). Clause 5.12.9 and Chapter 10 adopt the same approach. Clause 5.12.3 prevents pumping that simply moves water from one tide-locked drain into a tide-locked river.

5.13 Sand-bar closure of river mouths

5.13.1 **Where it applies.** This clause applies to outfalls into any river or creek whose mouth is subject to seasonal sand-bar closure. That includes the mouths of the coastal States (there is none in Chhattisgarh: Clause 5.1.4), and any other mouth listed in Annex 5B.

5.13.2 **Assumption at the start of the SW monsoon.** The mouth shall be assumed **closed** by a bar at the start of the June–September season, at the crest level of the most recent pre-monsoon survey. This applies unless WRD certifies that:

- a mouth-maintenance programme (dredging, bar cutting or training walls) has kept the mouth open at the start of each of the previous three SW monsoon seasons; and
- the programme is funded for the current season.

5.13.3 **Tailwater with a closed mouth.** With the mouth closed, the tailwater at the outfall shall be the higher of:

- (a) the tidal tailwater of Clause 5.7; and
- (b) the level in the river reach behind the bar, computed with the bar as a weir at its surveyed crest level. That level shall include the river flow of Clause 5.8 and the head needed to pass the flow over or through the bar.

5.13.4 **No credit for emergency cutting.** Bar cutting or breaching during the storm shall not be assumed in design, unless WRD has a documented operating procedure, with equipment stationed for it. Where it does, the bar may be assumed lowered to the procedure's target level only after the time the procedure states.

5.13.5 **Outfall-closed check.** Where the closed-mouth level of 5.13.3(b) exceeds the outfall invert for longer than the default lock windows, the lock duration of Clause 5.10 shall be extended to that period.

> Commentary. Chhattisgarh has no coast and no river mouth, so Clause 5.13 does not apply in the State (Clause 5.1.4). Its nearest inland counterparts are the silt, weed, waste and ash that rivers and drains leave at an outfall. In Bhilai water hyacinth from the drains of the steel-plant township blocked the Junwani and Kosanala culverts in July 2025; in Raipur, drains reported to have been narrowed by encroachment from seven or eight feet to four or five overflowed in July 2026; and at Korba slurry from the ash dam of BALCO has reached the Belgari (Rukbahari) nala and the Hasdeo (all as reported). A flap gate held open or shut by silt, ash or weed, or a gate that cannot be moved, turns a designed outfall into a dead end or a back-flow path. No written gate-operation rule for the gates of any dam, barrage or talab outlet in the State was found during research [TBC]. Clause 5.8.5 deals with both by giving no credit for a gate operation or a storage that the operator's records do not show. Clause 5.13.2 has no direct precedent. It puts the burden of proof on showing that the mouth is open rather than assuming it.

5.14 Documentation

Every project subject to this chapter shall submit a **Downstream and Tailwater Report** (Form 5-1). It shall contain:

1. a plan of the analysis extent showing:
    - the site;
    - each analysis point with its contributing catchment area and the site's percentage of it; and
    - the end point and the reason it was chosen (Clause 5.3);
2. the model description:
    - software and version;
    - the network schematic, with survey sources for the drains and crossings;
    - land use and CN for off-site areas;
    - any upstream detention included; and
    - calibration or validation evidence where available;
3. for each analysis point, AEP and duration, the existing and with-project:
    - peak flow;
    - time to peak;
    - maximum water level and velocity; and
    - duration of inundation (Clause 5.4);
4. the comparison against Clause 5.5, with each failure identified and the remedy adopted;
5. the tailwater derivation:
    - tide-table edition and port;
    - MHWS, HAT and MSL relative to chart datum;
    - the datum connection to the Survey of India benchmark and the project datum;
    - surge source and value;
    - SLA; and
    - river levels and their sources (Clauses 5.6 to 5.8);
6. the joint-case table of Clause 5.9 with results;
7. the outfall-closed check (Clause 5.10):
    - storm and lock alignment;
    - stage hydrographs; and
    - the volume balance for each lock window;
8. a schedule of flap gates, tide gates and non-return valves, with certified head–discharge data (Clause 5.11);
9. pumped outfall design, power supply, flood immunity and controls, where provided (Clause 5.12);
10. the sand-bar assumption and supporting WRD certification, where relevant (Clause 5.13); and
11. inspection and maintenance requirements for Chapter 10.

Small sites under Clause 5.3.4 shall submit Form 5-2 (Simplified Downstream Statement) instead.

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Annexes and forms

Annex / Form	Content
5A	Tidal datums (MSL, MHWS, HAT vs CD and Survey of India datum), surge allowances and 1% still-water levels by coastal segment
5B	Major drains, riverine outfall reaches, and river mouths subject to sand-bar closure; map
5C	Worked example: outfall-closed check for a site on a nala draining to the Kharun in Raipur
Form 5-1	Downstream and Tailwater Report
Form 5-2	Simplified Downstream Statement (small sites)

CHAPTER 6

Minor and Major Drainage Systems

6.1 Purpose and scope

6.1.1 This chapter sets the level of service and the hydraulic criteria for the conveyance system that carries runoff from a development, a road or a public drainage scheme to a receiving waterway, tank or outfall. It covers:

- (a) the dual drainage concept: a minor system and a major system (Clause 6.3);
- (b) design return periods by land use and road class (Clause 6.4);
- (c) street hydraulics: spread, depth and flow hazard (Clause 6.5);
- (d) inlets, including clogging (Clause 6.6);
- (e) pipes and open and covered drains (Clause 6.7);
- (f) hydraulic grade line analysis (Clause 6.8);
- (g) overland flow paths, ponding at sags and building floor levels (Clauses 6.9 to 6.11);
- (h) culverts and road crossings (Clause 6.12); and
- (i) Chhattisgarh construction-practice requirements for covered drains, silt, solid waste, utilities and desilting access (Clause 6.13).

6.1.2 The detailed hydraulic design of individual structures (culverts, channels, energy dissipators, screens, pumps, gates, manholes) is in Chapter 9. Rainfall is in Chapter 2, runoff methods in Chapter 3, on-site storage in Chapter 4, tailwater in Chapter 5, waterbodies in Chapter 7 and water quality in Chapter 8.

6.1.3 This chapter applies to all public drains, all roads with kerbs or roadside drains, and all private drainage systems in developments that meet the applicability thresholds of Chapter 1. In Tier B local bodies, the simplified procedures and design aids referred to in Chapter 1 may be used for elements serving ≤ 16 ha, but the criteria of this chapter still apply.

> Commentary. Chhattisgarh's drains were not designed against any stated storm. The only design return period found for a city's drains is in the development plan of Nava Raipur Atal Nagar, which says that "5 years Maximum rainfall intensity should be taken for design of system" and keeps road levels 0.5 to 1.0 m above the observed high flood level (NRDA, Naya Raipur Development Plan-2031); it is a plan for one city, not a State standard. Raipur's plan for storm drainage under AMRUT recorded drains on 49% of the network, about 480 km of pucca drains of which "Capacity of drain is not sufficient In rainy season most of the drains are overflowed", and 11 road stretches waterlogged two or three times a year, and proposed ₹1,191 crore of works without stating a return period, a rainfall intensity or a runoff coefficient (RMC, 2015); the CAG's audit of AMRUT in the State, which covered water supply, sewerage and parks, never mentions drains (CAG, audit for the year ended March 2022). Where the drains overflow, the water runs along the roads and collects in the low colonies and the underpasses: in Raipur on 5–6 July 2026 the Fafadih and Gudhiyari underpasses, Tatibandh, Shankar Nagar, Mowa and the railway station were flooded, five ponds in Chaubey Colony overflowed into homes and sewage entered houses; in Bilaspur in September 2025 the area from Sarkanda to Abhishek Vihar "turned into sprawling water bodies"; and in Bilaspur in July 2026 more than 400 houses were flooded after a storm whose depth was reported both as 28 cm and as 415.6 mm in 24 hours (all as reported), a conflict that IMD has not resolved [TBC]. The talabs that once held the overflow have been lost: Raipur is reported to have had about 130 ponds and to have about 40 now (as reported). The drains are also divided among owners: the Municipal Corporations, RDA, NRDA, the Chhattisgarh Housing Board, the Bhilai Steel Plant, WRD and the coal and power companies all build or hold drains and

receiving waters, and no rule divides them (see the Commentary to Clause 6.9). Fixing the rainfall (Chapter 2) is only half the answer. The other half is a second, surface-level system that carries the flows the pipes cannot, without flooding buildings. That is the dual drainage concept of this chapter, taken from Kansas City Metro APWA Section 5600 (5601.8, Levels of Service), which requires every element to have a 1% AEP overflow path.

6.2 Definitions

Term	Meaning in this Manual
Minor system	Inlets, pipes, covered drains and open drains that carry runoff up to the minor design AEP of Table 6.1 without surcharge above the limits of Clause 6.8.
Major system	The surface flow paths (streets, overland flow paths, open channels, floodways and storage) that carry flow above the capacity of the minor system, up to the 1% AEP (100-yr) event.
Overland flow path	A defined surface route, in public land or a recorded easement, that carries major-system flow to a receiving waterway.
Spread (T)	Width of flow in the gutter and carriageway, measured from the face of the kerb.
Sag	A low point on a road or flow path from which water can leave only through an inlet or by ponding to an overflow level. A trapped sag has no surface overflow route.
Clogging factor	The fraction of an inlet's clean hydraulic capacity assumed lost to debris (Clause 6.6.3).
Hydraulic grade line (HGL)	The line joining the free-surface levels in a conduit system, or the piezometric levels where the system is under pressure.
Covered drain	A rectangular drain, usually beside a road, closed by cast-in-situ or precast cover slabs that form a footpath.
Sediment allowance	The depth or area of a drain or pipe assumed filled with silt when its design capacity is computed (Clause 6.7.8).
Check event	An AEP rarer than the design AEP at which the combined minor and major system must still meet the limits of Table 6.2.
High-density area	An area notified as such by the approving authority under Clause 6.4.4.
Class I area	An urban local body with a population of 1 lakh or more (the CPHEEO definition).

6.3 The dual drainage concept

6.3.1 Every drainage system shall be designed as two systems that work together:

- (a) a **minor system** that carries the minor design flow of Table 6.1 below ground or in drains, keeping streets usable and spread within Table 6.2; and
- (b) a **major system** that carries all flow above the minor-system capacity, up to the 1% AEP (100-yr) flow, on the surface without entering buildings and within the depth and hazard limits of Clause 6.5.

6.3.2 **Every element has a 1% path.** Every inlet, sag, pipe, drain, culvert and storage element shall have an identified 1% AEP overland flow path to a receiving waterway, tank or outfall. The path shall be shown on the drainage plan (Clause 6.15).

6.3.3 **Major-system design flow.** The major system at any point shall carry the 1% AEP peak flow less the minor-system capacity at that point. The minor-system capacity used in this subtraction is the capacity of the inlets after the clogging factors of Clause 6.6.3, or the pipe capacity, whichever is less. At sags and at any location used to set building floor

levels, the major system shall also be checked with the minor system **fully blocked** (Clause 6.10.3).

6.3.4 **Rainfall and method.** Design flows shall use the rainfall of Chapter 2 (including the climate factor of Clause 2.8) and the methods of Chapter 3:

- (a) the Rational Method only for catchments ≤ 16 ha and a single conveyance element. It shall not be used to accumulate flows down a network whose total catchment exceeds 16 ha;
- (b) a hydrograph method with hydraulic routing (for example SWMM, HEC-HMS with a hydraulic model, or an equivalent 1D/2D model) for networks above 16 ha and for any network with storage, pumping or a tidal outfall;
- (c) a minimum time of concentration of 10 minutes.

6.3.5 **No transfer between catchments.** Neither the minor nor the major system shall divert flow from one natural catchment to another unless the approving authority accepts a catchment-wide study showing no adverse effect.

> **Commentary.** The CPHEEO (2019) manual defines major and minor systems in its glossary but gives no return period for the major system. APWA 5600 (2006, 5601.8) requires the system to convey the 1% peak and, where in-system capacity is less, an overflow system to carry the remainder. Clause 6.3.3 subtracts only the *clogged* inlet capacity, not the full pipe capacity, because in practice the pipes cannot take more water than the inlets admit. The fully-blocked check at sags has no direct precedent; this Manual sets it because of the Raipur experience that drains under footpaths are often silted or blocked by solid waste when the heaviest rain arrives (CPHEEO 2019 Part A, Ch 1 and 2, Figures 1.2 and 2.1 on solid waste dumping in drains). The prohibition on inter-catchment transfer follows the DID Malaysia 2008 submission checklist, which prohibits inter-catchment diversion.

6.4 Design return periods

6.4.1 The minor system, roads and crossings shall be designed for the AEPs of Table 6.1. Where both a land-use row and a road-class row apply to an element, the **rarer** AEP (longer return period) governs.

Table 6.1 — Design AEPs for conveyance (matches Clause 2.4)

Element	Design AEP (return period)	Check event	Basis
Minor system: Raipur, Bhilai, Durg, Bilaspur and Korba core and other high-density areas	10% (10-yr)	1% (100-yr) overland path	This Manual (see Commentary)
Minor system: other Class I areas	20% (5-yr)	1% overland path	This Manual; CPHEEO Table 4.1
Minor system: other areas	50% (2-yr)	1% overland path	This Manual; CPHEEO Table 4.1
Arterial road drainage and crossings	2% (50-yr) for arterials that are notified evacuation or emergency routes; 4% (25-yr) for other arterials	Next higher AEP: 1% for a 2% design; 2% for a 4% design	This Manual (25–50-yr); split set by this Manual
Collector road drainage and crossings	4% (25-yr)	2% (50-yr)	This Manual
Local road drainage and crossings	10% (10-yr)	4% (25-yr)	This Manual
Road underpasses and other trapped sags on public roads	2% (50-yr)	1% (100-yr)	This Manual (Clause 6.10.5)
Airports, railway stations, power stations and other critical infrastructure	1% (100-yr) in Class I areas; 2% (50-yr) elsewhere	—	CPHEEO 2019 Table 4.1

Element	Design AEP (return period)	Check event	Basis
Major (overland) system, every element	1% (100-yr)	—	This Manual
Building floor levels	1% level + 0.5 m freeboard	—	This Manual (Clause 6.11)

6.4.2 **Check events.** At the check event of Table 6.1, the combined minor and major system shall carry the check-event flow within the 1% AEP limits of Table 6.2 and without water entering any building. The 1% overland path requirement of Clause 6.3.2 applies in addition to the check event.

6.4.3 **Higher standards.** An approving authority may adopt a rarer AEP for any element, including through the high-powered committee route in Note 2 of CPHEEO Table 4.1. It shall not adopt a more frequent AEP than Table 6.1.

6.4.4 **High-density areas.** The approving authority shall notify, by map, the high-density areas to which the 10% AEP minor-system standard applies. Until it does, the 10% AEP standard applies to:

- (a) the old towns of Raipur, Durg, Bilaspur and Korba, and the central areas of Bhilai: the older, densely built area within the Municipal Corporation's area that the applicable development plan under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973 (the Raipur Development Plan (Revised) 2021 or its successor, the Durg–Bhilai development plan or the plan of the town) identifies as the old town, core or central business district, including Purani Basti and Sadar Bazar in Raipur, the old town of Durg, the old town of Bilaspur and the old town of Korba, and in Bhilai the central business areas outside the township of the Bhilai Steel Plant, with the boundary of each to be confirmed by the Municipal Corporation and T&CP [TBC];
- (b) in every other Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat, the area that the applicable development plan identifies as the old town or the central business district, and the old village settlements (basti) that have been taken into the built-up area; and
- (c) any area in a Tier A body where the master plan permits a floor space index above the base value for the zone.

6.4.5 **Retrofit.** Where an existing drain cannot practicably be enlarged to the Table 6.1 AEP, the shortfall shall be made good by upstream storage (Chapter 4), regional storage (Chapter 7) or an improved major-system path. The approving authority shall record the residual level of service.

> **Commentary.** The values in Table 6.1 match the Clause 2.4 summary and add rows for underpasses and critical infrastructure. They exceed the CPHEEO (2019) national advisory minimums of 5-yr for Class I cities and 2-yr elsewhere (CPHEEO Part A, Table 4.1). Note 2 of that table allows a State committee to adopt higher values. The 10-yr tier for high-density areas follows Gupta (IIT Bombay), who recommends "1 in 10 year for residential-high density", and Malaysia's MSMA 2, which uses 10-yr minor and 100-yr major systems for link houses, apartments, commercial and industrial land. The road values resolve a conflict in Indian codes. IRC:SP:50-2013 recommends 1–2 years for urban road drains. IRC:SP:42-2014 suggests 25 years for urban roadside drains and a check flood at the next higher return period. APWA 5600 (2006) uses 50-yr for arterial, 25-yr for collector and 10-yr for residential street crossings. Table 6.1 adopts the APWA tiers and the IRC:SP:42 check-flood concept. Splitting arterials into 50-yr evacuation routes and 25-yr others has no direct precedent and is set by this Manual. The 50-yr standard for underpasses is likewise set by this Manual, because an underpass is a trapped sag where the failure mode is deep water on a live carriageway. The default high-density areas in Clause 6.4.4 are set by this Manual and apply until the approving authority notifies a map.

6.5 Street hydraulics: spread, depth and flow hazard

6.5.1 **Gutter flow.** Flow in a triangular kerb-and-gutter section shall be computed with the modified Manning (Izzard) equation:

> Q = (Ku / n) · Sx^(5/3) · SL^(1/2) · T^(8/3)

where Q is the gutter flow (m³/s), Ku = 0.376 (SI), n is the Manning roughness of the gutter (Table 6.6), Sx is the cross slope (m/m), SL is the longitudinal slope (m/m) and T is the spread (m). Composite and depressed gutter sections shall be analysed by the methods of FHWA HEC-22 (4th edition, 2024).

6.5.2 **Spread and depth limits.** Street flow shall not exceed the limits of Table 6.2.

Table 6.2 — Street flow limits

Road class	Minor design event (Table 6.1)	Check event and 1% AEP event
Arterial	Spread shall leave at least one through lane in each direction clear of water. No flow across the crown.	Depth ≤ 150 mm at the crown and ≤ 350 mm at the deepest point; hazard limits of Clause 6.5.3
Collector	Spread T ≤ 3 m. No flow across the crown except at a sag intersection.	As above
Local	Spread T ≤ 3 m. Flow shall not overtop the kerb.	As above
All classes, at intersections	Cross-flow over the crown ≤ 0.03 m³/s at sag intersections and superelevation transitions; none at mid-block sags.	As above

6.5.3 **Flow hazard limits.** In the check event and the 1% AEP event, the product of depth (D, m) and velocity (V, m/s) of street and overland flow shall not exceed the limits of Table 6.3.

Table 6.3 — Flow hazard limits for streets and overland flow paths

Location	Required hazard class (Table 6.4)	Limits
Carriageways of all roads	H2 or better	D·V ≤ 0.6 m²/s; D ≤ 0.35 m (Table 6.2); V ≤ 2.0 m/s
Footpaths, pedestrian crossings, school, hospital and transit-stop frontages	H1	D·V ≤ 0.3 m²/s; D ≤ 0.3 m; V ≤ 2.0 m/s
Notified evacuation and emergency routes	H1	D·V ≤ 0.3 m²/s; D ≤ 0.3 m; V ≤ 2.0 m/s
Overland flow paths in public open space or easements, away from roads	H3 or better, with warning signs	D·V ≤ 0.6 m²/s; D ≤ 1.2 m; V ≤ 2.0 m/s

Table 6.4 — Combined flood hazard vulnerability classes (reproduced for reference)

Class	Description	D·V limit (m²/s)	Depth limit D (m)	Velocity limit V (m/s)
H1	Generally safe for vehicles, people and buildings	≤ 0.3	0.3	2.0
H2	Unsafe for small vehicles	≤ 0.6	0.5	2.0
H3	Unsafe for vehicles, children and the elderly	≤ 0.6	1.2	2.0

Class	Description	D·V limit (m²/s)	Depth limit D (m)	Velocity limit V (m/s)
H4	Unsafe for vehicles and people	≤ 1.0	2.0	2.0
H5	Unsafe for vehicles and people; buildings vulnerable to structural damage	≤ 4.0	4.0	4.0
H6	Unsafe for vehicles and people; all buildings vulnerable to failure	> 4.0	—	—

Source: AIDR Guideline 7-3, *Technical flood risk management guideline: Flood hazard* (2014), Tables 1 and 2, modelled after Smith and Wasko (2012).

6.5.4 Depth and velocity for Clause 6.5.3 shall be taken from the gutter-flow calculation for simple sections, or from a 1D or 2D hydraulic model where flow leaves the kerb-and-gutter section.

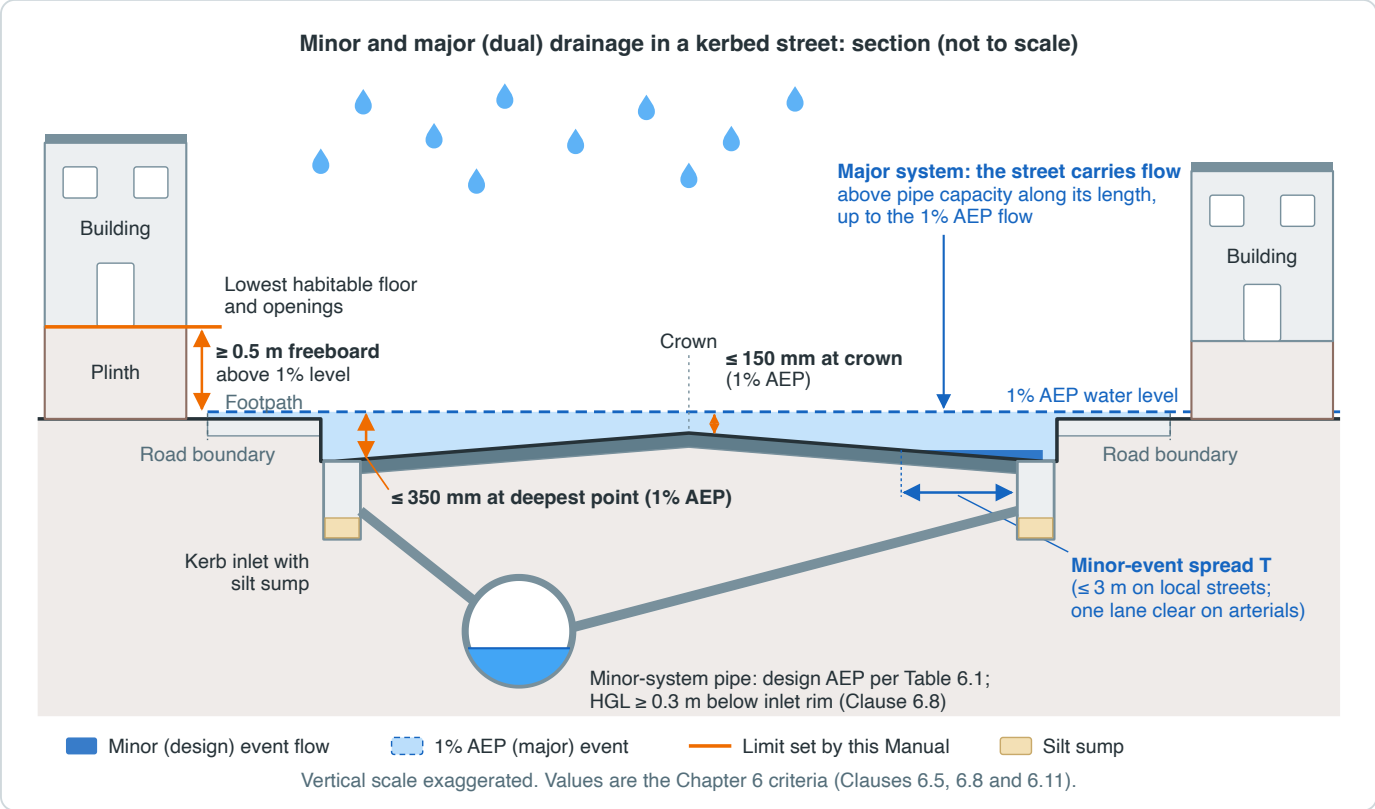


Figure 6.1 — Minor and major drainage in a kerbed street. Original drawing for this Manual, adapted from Kansas City Metro APWA, Section 5600 (2006, 5601.8) and FHWA, HEC-22 Urban Drainage Design Manual (4th ed., 2024).

Commentary. The Izzard form of Manning's equation with Ku = 0.376 is used both in FHWA HEC-22 and in the CPHEEO (2019) manual (Part A, Example 5.5, where the constant is written Kc = 0.376). The arterial spread rule (one lane clear) and the 3 m spread for local and collector streets are close to the APWA 5600 (2006, 5604.2) limit of 12 ft (3.66 m) from the back of kerb in each outside lane. The cross-flow limit of 0.03 m³/s is the SI equivalent of the APWA limit of 1.0 cfs at intersection sumps and superelevation transitions, with no cross-flow at mid-block sumps. The 150 mm and 350 mm depth limits are close to the APWA 5600 limits for 1% overflow at road crossings (7 in, about 180 mm, at the crown; 14 in, about 360 mm, at the deepest point). Mumbai's criterion of no flooding above 15 cm (IRC:SP:50-2013, §6.4.1) is a local precedent for the 150 mm crown value.

The hazard classes of Table 6.4 are the combined hazard curves published by the Australian Institute for Disaster Resilience (AIDR Guideline 7-3, 2014), which are modelled after Smith and Wasko (2012). Requiring H2 or better on carriageways means the 1% depth limit of 350 mm (Table 6.2) governs the depth, and the D·V product controls

fast, shallow flow on steep streets. H2 is described as "unsafe for small vehicles". Two-wheelers are the dominant vehicles on Chhattisgarh streets, so the 1% event must be treated as a road-closure event on all roads except notified evacuation routes, which must meet H1. The assignment of hazard classes to locations in Table 6.3 has no direct precedent and is set by this Manual.

6.6 Inlets

6.6.1 **Inlet types.** Inlets may be kerb-opening inlets, grate inlets, combination inlets (a grate with a kerb opening) or slotted and trench drains. On public streets:

- (a) kerb-opening or combination inlets shall be used, unless the approving authority accepts grate-only inlets for a specific location;
- (b) inlet openings shall be set at road level, with a gutter depression where practicable, so that water is not held on the carriageway by a raised kerb or footpath;
- (c) grates in carriageways, cycle lanes and footpaths shall be of a bicycle- and pedestrian-safe pattern (Chapter 9, Clause 9.11).

6.6.2 **Inlet capacity.** The clean capacity of inlets on grade and in sags shall be computed by the methods of FHWA HEC-22 (4th edition, 2024), or from tested ratings published by the manufacturer for the exact grate and kerb-opening geometry. The empirical kerb-inlet equations of CPHEEO (2019) Part A §5.6.2 may be used as a cross-check only.

6.6.3 **Clogging factors.** The design capacity of every inlet shall be its clean capacity reduced by the clogging factors of Table 6.5:

Table 6.5 — Inlet clogging factors (share of clean capacity assumed lost)

Inlet type	On grade	In a sag
Grate	50%	50%
Kerb opening	10%	20%
Combination	Apply the grate and kerb factors to each part separately	Apply the grate and kerb factors to each part separately

Design capacity = (1 − clogging factor) × clean capacity.

6.6.4 **Inlet spacing.** Inlets shall be located by a sequential calculation that starts at the top of each gutter run. At each inlet, the intercepted flow is computed, and the bypass flow is added to the gutter flow arriving at the next inlet. Spacing shall be such that the spread limits of Table 6.2 are met everywhere in the minor design event. In addition:

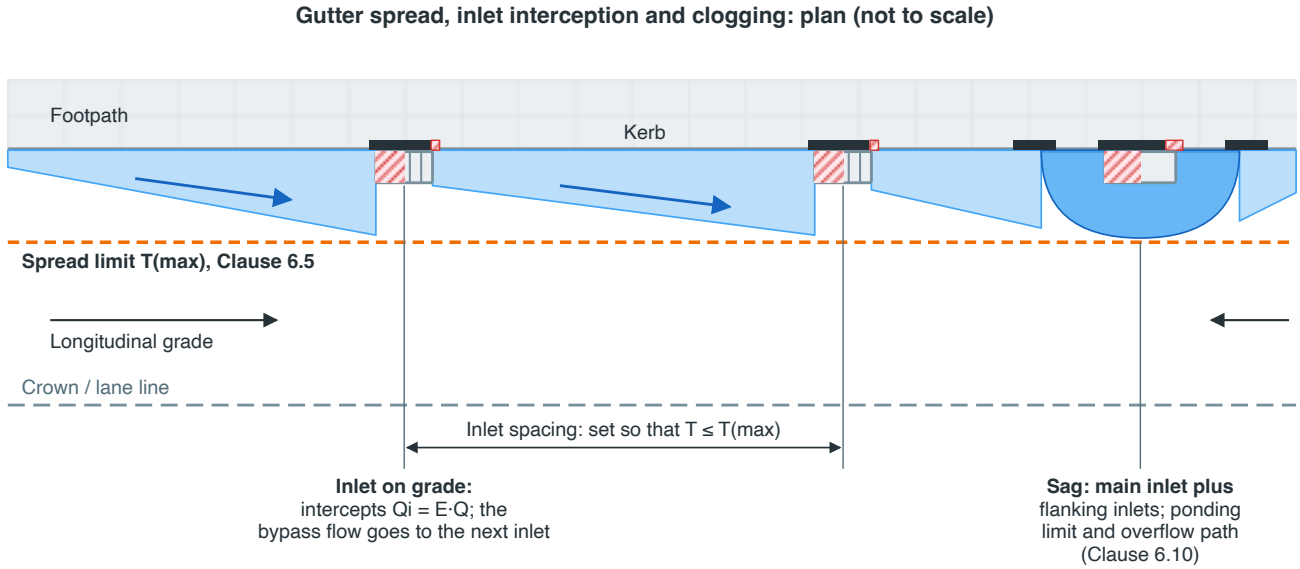
- (a) inlet spacing shall not exceed 30 m on kerbed roads, whatever the calculation gives;
- (b) inlets shall be provided at every sag, immediately upstream of pedestrian crossings and intersections, upstream of bridge decks and superelevation transitions, and at the end of every gutter run;
- (c) bypass flow from the last inlet of a run shall not be passed onto a road of a higher class or into private land.

6.6.5 **Sag inlets.** At a sag on a kerbed road:

- (a) the sag inlet's design capacity (after clogging) shall be not less than the total minor design flow arriving at the sag;
- (b) flanking inlets shall be placed on each side of a sag on any road where ponding could spread beyond the Table 6.2 limits, and at every underpass;
- (c) the ponding at the sag shall comply with Clause 6.10.

6.6.6 **Inlet connections.** The pipe connecting an inlet to the main drain shall have a diameter of at least 300 mm and shall be laid at a gradient that gives at least 0.9 m/s at the inlet's design flow.

6.6.7 **Silt sumps.** Every inlet chamber on a public road shall have a silt sump at least 300 mm deep below the invert of its outlet pipe, accessible for cleaning from the surface.



Clogging factors (share of clean capacity lost), Clause 6.6

Inlet type	On grade	Sag
Grate	**50%**	**50%**
Kerb opening	10%	20%
Combination	Each part as above	Each part as above

Gutter flow spread (minor event)
Ponding at sag
Assumed clogged portion
Kerb opening

Figure 6.2 — Gutter spread, inlet interception and clogging. Original drawing for this Manual, adapted from FHWA, HEC-22 Urban Drainage Design Manual (3rd ed., 2009; 4th ed., 2024) and CPHEEO, Manual on Storm Water Drainage Systems (2019).

> **Commentary.** The clogging factors are more severe than the APWA 5600 (2006, 5604.2) rating of sump inlets at 80% of theoretical capacity (a 20% reduction), because grates in Chhattisgarh streets are routinely blinded by plastic and leaves. CPHEEO (2019) Part A shows solid waste dumped in drains as a primary cause of drain failure (Figures 1.2 and 2.1). APWA 5600 (2006) allows only kerb-opening inlets on public streets; Clause 6.6.1(a) is less strict because combination inlets are useful on flat roads. The 30 m maximum spacing and the treatment of combination inlets in Table 6.5 have no direct precedent and are set by this Manual. A fixed cap stops a single blocked inlet from leaving a long dry gutter run. The 300 mm silt sump is set by this Manual. It matches the 0.3 m sump shown on the CPHEEO catch-basin detail (CPHEEO 2019, Figure 5.9).

6.7 Pipes and open and covered drains

6.7.1 **Uniform flow.** Pipe and drain capacity shall be computed with Manning's equation:

> V = (1/n) · R^(2/3) · S^(1/2); Q = A · V

where V is the mean velocity (m/s), n is the Manning roughness (Table 6.6), R is the hydraulic radius (m), S is the friction slope (m/m), A is the flow area (m²) and Q is the discharge (m³/s). Where backwater from a tailwater, a junction or a structure affects the water level, a gradually varied flow (standard step) or hydraulic network model shall be used, and the HGL requirements of Clause 6.8 apply.

6.7.2 **Manning roughness.** Design values of n shall be taken from Table 6.6. For concrete pipes and masonry or concrete drains, the design value shall be not less than the "fair" or "rough" condition value listed, to allow for sediment, biofilm and ageing.

Table 6.6 — Manning's n for design (from CPHEEO 2019)

Material	Condition	n (CPHEEO)	Minimum design n in this Manual
Cement concrete pipe (collar joints)	Good / fair	0.013 / 0.015	0.015
Spun concrete pipe (RCC, PSC) with S/S joints	Manufacturer's design value	0.011	0.013
HDPE / uPVC pipe	—	0.010	0.011
Plastic (smooth), asbestos cement	—	0.011	0.011
Concrete drain, steel trowelled / wood trowelled	—	0.014 / 0.015	0.015
Sand and cement plaster	—	0.015	0.015
Brick masonry	Good / rough	0.015 / 0.017	0.017
Masonry	Bad condition	0.020	0.020
Rubble set in cement	—	0.017	0.017
Earth channel	Regular, good / ordinary	0.020 / 0.025	0.025
Earth channel	With stones and weeds / poor	0.030 / 0.035	0.035
Earth channel	Partly obstructed by debris or weeds	0.050	0.050
Gutter: concrete	—	0.013	0.013
Gutter: hot-mix asphalt	—	0.015	0.015
Gutter: sprayed seal	—	0.018	0.018

Source of the CPHEEO column: CPHEEO (2019) Part A, Table 5.1 (itself from the CPHEEO Manual on Sewerage and Sewage Treatment Systems, 2013) and Table 5.8 (gutters).

6.7.3 **Velocity limits.** At the minor design flow:

- (a) minimum (self-cleansing) velocity: **0.6 m/s** in open and covered drains and **0.9 m/s** in pipes;

- (b) maximum velocity: **3.0 m/s** in lined drains and in pipes, unless the drain or pipe is designed as a chute under Chapter 9. CPHEEO permits up to 6.0 m/s in plastic or plastic-lined pipes in hilly regions; this may be used in Mainpat, Chirmiri and Jashpur Nagar in the northern hills and other hill towns with the approving authority's agreement;
- (c) maximum velocity in unlined and vegetated channels: Chapter 9, Table 9.2;
- (d) the Froude number of flow in open and covered drains shall not exceed 0.8 at the design flow, unless the drain is designed for supercritical flow under Chapter 9.

6.7.4 **Gradient.** Each pipe and drain shall be laid at a gradient that achieves Clause 6.7.3(a). Where flat terrain or a fixed outfall level makes this impossible, the designer shall:

- (a) report the velocity achieved;
- (b) double the sediment allowance of Clause 6.7.8; and
- (c) provide access for mechanical desilting at the spacing of Clause 6.13.2.

6.7.5 **Pipe flow condition.** Pipes shall be sized so that the minor design flow does not exceed the pipe-full capacity. The greater discharge that a circular pipe carries at about 0.95 of its depth shall not be relied on.

6.7.6 **Minimum sizes.**

- (a) Pipes: 300 mm internal diameter; **450 mm** for public mains.
- (b) Open drains: bed width 0.3 m.
- (c) Covered public drains (drains closed by cover slabs): clear internal width **0.6 m** and clear internal depth **0.6 m**.

6.7.7 **Cover to pipes.** Pipes shall have at least 0.6 m of cover to finished level, and at least 0.9 m under carriageways unless a structural design for the traffic load is submitted. Where the seasonal high groundwater table is within 1 m of the pipe crown, the pipe shall be checked for flotation with the pipe empty (CPHEEO 2019, Table 11.2) and anchored where necessary.

6.7.8 **Sediment allowance.** The design capacity of open drains, covered drains and pipes of 600 mm diameter or more shall be computed with the following depth of sediment at the invert:

- (a) open and covered drains: the greater of **75 mm** and **10%** of the clear internal depth;
- (b) pipes of 600 mm diameter or more: **5%** of the internal diameter;
- (c) pipes below 600 mm: no allowance, but Clause 6.7.3(a) shall be met.

The Manning n for the reduced section shall be that of the drain material in Table 6.6.

6.7.9 **Freeboard in open and covered drains.** At the minor design flow, the water surface (computed with the sediment allowance) shall be below the top of the drain wall, or below the soffit of the cover slab in a covered drain, by at least the freeboard of Table 6.7.

Table 6.7 — Minimum freeboard in open and covered drains

Drain	Minimum freeboard	Source
Bed width up to 300 mm	100 mm	CPHEEO 2019 Table 5.3 (from IRC:SP:50-2013)
Bed width 300–900 mm	150 mm	CPHEEO 2019 Table 5.3
Bed width 900–1,500 mm	300 mm	CPHEEO 2019 Table 5.3
Wider drains and engineered channels, Q < 3 m³/s	450 mm	CPHEEO 2019 Table 5.12 (from IS 7784 (Part 1): 1993)
Q 3 to < 30 m³/s	600 mm	As above
Q 30 to < 300 m³/s	900 mm	As above
Q 300 to < 3,000 m³/s	1,200 mm	As above

Drain	Minimum freeboard	Source
Q ≥ 3,000 m³/s	1,500 mm	As above
Any roadside drain	Not less than 150 mm	IRC:SP:42-2014
Steep (supercritical) channels	Equal to the flow depth	CPHEEO 2019 §5.3.2

At bends, the freeboard shall be measured from the water level on the outside of the bend, including the superelevation $\Delta y = V^2 T / (2\,g\,r_c)$, where T is the top width and r_c the centreline radius (CPHEEO 2019, Eq. 5.29).

6.7.10 **Bends and junctions.**

- (a) The centreline radius of a bend in a drain shall be at least 3 times the drain width (CPHEEO 2019 §5.3.3). In engineered channels it shall be at least 2 times the top width and not less than 30 m (CPHEEO 2019 Table 5.11).
- (b) Where drains meet, a junction sump shall be provided with an internal width of at least twice the width of the outgoing drain. Incoming drains shall enter at less than a right angle to the outgoing flow, and the outgoing invert shall be below the sump floor so that water does not stand in the sump (CPHEEO 2019 §5.3.4).

6.7.11 **Manholes and access chambers on pipes.** Manholes shall be provided at every junction, change of size, alignment or gradient, and at the head of every pipe. Spacing on straight runs shall not exceed Table 6.8.

Table 6.8 — Maximum spacing of manholes on straight pipe runs

Pipe diameter	Maximum spacing
300–600 mm	40 m
700–1,000 mm	100 m
1,050–1,400 mm	150 m
1,500 mm and above	300 m

Source: CPHEEO 2019 Table 11.5. Manhole sizes, drops and covers: Chapter 9, Clause 9.12.

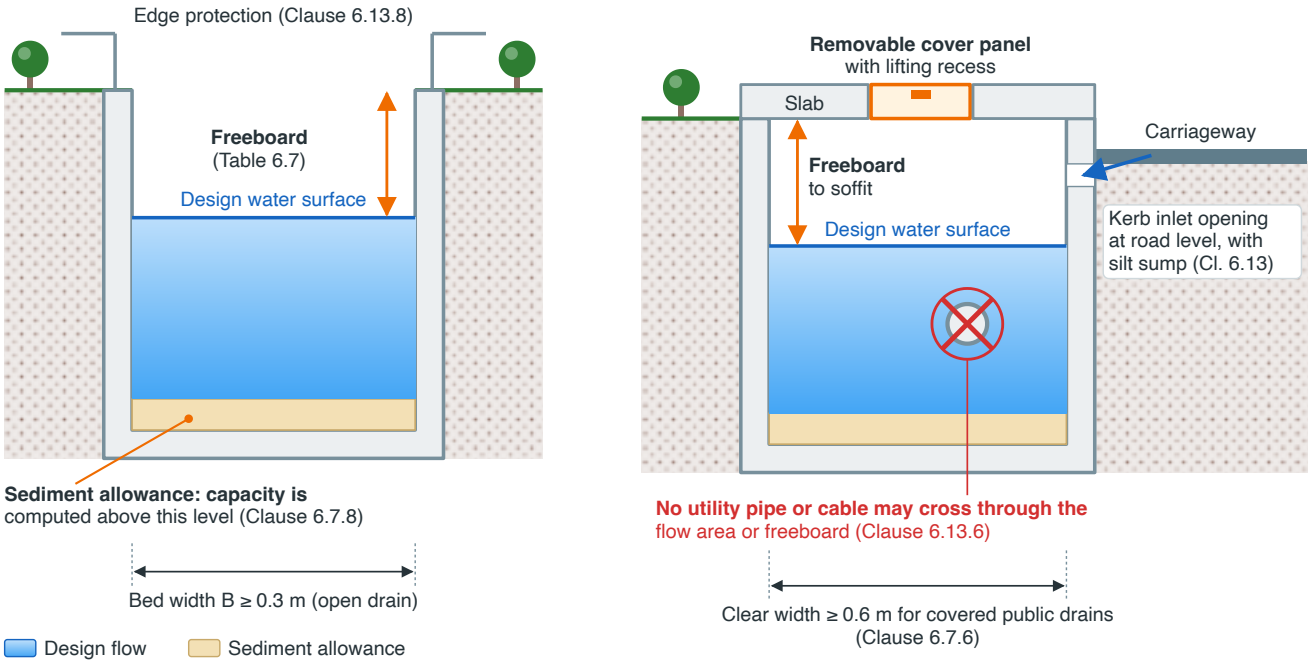


Figure 6.3 — Drain sections: freeboard, sediment allowance and access. Original drawing for this Manual, adapted from CPHEEO, Manual on Storm Water Drainage Systems (2019), Tables 5.3 and 5.12, and IRC:SP:50-2013.

Commentary. The n values are those of CPHEEO (2019) Table 5.1. The higher minimum design values have no direct precedent; this Manual sets them because CPHEEO notes that the 0.011 value for spun pipes "may be taken as given by the manufacturer". That is a clean-pipe value, and Chhattisgarh drains carry heavy silt and solid waste. The 0.6 m/s minimum velocity is CPHEEO's (Table 5.2), based on carrying 0.09 mm sand (§5.3.1). The 0.9 m/s value for pipes is stricter, because a silted pipe cannot be cleaned as easily as an open drain. The 3.0 m/s maximum is from CPHEEO Table 5.2 (WPCF/ASCE 1982). The Froude limit of 0.8 is from CPHEEO §5.3, which advises avoiding critical flow because the water surface becomes unstable.

The 0.6 m × 0.6 m minimum for covered drains has no direct precedent and is set by this Manual. A covered drain narrower than this cannot be desilted by hand or by small machines through a cover opening, and silted, inaccessible drains were a recurring finding after the waterlogging of Raipur on 5–6 July 2026, which residents blamed on inadequate desilting and on drains narrowed by encroachment from seven or eight feet to four or five (as reported), when RMC had recorded as early as 2015 that the "choking of darins [sic]" was one of the main reasons for waterlogging (RMC, AMRUT service-level improvement plan for storm water drainage, 2015), and the warning that the Bhilai Municipal Corporation gave the Bhilai Steel Plant in July 2025 over uncleaned township drains, whose water hyacinth had blocked the Junwani and Kosanala culverts (as reported). The 0.6 m minimum cover is the value used in the CPHEEO (2019) worked pipe-design computation sheet (Part A, Chapter 5). The 0.9 m cover under carriageways is set by this Manual.

The sediment allowance values (75 mm or 10% for drains, 5% for large pipes) are set by this Manual. No Indian standard sets a sediment allowance for design. Raipur's experience is that drains run partly full of silt for much of the year, so a capacity computed for a clean section overstates the real capacity.

The CPHEEO freeboard table (Table 5.3) prints the first row as "Beyond 300 mm bed width". Read with the second row ("Beyond 300 mm & up to 900 mm"), the first row must mean "up to 300 mm"; Table 6.7 is written that way. The CPHEEO manhole spacing table (Table 11.5) gives overlapping ranges (700–1,050 mm and 1,000–1,400

mm); Table 6.8 removes the overlap by assigning 1,050 mm to the 150 m class.

6.8 Hydraulic grade line

6.8.1 **HGL analysis.** For every pipe and covered-drain network, the designer shall compute the HGL for the minor design event and for the check event, starting at the outfall and working upstream. The analysis shall include friction losses and losses at junctions, manholes, bends, inlets, outlets and gates.

6.8.2 **Starting water level.** The starting (tailwater) level at the outfall shall be the higher of:

- (a) the tailwater level of Chapter 5: mean high water springs (MHWS) plus the sea-level allowance of +0.3 m for minor-system design at tidal outfalls, and the level in the receiving drain, river or tank for the coincident event at other outfalls; and
- (b) the level computed at the outlet pipe from the critical depth and the pipe diameter, (dc + D)/2, where the receiving water is lower.

6.8.3 **Minor-event limit.** In the minor design event, the HGL shall be at least **0.3 m below** the rim of every inlet and the cover level of every manhole and access chamber.

6.8.4 **Check-event and 1% limit.** In the check event and the 1% AEP event, the HGL may rise above the inlet rims. Water that leaves the minor system shall then be carried by the major system within the limits of Table 6.2 and shall not enter any building. Manholes that can surcharge above ground in these events shall have bolted or locked covers (Chapter 9, Clause 9.12).

6.8.5 **Structure losses.** Losses at manholes and junctions shall be computed by the energy-based access-hole method of FHWA HEC-22 (2009 or 2024 edition), or by an equivalent method in an approved hydraulic model. A simplified loss-coefficient method ($h = K V^2/2g$) with coefficients from HEC-22 may be used for Tier B networks below 16 ha.

6.8.6 **Invert and soffit levels.** Where pipe size increases at a manhole, the soffits (crowns) of the incoming and outgoing pipes shall be matched, or the outgoing soffit set lower. The outgoing invert shall not be higher than the lowest incoming invert. Vertical drops in manholes shall follow Chapter 9, Clause 9.12.

6.8.7 **Gates and valves.** The head loss through flap gates and non-return valves in the open position shall be included in the HGL (Chapter 9, Clause 9.10).

6.8.8 **Private connections.** Building drainage that connects to a public drain whose 1% HGL is above the building's lowest drainage fitting shall have a non-return valve.

> Commentary. The 0.3 m clearance between the minor-event HGL and the inlet rim is set by this Manual. It keeps a margin for losses that a model underestimates, and it stops water from bubbling out of inlets in the event the system is designed for. In Chhattisgarh many town drains end in a river held high by a flood or a dam release, or in a talab, lake or reservoir at its full level (Clause 5.8.5). Raipur's nalas end in its talabs or in the Kharun, which set its highest recorded level at Pathardihi in August 2020 (CWC data, as copied by SANDRP) and overflowed at Mahadev Ghat in September 2026 (as reported), and the new treatment plants of Raipur intercept the nalas with diversion weirs (CECB, 2021), each of which, unless it is designed for the storm flow, raises the level in every drain above it (Clause 8.9). Korba's nalas end in the Hasdeo below the Bango dam and the Darri barrage, and the channels of Nava Raipur Atal Nagar end in lakes that its parks feed. A drain whose HGL is started from a free outfall, or computed one length at a time without the river, weir or talab level, fails in exactly that way. An HGL computed from the outfall, with the tidal tailwater of Chapter 5, exposes those errors at design stage. The MHWS plus 0.3 m starting level is from CPHEEO (2019) Table 6.2. The access-hole method is described in HEC-22 from the 2009 edition onward.

6.9 Major system: overland flow paths, easements and drainage reserves

6.9.1 **Streets as the primary major system.** Streets shall be graded so that the 1% AEP major-system flow is carried along the road reserve to a receiving waterway without being trapped, except at sags designed under Clause 6.10.

6.9.2 **Overland flow paths outside streets.** Where a major-system path leaves the road reserve, it shall be:

- (a) in public land, in the community open space of a layout, or in an easement recorded on the layout approval and the title;
- (b) at least as wide as the 1% AEP flow top width plus 0.5 m on each side, and not less than 3.0 m;
- (c) kept free of buildings, compound walls, fences, fill, stored materials and planting that would obstruct flow;
- (d) graded to a continuous fall with no sag that lacks an outlet; and
- (e) within the hazard limits of Table 6.3.

6.9.3 **Compound walls.** A compound wall or fence shall not be built across a mapped overland flow path. Where a wall must cross one, it shall have openings with a total area that passes the 1% AEP flow with a 50% blockage, without raising the upstream water level above the limits of this chapter.

6.9.4 **Access along open drains.** Open drains with a top width of 2 m or more shall have a maintenance access strip at least 3.5 m wide along one bank, connected to a public road at intervals of not more than 500 m.

6.9.5 **Supply channels, surplus courses and tanks.** Tank supply channels and surplus courses are part of the major system and shall be protected under Chapter 7.

6.9.6 **Drainage reserves.** A drainage reserve is a strip of land, including the drain, kept free of buildings so that the drain can be maintained, widened and used as a flood path. A drainage reserve shall be provided:

- (a) along every public drain or channel with an internal width of 1.0 m or more that runs through or along a layout or site being developed or redeveloped;
- (b) along every drain or channel shown in an adopted Basin Stormwater Master Plan (Chapter 11), at the width of the future drain in that plan; and
- (c) along every new public drain with an internal width of 1.0 m or more built by a public body.

6.9.7 **Width.** The reserve shall be not less than the width in Table 6.9, measured from the drain's internal width W. The width is based on the **required** drain size (for a planned drain, the size in the master plan), not the existing size. Where the access strip of Clause 6.9.4 is wider than the margin in Table 6.9, the wider governs.

Table 6.9 — Minimum drainage reserve width (adopted from PUB Singapore)

Internal width of drain, W	Open drain	Closed (covered) drain
1.0 m to 1.2 m	3.0 m	W + 0.6 m
more than 1.2 m to 3.0 m	W + 2.6 m	W + 1.6 m
more than 3.0 m to 4.5 m	W + 3.5 m	W + 1.6 m
more than 4.5 m to 6.0 m	W + 4.5 m	W + 3.0 m
more than 6.0 m to 9.0 m	W + 6.0 m	W + 3.0 m
more than 9.0 m to 25.0 m	W + 7.5 m	W + 3.0 m
more than 25.0 m	W + 12.0 m	W + 3.0 m

For an open drain, the larger part of the margin shall be on the side used for maintenance access.

6.9.8 **Use and protection.**

- (a) The reserve shall be shown as a separate plot on the layout or site plan and transferred to the Approving Authority free of cost, as roads and open space reservations are. Where the drain is a WRD channel, the reserve is transferred to WRD.
- (b) No building, compound wall, fence, ramp, step, fill or other structure shall be built in the reserve. Compound walls and boundary fences shall stand outside it.
- (c) The access strip in the reserve shall be level and continuous, and connected to a public road. Bollards or posts and chains may be used at entrances to keep out vehicles other than maintenance vehicles.
- (d) Where the reserve is not fenced off, its edge shall be marked with concrete markers at not more than 50 m spacing.
- (e) Services shall cross under the drain, with at least 1.0 m clearance below the invert, or over it on an approved structure. Services shall not run along the reserve inside the drain section.
- (f) A structure within 2 m of the edge of the reserve shall be designed to remain stable when the drain is excavated to 1.0 m below its invert. The design shall be signed by a qualified structural engineer.
- (g) The reserve counts toward the site's open space requirements only where the Approving Authority's rules allow it; it does not reduce any setback required by Chapter 7.

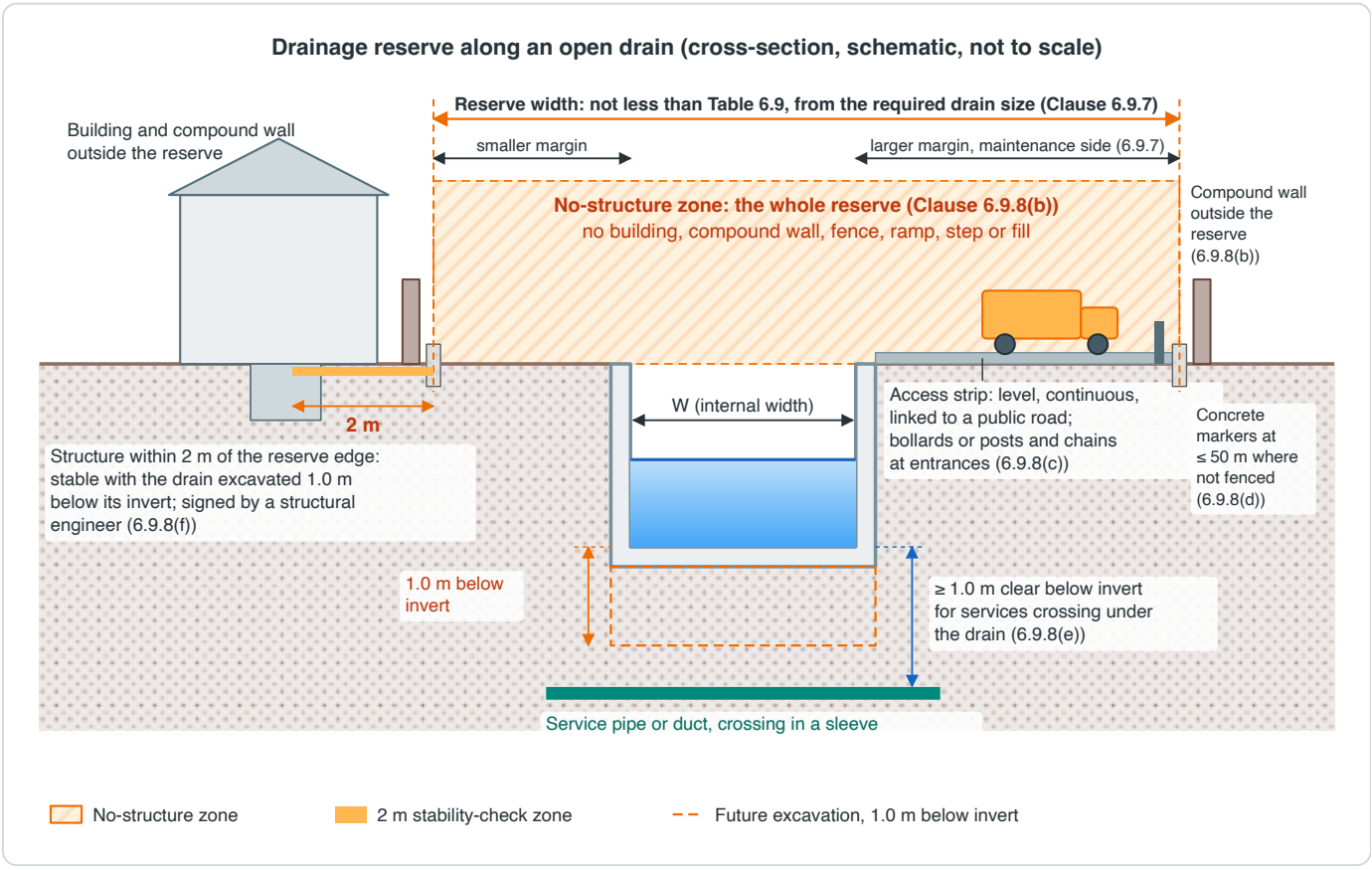


Figure 6.4 — Drainage reserve along an open drain: reserve width from Table 6.9, no-structure zone, access strip on the maintenance side, the 1.0 m service clearance below the invert and the 2 m stability-check zone (Clauses 6.9.6 to 6.9.8). Original drawing for this Manual, adapted from PUB Singapore, Code of Practice on Surface Water Drainage (7th ed., with Addendum No. 3, 2025), clauses 5.1, 5.3, 9.6 and 9.7 and Appendix 1.

Commentary. Drainage reserves follow Singapore, where "the width of a drainage reserve is based on the required drain size" and the reserve is set aside as a separate plot, "kept free of all encumbrances" (PUB Code of Practice on Surface Water Drainage, 7th ed. with Addendum No. 3, 2025, cl. 1.1). Table 6.9 reproduces PUB's Appendix 1 widths. Clauses 6.9.8(b) to (f) follow PUB clauses 5.1 (no structure within a drain or drainage reserve), 5.3 and 5.3.1 (stability for excavation 1.0 m below the invert; engineer's calculations within 2 m of the reserve), 9.6 (level,

continuous access, with bollards or posts and chains) and 9.7 (markers at a maximum of 50 m). The 1.0 m clearance for services crossing under a drain is from PUB's development-plan guidance. In Chhattisgarh, buildings and compound walls are often built up to the edge of a drain, so it cannot be widened or desilted by machine. Reserving the land when a plot is developed or redeveloped is the cheapest time to secure it, because widening later needs land acquisition. Basing the width on the master-plan drain size (6.9.6(b)) protects the land a future drain will need. Chhattisgarh has a building-rule distance along water bodies and watercourses, but no reserve width along a nala. The Chhattisgarh Bhumi Vikas Niyam 1984 are the 1984 rules inherited from Madhya Pradesh in 2000, retitled and amended since by the Housing and Environment Department under s.85(1) read with s.24(3) of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973; their consolidated Chhattisgarh text was not read during research. The inherited 1984 text bars building where the site is within 9 m of the highest water mark and, for a major watercourse, within "9 meters from average high flood mark or 15 meters from the defined boundary of water course, whichever is more" (r.50(b)), and on a site that is not properly drained or is incapable of being well drained (r.50(f)); Rule 50 is still the Chhattisgarh rule on the unsuitability of a site: the T&CP Directorate's report format of 2011 asks whether a site is unsuitable under it, whether the land is a waterlogging area and whether it lies on a canal, nala or river, and if so at what distance, and records the area left for nalas (letter No. 1680/TCR-2/नग्रानि./2011 of 23 June 2011); but the Chhattisgarh text of r.50 was not found, and whether it keeps the 9 m and 15 m is to be confirmed [TBC]. The Directorate's checklist of 2011 asks, for land next to a nala, canal, river or watercourse, for the no-objection of WRD or the Revenue Department and for the high flood point from WRD (No. 1569/स्टेनो/नग्रानि./2011), the only flood-level requirement found in the planning process. The amendment of 6 October 2018 (No. F 7-27/2017/32) shows that the Niyam contain a nala distance: its site-inspection form asks whether a major drain (nala) is adjacent to the plot and, if so, its nearest distance, which is to comply with a rule of the Niyam whose number the Gazette leaves blank; no amendment found, from 2008 to 2023, changes r.50 or adds a rule on drainage, site levels or detention. Integrated townships may not be laid out on land beside a perennial river (Appendix ण, 17 December 2008), nor may permission be given on such land for agricultural land in a planning area without a development plan (Housing and Environment Department order No. F 7-35/2012/32 of 2 January 2013, para 2.4(iii)), and the Chhattisgarh Act of 2002 on the regularisation of unauthorised development bars the regularisation of construction on the bed or edge of a reservoir or on a natural drain (s.7(1)(चार)). Neither defines a nala, a watercourse or the high flood mark, so the line from which the distances are measured is not fixed. For this Manual, as its own proposal and not as the meaning of the Niyam, the highest water mark or high flood mark is taken to be no lower than the higher of the FTL and the monsoon water line for a talab, lake or wetland, and no lower than the top of bank for a channel (Chapter 7, definitions after Clause 7.2); every nala that drains 8 ha or more is a natural channel under Clause 7.4.1(b), whoever maintains it and whether or not it is lined, piped or covered; and land recorded in the revenue records as a nala, talab or dabri is part of the channel or waterbody whatever its present use (set by this Manual). The State Wetland Authority has applied the bar of the Wetlands (Conservation and Management) Rules 2017 on work within 50 m of the highest flood level to the Karbala and Narhaiya talabs in Raipur and to the Jhanjh wetland in Nava Raipur Atal Nagar, where in March 2025 it directed NRDA to withdraw two tenders (as reported). Village tanks vest in the State under s.251 of the Chhattisgarh Land Revenue Code 1959, and encroachments on ponds recorded in the revenue record at Baloda were proceeded against under s.248 after an order of the NGT (O.A. 78/2024, order of 14 November 2024). The Municipal Corporation Act 1956 as adapted in Chhattisgarh was not read in its Chhattisgarh text [TBC]; its Madhya Pradesh text vests public tanks, streams and drains in the Corporation (s.82) and lets the Commissioner require a building erected over a drain vested in it to be pulled down (s.198). The State has no floodplain zoning law and no notified flood zone, and no demarcation by WRD of the floodplain of any river was found during research. None of these gives a reserve width along a drain. This Manual therefore gives the reserve a width, as its own proposal and not as a statutory width: a **hydraulic corridor** made up of the reserve of Table 6.9, based on the drain size needed to carry the design flow (Clause 6.9.7), the access strip of Clause 6.9.4 on the access bank, and, along a river, a natural drain or a drain of WRD that drains 8 ha or more, the setbacks of Clause 7.4 measured from its top of bank, with a Zone 1 of at least 15 m and, where the Niyam's distances apply, not less than the 9 m or 15 m measured as above; where a 1% AEP flood extent has been mapped, it governs Zone 2, and where the Niyam, the 50 m wetland line, a development plan or a court sets a wider line, that line applies for as long as it binds (Chapter 7). The corridor is set by this Manual. It should be shown in the development plans made under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973, among them those of Raipur, Durg–Bhilai and Nava

Raipur Atal Nagar, and checked at the T&CP site report and checklist, which already ask for the nala distance and WRD's high flood point, and at the site inspection that the 2018 amendment introduced, rather than made a new NOC.

The reserve matters because the drains of Chhattisgarh's towns are divided among owners, and no rule divides them. The Municipal Corporations, Nagar Palika Parishads and Nagar Panchayats hold the public drains of their areas under the Municipal Corporation Act 1956 and the Municipalities Act 1961, both as adapted (Chhattisgarh texts not read [TBC]). In Nava Raipur Atal Nagar the roads, drains, streams and network of lakes and reservoirs were planned by NRDA, a Special Area Development Authority under the 1973 Adhiniyam (its notification No. 4066 of 23 May 2022 delegates those powers), whose development plan preserves the four major streams of the area and makes the parks "storm water carriers" to the lakes (NRDA, Naya Raipur Development Plan-2031; as reported). RDA and the Chhattisgarh Housing Board build schemes whose drains are later handed over. In Bhilai the drains of the steel plant's township are cleaned by the Town Services of the Bhilai Steel Plant (SAIL), and in July 2025 the Commissioner of the Bhilai Municipal Corporation warned the plant over uncleaned township drains (as reported); the formal split between the two was not found [TBC]. Bilaspur's Arpa special-area authority, whose development plan was approved in 2014, no longer needs to be asked for permission to sell, build on or develop land, after orders of the High Court of 2019 (Housing and Environment Department letter of 14 February 2019). WRD holds the rivers, dams, barrages and canals; the village tanks vest in the State; and in the coalfields and around the power plants the mining lessees, among them SECL, and the power companies, among them NTPC, CSPGCL and BALCO, hold the mine drains, voids, ash ponds and ash dykes of their sites. No rule dividing the natural nalas from the municipal drains was found during research. For the purposes of this Manual, and until the State issues such a rule [TBC], the drains are assigned as follows: rivers, river beds, banks and floodplains, and the dams, reservoirs and barrages of the State, among them Gangrel, Minimata Bango, the Hasdeo barrage at Darri, Kharang, Maniyari, Kelo, Kopra, Mongra, Arpa Bhaisajhar and the barrages on the Mahanadi, with their gates and spillways, flood embankments and irrigation canals, to WRD; the public drains, the drains alongside or under public streets and the nalas within the limits of an urban local body, other than those assigned to WRD, and the tanks vested in it, to its Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat, in Raipur to RMC; the roads, drains, streams, lakes and reservoirs of Nava Raipur Atal Nagar to NRDA; the drains of a scheme of RDA, the Chhattisgarh Housing Board or a colony developer to the body that built it until they are handed over to the local body in writing; the drains of the township of the Bhilai Steel Plant to the plant, and the drains of Bhilai outside the township to the Bhilai and Risali Municipal Corporations [TBC]; the village tanks vested in the State under s.251 of the Land Revenue Code to the Collector, with the rights of irrigation and nistar preserved; the drains of panchayat areas to the gram panchayat; the drains, garland drains, diversion channels, settling ponds and voids within a mining lease to the lessee under its environmental clearance and the rules of DGMS, and the ash ponds, ash dykes and plant drains of a power plant to its operator; and highway drains to PWD or NHAI (set by this Manual). Under Clause 6.9.8(a), the reserve along a WRD drain is transferred to WRD. Where a channel's status is disputed, the corridor follows the channel and not its name: the Kharun at Raipur is both a river and the receiving channel of the city's nalas, which the weirs of the city's new treatment plants intercept (CECB, 2021). No approved city drainage master plan with a design return period was found for any town in the State. The only design storm found for a city's drains is the "5 years Maximum rainfall intensity" of the Naya Raipur Development Plan-2031, a city plan and not a State standard (NRDA, 2008); RMC's plan for storm drainage under AMRUT proposed ₹1,191 crore of works without stating a return period, a rainfall intensity or a runoff coefficient (RMC, 2015). A drain sized to fit the land left beside it cannot be enlarged, which is why the reserve is based on the required size. Until a Basin Stormwater Master Plan is adopted (Clause 6.9.6(b)), the required drain size of Clause 6.9.7 is the size computed by the designer under this chapter. This Manual does not prescribe a lined or an unlined section. It requires the section to meet the velocity limits of Clause 6.7.3 and Chapter 9, and a reserve wide enough for an unlined or vegetated section is a reserve that keeps both options open.

Commentary. Requirements 6.9.2(b)–(e), 6.9.3 and 6.9.4 have no direct precedent and are set by this Manual. Compound walls around individual plots are almost universal in Chhattisgarh and are one of the most common ways that overland flow paths get blocked. The 50% blockage assumption for wall openings is set to match the

grate clogging factor of Clause 6.6.3. The 3.5 m access width is set by this Manual, sized for a small excavator or desilting vehicle. For comparison, APWA 5600 (2006, 5608.4) requires a 15 ft (about 4.6 m) maintenance access strip for detention facilities.

6.10 Ponding at sags and low points

6.10.1 **Minor event.** At every sag, the minor design flow shall be removed by inlets (after clogging) with ponded spread within Table 6.2.

6.10.2 **1% AEP event.** At every sag:

- (a) the ponded depth shall not exceed 350 mm at the deepest point of the carriageway;
- (b) a surface overflow route shall be provided (for example over a lowered kerb and footpath into a designated overland flow path), so that the ponding level cannot rise above the overflow level; and
- (c) building floor levels around the sag shall comply with Clause 6.11.

6.10.3 **Fully-blocked check.** At sags, the 1% AEP ponding level shall also be computed with the minor system fully blocked and all flow leaving by the surface overflow route. Floor levels around the sag shall be set from the higher of the two ponding levels.

6.10.4 **Trapped sags.** Where a sag has no surface overflow route, the inlets and conduits (after clogging) shall carry the 1% AEP flow, and the approving authority shall agree the arrangement.

6.10.5 **Road underpasses.** Underpasses and other trapped sags on public roads shall:

- (a) be drained by a minor system (with pumping under Chapter 9, Clause 9.9, where gravity drainage is not possible) designed for the 2% AEP (50-yr) flow, checked at the 1% AEP flow;
- (b) have flanking inlets at both approaches to intercept flow before it reaches the low point;
- (c) be protected from inflow from surrounding streets by raised approach crests where practicable; and
- (d) have depth markers and automatic warning or closure signals that operate at 150 mm depth on the carriageway.

Commentary. The fully-blocked check, the trapped-sag rule and the underpass requirements have no direct precedent and are set by this Manual. The Fafadih and Gudhiyari underpasses in Raipur were flooded in the storm of 5–6 July 2026, when water stood knee- to waist-deep in Tatibandh, Shankar Nagar and Mowa (as reported); no rainfall depth was reported for the storm [TBC]. The roads already act as the major system: RMC recorded storm drains on 49% of the city's network and 11 road stretches of 50 m or more that are waterlogged two or three times a year (RMC, AMRUT service-level improvement plan for storm water drainage, 2015). Clause 6.10.5(d) links the 150 mm crown depth limit of Table 6.2 to the point at which traffic should stop.

6.11 Building floor levels

6.11.1 The lowest habitable floor, and every opening through which water could enter a building (doors, ventilators, basement ramps, lift pits and service entries), shall be at least **0.5 m above the 1% AEP flood level**.

6.11.2 The 1% AEP flood level is the highest of:

- (a) the 1% AEP major-system water level in the street or overland flow path next to the building (Clauses 6.9 and 6.10, including the fully-blocked check at sags);

- (b) the 1% AEP flood level of any river, channel, tank or waterbody (Chapter 7); and
- (c) the 1% AEP level with the tailwater of Chapter 5; and
- (d) where the Approving Authority has adopted a 1% AEP flood depth map under Clause 7.3.4, the ground level at the plot plus the mapped 1% AEP depth.

6.11.3 **Default where no level has been computed.** For a plot below the Chapter 1 thresholds, or where no 1% AEP level has been computed for the plot, the lowest habitable floor and openings shall be at least the higher of:

- (a) **0.65 m above the highest crown level of the road on which the plot fronts**; and
- (b) where an adopted 1% AEP flood depth map covers the plot, **the ground level plus the mapped 1% AEP depth plus 0.5 m**.

The Approving Authority may adopt a regional map, such as a State-approved map or a published national map of modelled 1% AEP flood depth (for example the India Flood Atlas), until a local study replaces it. A map that does not model storm surge shall not be used in place of Clause 6.11.6.

6.11.4 **Basements.** The crest of every basement ramp and the sill of every basement opening shall meet Clause 6.11.1. Basement drainage shall be pumped to the surface drain through a non-return valve.

6.11.5 **Services.** Electrical panels, transformers, standby generators, lift machinery and pumps that serve a building shall meet Clause 6.11.1.

6.11.6 **Coastal band.** The coastal band is the land within **500 m** landward of the High Tide Line along the sea front, as defined by the CRZ Notification, 2019, with the High Tide Line as demarcated in the approved Coastal Zone Management Plan. Where the Survey of India Hazard Line lies further inland than 500 m, the band extends to the Hazard Line. Within the coastal band, in addition to Clauses 6.11.1 to 6.11.5:

- (a) the lowest habitable floor and every opening shall also be at least **0.5 m above the coastal flood level**, which is the 1% AEP still-water level (Clause 5.7.3(b), Annex 5A) plus **twice** the sea-level allowance of Clause 5.7.4 (so at least 0.6 m), because buildings have a design life of more than 50 years (Clause 5.7.4(c));
- (b) where the plot fronts the open beach, the coastal flood level shall include wave set-up from a site study;
- (c) no habitable room, electrical service room or plant listed in Clause 6.11.5 shall be placed in a basement; and
- (d) every basement opening, ramp crest and ventilation opening shall meet Clause 6.11.6(a).

Clause 6.11.6 does not authorise any construction that the CRZ Notification prohibits. Where both apply, the stricter governs.

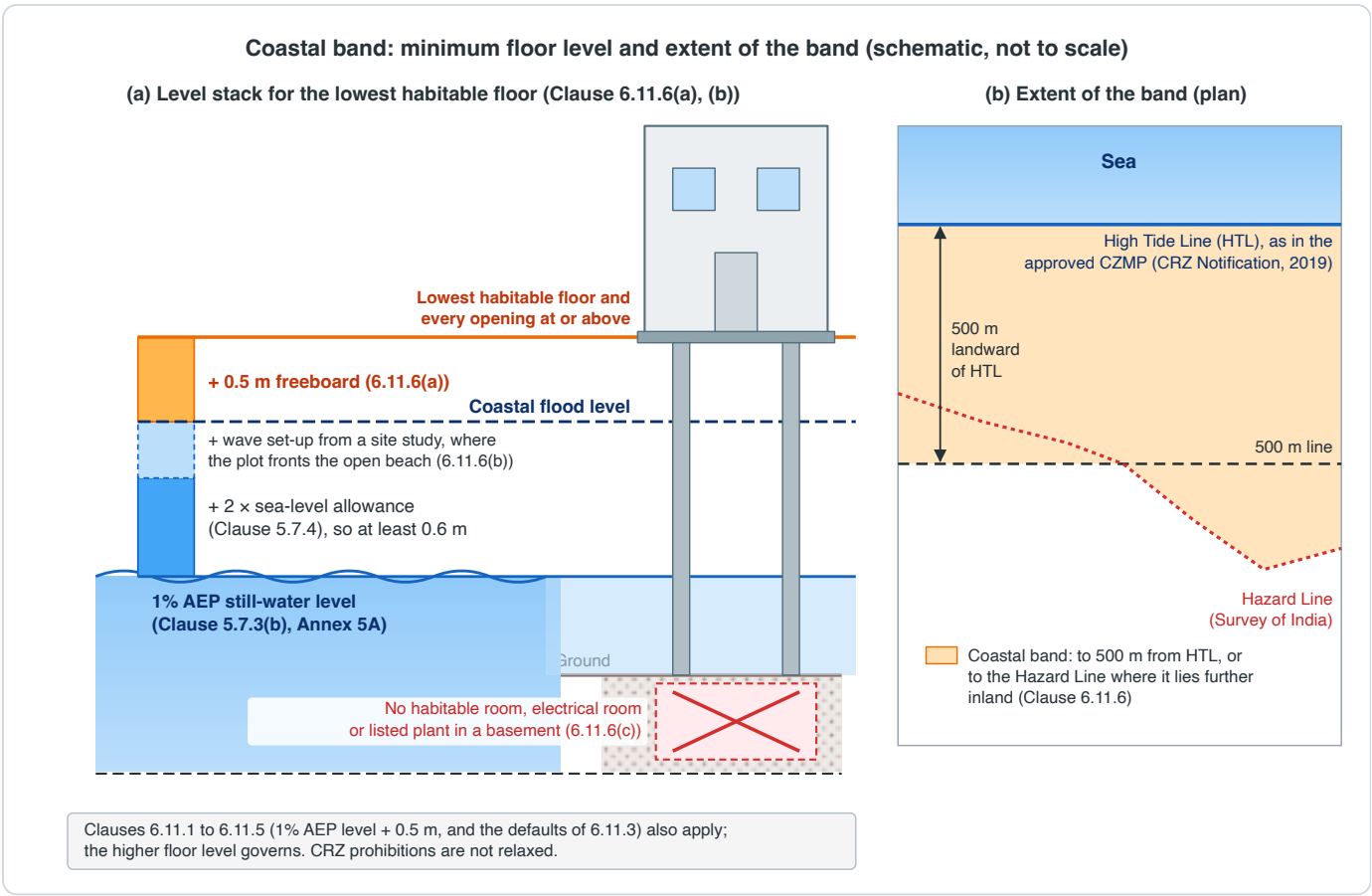


Figure 6.5 — Coastal band: the minimum floor level built up from the 1% AEP still-water level, twice the sea-level allowance, wave set-up where the plot fronts the open beach and 0.5 m freeboard; and the extent of the band, to 500 m from the High Tide Line or to the Hazard Line where it lies further inland (Clause 6.11.6). Original drawing for this Manual, adapted from the CRZ Notification, 2019 (para 1(i) and Annexure IV) and PUB Singapore, Code of Practice on Surface Water Drainage (7th ed.), cl. 2.1.1.

Commentary. The 0.5 m freeboard above the 1% AEP level follows the DID Malaysia 2008 checklist (platform and lowest floor at 100-yr ARI + 0.5 m). It is more than the APWA 5600 (2006, 5601.8 A2) requirement of 1 ft (about 0.3 m) above the 1% stage. The 0.65 m default in Clause 6.11.3 is derived from the other limits of this chapter, not separately sourced: the 1% water level at the crown may be up to 150 mm (Table 6.2), plus the 0.5 m freeboard. It has no direct precedent and applies to plots where no flood level exists. URDPFI (2015) also links minimum plinth levels to flood frequency in its flood-plain zoning, which is covered in Chapter 7.

Clause 6.11.3(b) lets a small plot use a mapped 1% AEP depth where no local study exists, instead of guessing from the road level alone. Modelled national maps are coarse (about 30 m), and most model rainfall and river flooding but not storm surge, which is why 6.11.3 keeps the road-crown check and Clause 6.11.6 sets a separate coastal rule.

The coastal band of Clause 6.11.6 uses the 500 m line of the CRZ Notification, 2019 (G.S.R. 37(E), para 1(i): "the land area from High Tide Line ... to 500 meters on the landward side"), because every coastal authority already maps it and it needs no new survey. The Hazard Line, demarcated by the Survey of India to show the extent of flooding and shoreline change, extends the band where it lies further inland (CRZ Notification, 2019, Annexure IV). The band takes the coastal level from the joint-case still-water level of Chapter 5 rather than from the rainfall map. The higher standard reflects the setting: a coastal flood rises with the sea, cannot be drained while the tide is up, and will rise further with the sea. Singapore takes the same approach with a fixed minimum platform level of 4.0 m above datum on its southern coast and 4.5 m on its northern coast, whatever the rainfall flood level (PUB Code of Practice on Surface Water Drainage, 7th ed., cl. 2.1.1). Each coastal State's tide and surge levels vary too much along its coast for one fixed level, so this Manual uses the computed still-water level instead. Using twice the sea-

level allowance applies the sensitivity case of Clause 5.7.4(c) as the design case, because a building, unlike a drain, cannot easily be raised later. The basement restriction in 6.11.6(c) is set by this Manual: a basement in the coastal band fills from the sea as well as from rain, and cannot be pumped out to a tide-locked outfall. Chhattisgarh has no coast, no High Tide Line and no Coastal Regulation Zone, so the coastal band is empty in the State and Clause 6.11.6 does not apply there. Clauses 6.11.3 and 6.11.4 matter instead. The plinth and basement rules of the Chhattisgarh Bhumi Vikas Niyam 1984 were not read in their Chhattisgarh text [TBC]. The amendment of 6 October 2018 added a plinth-level inspection and a plinth certificate, and substituted r.73(3)(क), under which a basement may be approved after leaving 3 m clear from the plot boundary for a building up to 18 m high and 6 m above that; neither it nor the amendment of 29 April 2020 sets a plinth height, bars a basement in low-lying or flood-affected land, or defines either; and in 2009 the T&CP Directorate required its own approval for basements of less than 10,000 sq ft because basement space was being misused. In Nava Raipur Atal Nagar the development plan keeps road levels at least 0.5 to 1.0 m above the observed high flood level (NRDA, Naya Raipur Development Plan-2031), so a floor set under Clause 6.11.3 from the road crown stands above it. Clause 6.11.3, which sets the lowest habitable floor and every opening 0.65 m above the road crown where no flood level has been computed, and Clause 6.11.4, which applies the same level, or 0.5 m above the 1% AEP flood level, to the crest of every basement ramp and the sill of every basement opening, are set by this Manual and add to the Niyam; the plinth certificate of 2018 (r.96 and Appendix Q1) checks location, setbacks and deviations but no level, and is the natural stage at which to add a check of them (Chapter 10). Near the talabs, lakes and reservoirs, along the rivers and nalas and in the Hirakud backwater the lines of Chapter 7 also apply, and in the coalfields a plot on mine-disturbed ground shall take its floor and ramp levels from a level survey made for the design (Clause 5.8.5(e)).

6.12 Culverts and road crossings

6.12.1 **Design AEP.** A culvert or bridge that carries a road over a drain or waterway shall pass the design AEP flow of Table 6.1 for that road class without overtopping the road, and with the headwater limits of Chapter 9, Clause 9.4. Where the waterway is a State highway or national highway crossing, the design AEP shall also be not less than that of IRC:SP:42-2014 §7.1.4 for the span. The rarer AEP governs.

6.12.2 **Check event and 1% AEP.** At the check event and at the 1% AEP event, the crossing and the road together shall pass the flow:

- (a) with overtopping depth ≤ 150 mm at the road crown and ≤ 350 mm at the deepest point of the road;
- (b) within the hazard limits of Table 6.3;
- (c) with no overtopping at all of arterial roads that are notified evacuation routes; and
- (d) without raising the 1% AEP flood level at any upstream building or property boundary above the level for the existing condition, unless the affected owners and the approving authority agree.

6.12.3 **Where overtopping is not permitted.** Overtopping in the 1% AEP event is permitted only where the structure span is less than 6 m **and** the 1% AEP peak flow is not more than 7 m³/s, unless a guard rail is installed downstream of the overtopping section. Otherwise, the crossing shall pass the 1% AEP flow without overtopping.

6.12.4 **Blockage check.** The 1% AEP check shall be repeated with a blocked inlet:

- (a) 50% of the inlet area blocked for culverts with a span or diameter below 1.5 m; and
- (b) 20% blocked for larger culverts.

The results of this check shall meet Clause 6.12.2(a) and (d).

6.12.5 **Minimum size.** Culverts under public roads shall be at least 900 mm in diameter, or 900 mm × 900 mm for box culverts.

6.12.6 **Hydraulic design.** Culvert hydraulics, including inlet and outlet control, tailwater, road overtopping and outlet protection, shall follow Chapter 9, Clauses 9.4 and 9.6.

> Commentary. The design AEPs are those of Table 6.1. The overtopping limits are the street-depth limits of Table 6.2. The span and flow threshold in Clause 6.12.3 is the SI conversion of APWA 5600 (2006): overtopping allowed only where the structure span is under 20 ft and the 1% peak is 250 cfs or less, unless a downstream guard fence is installed. IRC:SP:42-2014 (§7.1.4) sets 25-yr for NH/SH culverts up to 2 m span and 50-yr for 2–6 m spans. The blockage percentages in Clause 6.12.4 and the 900 mm minimum size have no direct precedent and are set by this Manual. The 50% value matches the grate clogging factor.

6.13 Chhattisgarh construction-practice requirements

6.13.1 **Covered roadside drains.** Where a roadside drain is covered by slabs that form a footpath:

- (a) it shall be designed as an open channel with the freeboard of Table 6.7 measured to the soffit of the cover slab, and its HGL shall meet Clause 6.8 measured to the footpath and inlet levels;
- (b) road runoff shall enter through openings in the drain wall at road level (Clause 6.6.1(b)), each with a silt sump (Clause 6.6.7), at the spacing of Clause 6.6.4;
- (c) footpath runoff shall enter through slots or grated openings in the cover, not by ponding on the footpath; and
- (d) the clear section shall meet Clause 6.7.6(c).

6.13.2 **Removable covers and access openings.** Covered drains shall have:

- (a) removable cover panels at not more than **10 m** centres, each at least 0.6 m long and spanning the full clear width, with recessed lifting points, liftable by two workers or by a hand-operated lifting device, and flush with the footpath;
- (b) access chambers with a clear opening of at least 0.6 m × 0.6 m at every junction, bend and change of section, and at not more than **30 m** centres on straight runs; and
- (c) covers rated for the traffic or pedestrian load at their location (Chapter 9, Clause 9.11).

6.13.3 **Silt traps.** In addition to the inlet sumps of Clause 6.6.7, silt trap chambers shall be provided:

- (a) upstream of every culvert under an arterial road;
- (b) upstream of every pump station and detention facility; and
- (c) at every outfall to a tank, river or other waterbody (with the gross pollutant trap required by Chapter 8).

Each silt trap shall be sized to hold the sediment expected between scheduled cleanings, using the sediment yield in Annex 6B, and shall be reachable by a desilting vehicle.

6.13.4 **Garbage and gross pollutant screens.** Screens shall be provided where an open drain enters a covered drain, a pipe or a culvert, and at outfalls under Chapter 8. Each screen shall:

- (a) be designed under Chapter 9, Clause 9.8;
- (b) have a bypass or overflow, so that a fully blinded screen does not flood properties or the road beyond the limits of this chapter; and
- (c) be checked hydraulically with 50% of its area blocked.

6.13.5 **No building over drains.** No structure other than an approved crossing or a removable cover shall be built over a public drain. Shop frontages, ramps and steps that cross a drain shall be removable or shall leave access openings that meet Clause 6.13.2.

6.13.6 **Utilities.** No water main, sewer, power or telecom cable, gas pipe or duct shall be laid along, or pass through, the flow area or freeboard of any drain. A utility that must cross a drain shall:

- (a) cross above the cover soffit or below the drain invert, inside a sleeve; and
- (b) be shown on the drainage plan with its level.

Where an existing utility crossing obstructs a drain, its area shall be deducted from the drain section, with a 50% blockage allowance for the debris it traps, until the utility is relocated. The utility owner shall relocate it when the drain is rebuilt.

6.13.7 **Sewage.** No sewage connection is permitted to a storm drain (Chapter 8). Covered drains shall be built with sealed joints where they run next to sewers.

6.13.8 **Edge protection.** Open drains deeper than 1.0 m beside a footpath, a carriageway or a public open space shall have railings or barriers. Open drains of any depth shall not be left uncovered across a footpath.

6.13.9 **As-built levels.** The as-built invert levels of every public drain, pipe and manhole shall be surveyed and submitted (Chapter 10). Inverts shall be within ±15 mm of design. An HGL re-check shall be submitted where they are not.

6.13.10 **Desilting.** Public drains shall be desilted before the south-west monsoon each year, as set out in Chapter 10.

> Commentary. Clause 6.13 has no direct precedent; this Manual sets it on the basis of Chhattisgarh practice. Most urban drains in the State are covered roadside drains under footpaths. They are often built with continuous cast-in-situ slabs and few openings, so they cannot be cleaned and fill with silt and solid waste within a season. CPHEEO (2019) notes that uncovered channels "are prone to the dumping of garbage" (Part A, Ch 2), which is why the drains are covered. Covering without access openings simply moves the problem out of sight. NDMA (2010) recommends that pre-monsoon desilting be complete by 31 March. In Chhattisgarh the south-west monsoon normally sets in at Jagdalpur on 11–13 June, at Raipur and Pendra Road on 16 June and at Bilaspur on 17 June, and normally withdraws from Pendra Road on 7 October, Bilaspur on 8 October, Raipur on 9 October and Jagdalpur on 12–14 October (IMD, new normals); IMD's Meteorological Centre at Raipur uses 10 June and 15 October for the State. So Clause 6.13.10 refers to the south-west monsoon, and Chapter 10 requires desilting and drain works to be complete by 31 May, or by 25 May in the Bastar division, where the monsoon arrives first; the gates, flap gates, pumps and standby generators are certified ready and test-run by 10 June (Clauses 5.11.5 and 5.12.9), before the normal onset everywhere in the State; and pumps and gates stay on standby until 20 October, or 31 October in the Bastar division, after the last normal withdrawal, because 21 of the 100 storms and depressions that affected the State between 1891 and 2014 came in October (IMD). Large storms also fall in June: Durg recorded 337.0 mm on 15 June 2004 and Raipur 297.0 mm on 30 June 2007 (IMD). No desilting order of UADD or of a Municipal Corporation with a completion date was found during research [TBC]; after the Raipur flooding of July 2026 residents blamed inadequate desilting (as reported). These dates are therefore set by this Manual.
>
> No rule of the Chhattisgarh Bhumi Vikas Niyam 1984 on building over a drain was read in its Chhattisgarh text [TBC]. The Municipal Corporation Act 1956, as adapted, was not read in its Chhattisgarh form either; in its Madhya Pradesh text it lets the Commissioner require a building erected over a drain vested in the Corporation to be pulled down (s.198) [TBC]. Natural drains and the drains of WRD are open channels with a drainage reserve (Clause 6.9), and should not be covered, decked or built over: Raipur's drains are reported to have been narrowed by encroachment from seven or eight feet to four or five (as reported). Where a drain is covered, Clause 6.13.2 applies to it in full; where it is open beside a footpath or a carriageway, Clause 6.13.8 applies.
>
> Clause 6.13.7 is not yet the practice. In January 2021 the Chhattisgarh Environment Conservation Board told the NGT that a sewerage system existed only in Bilaspur (CECB, 2021); in Raipur, Durg, Bhilai and the other towns, sullage and septic-tank effluent run in the storm drains and nalas to the Kharun, the Seonath and the Arpa, and the

treatment plants built for them intercept the nalas at their outfalls. Clause 8.9 sets how dry-weather flow in storm drains is surveyed and intercepted until the connections are removed, including the rule that interception shall not reduce the capacity of the drain (Clause 8.9.5(b)).

The 10 m and 30 m spacings and the ±15 mm tolerance have no direct precedent and are set by this Manual. Clause 6.13.6 responds to the common sight of water mains and cables running through drains. Each one traps debris and reduces the section. Mechanical cleaning through cover openings, rather than manual entry, is assumed throughout, consistent with the Prohibition of Employment as Manual Scavengers and their Rehabilitation Act, 2013.

6.14 Outfalls

6.14.1 Outfalls shall be designed for the tailwater conditions of Chapter 5. Outfalls below the 1% AEP still-water level shall have flap or tide gates or non-return valves (Chapter 5 and Chapter 9, Clause 9.10).

6.14.2 Outfall protection and energy dissipation shall be designed under Chapter 9, Clause 9.6. Outfalls to tanks and waterbodies shall also meet Chapters 7 and 8.

6.15 Submittal requirements

Every conveyance submittal shall include a **Conveyance Design Report** (Form 6-1) containing:

1. a catchment plan showing sub-catchments, the minor system, inlet locations and the major-system overland flow paths for the 1% AEP event (Clause 6.3.2);
2. the land-use and road-class AEP adopted for each element (Table 6.1), and the high-density area status;
3. design flows, with the rainfall sheet of Chapter 2 and the method of Chapter 3;
4. gutter spread, depth and D·V checks for the minor, check and 1% events (Tables 6.2 and 6.3);
5. an inlet schedule with clean capacity, clogging factor, design capacity and bypass flow (Clause 6.6);
6. pipe and drain design tables with n, velocity, sediment allowance and freeboard (Clause 6.7);
7. HGL profiles for the minor and check events, with the tailwater used (Clause 6.8);
8. sag ponding levels, including the fully-blocked check (Clause 6.10);
9. a floor-level schedule for every building (Clause 6.11);
10. culvert and crossing calculations (Clause 6.12 and Chapter 9); and
11. a utilities-crossing schedule and access-opening layout for covered drains (Clause 6.13).

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Annexes and forms

Annex / Form	Content
6A	Standard inlet types for Chhattisgarh with capacity curves (on grade and sag)
6B	Sediment yield for silt-trap sizing and calibration of the sediment allowance
6C	Standard drawings: covered roadside drain with removable covers, access chamber, inlet with silt sump
Form 6-1	Conveyance Design Report checklist

CHAPTER 7

Waterbodies, Talabs (Ponds) and Setbacks

7.1 Purpose and scope

7.1.1 This chapter treats waterbodies as part of the drainage system. It sets the rules for:

- (a) identifying and mapping every waterbody that a development or public work affects;
- (b) the setback zones around waterbodies, and the uses allowed in each zone;
- (c) fill in flood-prone land, and the compensatory storage needed where fill is allowed;
- (d) counting tank (talab) storage, including tank cascades, as design storage;
- (e) restoring and desilting tanks and ponds;
- (f) protecting supply channels, surplus courses, inlets and outlets;
- (g) wetlands, marshes and estuaries, which in Chhattisgarh, with no coast and no tidal water, means Kopra Jalashay near Bilaspur, the State's first Ramsar site, any wetland notified under the 2017 Rules (no notified wetland was found), the talabs and wetlands of more than 2.25 ha that the Chhattisgarh State Wetland Authority treats as protected, such as Karbala Talab and Narhaiya Talab at Raipur and the Jhanjh wetland at Nava Raipur Atal Nagar (as reported), the city talabs and the lakes and reservoirs of Nava Raipur Atal Nagar, the floodplain wetlands and depressions of the rivers, mine pit lakes, and the talabs, bandhs and dabris recorded in the Revenue records;
- (h) the interface with encroachment and its removal;
- (i) fee-in-lieu contributions to regional tank storage (with Chapter 4); and
- (j) the safety of tank bunds and other embankments.

7.1.2 This chapter applies to every development and public work to which this Manual applies under Chapter 1 and that:

- (a) adjoins a waterbody or lies wholly or partly within its setback (Clause 7.4);
- (b) discharges to a waterbody, directly or through a drain, within the downstream analysis extent of Chapter 5; or
- (c) alters a waterbody, its bund, sluice, surplus weir, supply channel or surplus course.

7.1.3 Where a statute or statutory rule sets a wider setback or a stricter restriction, the statute governs (Clause 1.3.3). Nothing in this chapter authorises any work that the Chhattisgarh Land Revenue Code, 1959 (sections 237, 248 and 251), the Municipal Corporation Act, 1956 and the Municipalities Act, 1961 as adapted in Chhattisgarh, the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973 and the CGBVN 1984 made under it, the Chhattisgarh Act of 2002 on the regularisation of unauthorised development (section 7(1)(iv)), the Wetlands (Conservation and Management) Rules, 2017, or any other law, prohibits.

> **Commentary.** Chhattisgarh has no Act for the protection of lakes, tanks or ponds as such. Its talabs, dabris and reservoirs are protected instead by revenue law, municipal law, the planning law and the building rules made under it, the wetland rules and the courts. In the villages, every tank on unoccupied land over which the village community exercised rights of irrigation or nistar vested in the State "6 अप्रैल 1959 से राज्य सरकार में पूर्णरूपेण" (absolutely, from 6 April 1959), without taking away those rights (Chhattisgarh Land Revenue Code, 1959, s. 251, read in a third-party copy); the Collector may set land apart for the exercise of nistar, tanks among it (s. 237); and a person who occupies such land without authority is penalised and may be evicted summarily by the Tehsildar (s. 248). That route has been used: on the order of the National Green Tribunal (Central Zone) of 14 November

2024 directing the Collector to remove the encroachments on land "recorded as a pond, a water body" in Nagar Panchayat Baloda, proceedings were begun under section 248 against those who had encroached on the ponds (O.A. 78/2024, Tahsildar's report of 27 December 2024). In the towns, public tanks, streams and drains vest in the local body under the Municipal Corporation Act, 1956 and the Municipalities Act, 1961, Madhya Pradesh Acts adapted by Chhattisgarh. Their Chhattisgarh texts were not read for this Manual; the Madhya Pradesh text of the 1956 Act vests public tanks, streams and drains in the Corporation and bars a vested tank from "any other purpose" (s. 82), keeps drains and latrines a hundred feet from a tank used for public water supply (s. 199), and keeps an older sanitary power that runs the other way, under which an owner may be required to fill up or drain off a private tank that is injurious to health (s. 202), and whether the Chhattisgarh text is the same is to be confirmed [TBC]. **This Manual treats every recorded talab as drainage infrastructure.** A talab is storage and conveyance in the drainage system, and filling one, under a sanitary power or otherwise, removes that storage; no design under this Manual shall assume or rely on the filling of a recorded talab unless a drainage assessment under Chapters 3 to 5 shows where its storage goes, and the lost storage is replaced under Clause 7.6.4 (see "Recorded talab" in the definitions after Clause 7.2). A pond that is a health risk is to be treated (desilting, inlet screening and exclusion of sewage, Clauses 7.9 and 8.9), not filled. The planning law adds three bars. The Chhattisgarh Act of 2002 on the regularisation of unauthorised development forbids the regularisation of construction that stands "जलाशय के तल या जलाशय के किनारे या किसी प्राकृतिक जल निकास पर", on the bed or the edge of a reservoir or tank or on a natural drain (s. 7(1)(iv)); the CGBVN 1984 refuse a building site that is unsuitable under Rule 50, which in the inherited 1984 text covers sites near water and sites that are not properly drained (Chhattisgarh text [TBC]; Clause 7.4, Disclosure); and integrated townships may not be laid out on land beside a perennial river (Appendix ण, 17 December 2008). Wetlands are the concern of the **Chhattisgarh State Wetland Authority**, reconstituted by an order of the General Administration Department of 17 December 2021 according to its report to the National Green Tribunal (another source gives 1 July 2020), with the Forest and Climate Change Department as its nodal department; it has stopped works within 50 m of the highest flood level of Raipur's talabs and next to the Jhanjh wetland at Nava Raipur Atal Nagar (as reported). Kopra Jalashay near Bilaspur is the State's first Ramsar site. No State law or order zoning the floodplains of the rivers was found for this Manual, and no demarcation by WRD of any river's floodplain (Clause 7.4, Disclosure). Two features set Chhattisgarh apart. The first is the city talab cluster of the plain: Raipur is a city of talabs, reported to have had about 130 of them, of which about 37 to 40 remain (as reported; the counts conflict), linked by colony nalas and overflows that run to the Kharun, and its smaller towns and villages keep the same pattern of talabs, bandhs and dabris on the low kanhar and dorsa ground. The second is the coalfield and the power plants of Korba, Raigarh and Surguja, where mine voids and pit lakes are waterbodies held by a lessee under the mining rules and the environmental clearance, overburden dumps are among the heaviest sources of runoff and sediment, and the ash ponds of the power plants are held behind dykes that have breached (as reported); this chapter brings them into the inventory and sets the conditions on which they may be treated as receiving water (definitions after Clause 7.2). This chapter gives the design rules that protect waterbody storage and conveyance. It does not replace the revenue, municipal, planning, mining and wetland procedures that record and protect waterbodies in law, or the directions of the High Court of Chhattisgarh and the National Green Tribunal.

7.2 Definitions

Term	Meaning in this Manual
Waterbody	A river (such as the Mahanadi, the Seonath, the Kharun, the Arpa, the Hasdeo, the Mand, the Kelo, the Maniyari, the Indravati and the Sabari), natural drain or nala (narwa), canal, reservoir, jalashay or barrage pond (such as Gangrel (Ravishankar Sagar), Minimata Bango, Kharang, Maniyari, the Kelo dam and Kopra Jalashay, and the ponds of the barrages at Darri, Mongra and Bhaisajhar), lake (such as the lakes and reservoirs of Nava Raipur Atal Nagar, among them Jhanjh), talab or pond (such as Budha Talab, Telibandha, Karbala Talab and Narhaiya Talab at Raipur), bandh (a tank held by an earthen bund, such as Maharajbandh at Raipur), dabri (a small dug pond), kund (a small walled or embanked tank), marsh or low wet land, floodplain wetland or depression, mine void or pit lake, ash pond, abandoned quarry or borrow pit, supply channel or surplus course, perennial or seasonal (as in Chapter 1).

Term	Meaning in this Manual
Tank (talab)	A waterbody impounded by an earthen bund, fed by its own catchment and/or a supply channel, with a surplus weir and one or more sluices.
Tank cascade	A chain of tanks in which the surplus or sluice outflow of one tank feeds the next (as in Chapter 3).
Full tank level (FTL)	The crest level of the surplus weir (spillway, overflow or outlet) of a tank, or the permanent crest of its regulating structure. The tank holds water up to FTL without spilling.
Maximum water level (MWL)	The highest water level in a tank when the surplus weir passes the tank's design flood. For this Manual, it is the greater of the level recorded by WRD and the level computed for the 1% AEP (100-yr) inflow with the climate factor of Clause 2.8 (Clause 7.7.4).
Top bund level (TBL)	The crest level of the tank bund.
Waterspread	The area of a tank submerged at FTL.
Foreshore	The land around a tank between the FTL contour and the MWL contour. Foreshore land is flooded whenever the tank surpluses, and is part of the tank's flood storage.
Supply channel (feeder channel, nala, narwa or canal)	A channel that brings water from a river, anicut or upstream tank into a tank.
Surplus course	The channel that carries the overflow of a surplus weir to the next tank, a river, a drain or the sea.
Sluice (sluice, sluice gate or regulator)	A controlled outlet through a tank bund, usually to an irrigation channel.
Top of bank	For a channel, the break in slope at the edge of the channel. Where it cannot be identified, the edge of the water surface at the 50% AEP (2-yr) flow.
1% AEP flood extent	The land flooded by the 1% AEP (100-yr) flood with the climate factor of Clause 2.8, from a hydraulic model that meets Chapter 3 and Chapter 5, or from a map adopted by the Authority under Clause 7.3.4.
Floodway	The waterbody and the adjoining land that must be kept free of obstruction to pass the 1% AEP flood without raising the 1% AEP water level by more than **0.1 m** at any point (Clause 7.6.1). For a tank, all land below MWL.
Flood fringe	The part of the 1% AEP flood extent outside the floodway.
Zone 1 (waterbody protection zone)	The inner setback zone defined in Clause 7.4.3.
Zone 2 (flood fringe setback)	The outer setback zone defined in Clause 7.4.4.
Setback	Zone 1 and Zone 2 together.
Compensatory storage	Excavated storage that replaces flood storage lost to fill, under Clause 7.6.4.
Flood cushion	Storage below FTL kept empty, under a documented operating rule, to absorb a flood (Clause 7.8).

Chhattisgarh terms. The following terms are used in this chapter in addition to those above (see also Annex 7E):

Term	Meaning in this Manual
Talab (तालाब)	A natural or excavated depression that stores rainfall runoff: "Pond, pool, tank, reservoir of water" in Platts, and "जलाशय । सरोवर । पोखरा" in the Shabdasagara (Annex 7E). A talab is a tank or pond under Clause 7.2, such as Budha Talab, Karbala Talab and Narhaiya Talab at Raipur. Many talabs have no surplus weir or sluice; where a talab has none, its FTL is the level at which it spills to a nali, a nala or low ground, found by survey. A village tank vested in the State under section 251 of the Chhattisgarh Land Revenue Code, 1959, and a public tank vested in a local body under the Municipal Acts as adapted in Chhattisgarh, are talabs for this chapter.
Recorded talab	A talab, bandh, dabri, kund, lake or reservoir shown in the Revenue records (as land "recorded as a pond, a water body" or otherwise), the local body's records, a Development Plan, a T&CP site report, a list of the Chhattisgarh State Wetland Authority, or a list filed before a court or the National Green Tribunal. A recorded talab is part of the drainage system for this Manual. Where it is proposed to be filled, in whole or part, including under a sanitary power of a Municipal Act, a drainage assessment under Chapters 3 to 5 shall show the storage and flow paths lost and their effect upstream and downstream, and the lost storage shall be replaced under Clause 7.6.4 before any design relying on the filling is approved. This definition and rule are set by this Manual.
Monsoon water line	The outer edge of a talab's, lake's, reservoir's or wetland's water in the monsoon: the larger of the edge shown by dated imagery or survey of the monsoon (July to September) in recent years and the edge recorded in the Revenue records, and where neither is available, the MWL contour. The boundary of a talab or lake under Clauses 7.3.3 and 7.13.1 is the largest of its Revenue record boundary, its surveyed FTL and its monsoon water line, and setbacks are measured from that boundary. This definition is set by this Manual.
Highest water mark; high flood mark; highest flood level	The inherited 1984 text of Rule 50(b) of the CGBVN 1984 measures its distances from "the highest water mark" and, for a major watercourse, from the "average high flood mark" (Chhattisgarh text not read [TBC]); the Chhattisgarh State Wetland Authority measures its 50 m from the highest flood level (as reported); and none of these is defined. For this Manual, the highest water mark, high flood mark or highest flood level of a talab, lake, reservoir or wetland is taken to be no lower than the higher of its FTL and its monsoon water line, and that of a channel no lower than its top of bank; where WRD has given a high flood point for the site under the T&CP checklist of 2011, the line is no lower than that point. This reading is set by this Manual; it is not a statutory interpretation, and a court or the State Government may read the rule otherwise.
City talab cluster; talab-locked outfall	A city talab cluster is a group of talabs within or next to a town, linked by colony nalas, overflow channels and the low ground between them, that drains by a nala to a river, such as the talabs of Raipur, which drain to the Kharun (Figure 7.3). The talabs of a cluster are modelled together as a cascade (Clause 7.7.2). A talab-locked outfall is a drain outfall into a talab, lake or reservoir that cannot discharge freely while the water is at or above its FTL or spill level. Its tailwater is the water level: the MWL, or the FTL plus the surcharge of the design flood, as the custodian records them, or, where neither is recorded, FTL + 0.5 m, a value set by this Manual (Chapter 5).
Dabri; bandh; kund	A dabri (डबरी) is "छोटा गड्ढा या ताल", a small pit or pond, in the Shabdasagara (Annex 7E); in this Manual it is a small dug pond of a farm or village, such as those dug under the State's rural schemes. A dabri is a waterbody for Clause 7.3 and a tank in a cascade for Clause 7.7.2, but its storage is never counted as design storage, because it is not surveyed, has no outlet and is usually full by the time of the peak storm. A bandh (बाँध) is an embankment or dam (Annex 7E), and in Raipur the name of embanked tanks such as Maharajbandh and Telibandha; a bandh is a tank under Clause 7.2 and its bund a tank bund (Clause 7.14.2). A kund (कुंड) is a small embanked or walled tank (Annex 7E); it is a waterbody for Clause 7.3, and a tank or pond where it receives runoff.
Nala; narwa; nali	A nala is a channel that runs a long way across the land, by which rainwater goes to a river (Annex 7E), such as the Shankar and Pulgaon nalas of Durg; narwa (नरवा) is given by the Shabdasagara as the same word, and is the stream word of the State's Narwa–Garuwa–Ghuruwa–Bari programme (as reported); a nali is a small street or house drain. Every nala or narwa that drains 8 ha or more is a natural channel under Clause 7.4.1(b), whoever maintains it and whether or not it is lined, piped or covered. No rule dividing natural nalas from municipal drains was found; Chapter 6 gives this Manual's working assignment, and the definition, not the owner or the name, decides which clauses apply.
Nistar	The rights of common use of village land and water by the village community, which the vesting of a tank in the State does not take away (Chhattisgarh Land Revenue Code, 1959, s. 251), and for which the Collector may set land apart, including tanks (s. 237) (Annex 7E). Bathing and washing at a talab's ghats, and watering cattle, are nistar uses. Nistar and irrigation rights in a talab are uses of it that Clause 7.8.2(d) protects, and the ghats are part of the talab for this chapter.

Term	Meaning in this Manual
Kanhar; dorsa; matasi; bhata	The four traditional land and soil classes of the Chhattisgarh plain, from the lowest ground to the highest: kanhar, deep black cracking clay (Vertisol); dorsa, clay loam, vertic in places (Alfisol); matasi, sandy loam to silt loam (Inceptisol); and bhata, thin gravelly soil less than 30 cm deep over murram or ferricrete (Entisol) (agronomy papers; Annex 7E). The city talabs and the paddy land lie on kanhar and dorsa. Kanhar and puddled paddy lowland within the 1% AEP extent are flood storage under Clause 7.6, and no infiltration facility is placed on kanhar (Clause 8.8.1).
Mine void; pit lake	A mine void is an opencast excavation left open in a coal or mineral lease, worked out or not; a pit lake is a void that holds water. A mine void or pit lake is a waterbody for Clause 7.3 and is recorded in the inventory with its lessee as custodian. It is **not** a design outfall, and its storage is **never** counted under Clause 7.7, unless the lessee and the DGMS consent in writing and its water level, spill level and spill path are surveyed; even then it is treated as a tank under Clauses 7.7, 7.8 and 7.14, with a stability check of its benches and highwalls and a water-quality plan. No development shall drain into an active mine or working (Coal Mines Regulations, 2017, reg. 149, as reported). A fenced pit lake to which the public has no access is not subject to Clause 7.9.4, but its fence and warning signs are maintained by the lessee. This definition and rule are set by this Manual.
Ash pond; ash dyke	An ash pond is a pond, held by an earthen ash dyke, into which a thermal power plant discharges its ash as slurry, such as the ash ponds of NTPC at Dhanras, of CSPGCL at Korba West and of BALCO near Korba. An ash pond is a waterbody for Clause 7.3, recorded with its operator as custodian; it is **never** a design outfall or design storage, no drain under this Manual discharges to it, and the path of a breach of its dyke is a hazard to be mapped (Chapter 5), not a design inflow. Ash dykes in the Korba area are reported to have breached, or to have released slurry to nalas and the Hasdeo, in 2024 to 2026 (Clause 7.14, Commentary). This definition and rule are set by this Manual.
Overburden dump; mine-disturbed ground	An overburden dump is a heap of the rock and soil removed to reach a mineral. Mine-disturbed ground is overburden dumps, backfilled or reclaimed mine land, compacted mine benches and haul roads, and subsidence ground; ground filled with ash is treated in the same way. For this Manual it is HSG D for every storm, with no infiltration credit (Chapter 3); runoff from it passes a sediment basin and stable toe drains (garland drains) before it reaches any urban drain, talab or outfall (Clause 8.12); and no overburden, spoil, ash or silt is placed in the setback of any waterbody (Clause 7.5.1).

7.3 Inventory and mapping of waterbodies

7.3.1 **Statewide inventory.** The Authority, with WRD and the Revenue and Disaster Management Department, shall maintain a GIS layer of every waterbody in its area (Annex 7A). For each waterbody it shall record:

- (a) its type (Clause 7.2), name and survey numbers;
- (b) its custodian (the Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat (in which public tanks, streams, drains and watercourses vest under the Municipal Corporation Act, 1956 and the Municipalities Act, 1961, as adapted in Chhattisgarh; section 82 of the 1956 Act in its Madhya Pradesh text, the Chhattisgarh text not read [TBC]), NRDA (for the lakes, reservoirs, streams and drains of Nava Raipur Atal Nagar), RDA, the Chhattisgarh Housing Board or a colony developer (for the ponds and drains of its scheme until they are handed over), the Bhilai Steel Plant (SAIL) (for the tanks and drains of its township), WRD (for its rivers, dams, reservoirs, barrages, irrigation tanks and canals, such as Gangrel, Minimata Bango, the Hasdeo barrage at Darri, Kharang and Kopra Jalashay), the State Government through the Collector and the Tehsildar (for a tank vested in the State under section 251 of the Chhattisgarh Land Revenue Code, 1959, or land set apart for nistar under section 237), the Gram Panchayat, a mine lessee, such as SECL (for a mine void, pit lake, settling pond or garland drain within its lease), a power company, such as NTPC, CSPGCL or BALCO (for an ash pond or ash dyke), the Chhattisgarh State Wetland Authority (for a notified wetland, or a wetland it treats as protected), a private owner, or another body);
- (c) its boundary as recorded in Revenue records;
- (d) for tanks: FTL, MWL, TBL, the surplus weir crest length and type, sluices, the supply channel and the surplus course, each with its datum;
- (e) its position in any tank cascade, with the upstream and downstream tanks;
- (f) the 1% AEP flood extent, where mapped; and
- (g) known encroachments, with the date recorded.

7.3.2 **Project mapping.** Every drainage report shall include a Waterbody Sheet (Form 7-1) showing all waterbodies:

- (a) within the site;
- (b) within 100 m of the site boundary; and
- (c) downstream, to the extent of the downstream analysis of Chapter 5.

The sheet shall show, on a plan at a scale of 1:1,000 or larger for (a) and (b):

- the Revenue record boundary;
- the surveyed FTL or top of bank;
- the MWL contour (tanks) and the 1% AEP flood extent;
- Zone 1 and Zone 2 (Clause 7.4); and
- every supply channel, surplus course, sluice and surplus weir.

7.3.3 **Sources.** Waterbodies shall be identified from all of the following. Where they conflict, the larger waterbody extent governs:

- (a) Revenue records (khasra) and village maps of the Revenue Department, in which talabs, dabris, nalas and land covered by water are recorded, including the tanks vested in the State under section 251 of the Chhattisgarh Land Revenue Code, 1959 and the land set apart for nistar under section 237, and land "recorded as a pond, a water body" whose unauthorised occupation is proceeded against under section 248 (as at Baloda, NGT O.A. 78/2024); the public tanks, streams, drains and watercourses vested in a Municipal Corporation or other local body under the Municipal Acts as adapted in Chhattisgarh; the water bodies, rivers and nalas shown in a Development Plan under the 1973 Adhiniyam, such as the Raipur Development Plan (Revised) 2021, the Naya Raipur Development Plan-2031 and the Arpa Special Area Development Plan 2033, and the waterlogging areas, nalas and areas left for nalas recorded in the T&CP site reports and checklists (2011), with the high flood points supplied by WRD; the boundary of Kopra Jalashay as designated under the Ramsar Convention, of any wetland notified under the Wetlands (Conservation and Management) Rules, 2017, and of the talabs and wetlands that the Chhattisgarh State Wetland Authority treats as protected, with the survey of the water bodies of Raipur district that it directed on 8 May 2023; the mining plans and mine closure plans of mine lessees, which show voids, pit lakes, settling ponds and garland drains; and the ash pond and ash dyke layouts of the power plants;
- (b) WRD and Gram Panchayat tank registers and records of FTL and MWL;
- (c) the Authority's inventory (Clause 7.3.1) and any list published under court directions;
- (d) historical topographic maps and satellite imagery, including the ISRO National Wetland Inventory and Assessment; and
- (e) a site survey by a licensed surveyor, levelled to the datum stated in Clause 7.3.5.

7.3.4 **Flood extent maps.** The Authority may adopt 1% AEP flood extent or flood depth maps for its area. A depth map adopted under this clause also sets floor levels under Clause 6.11. Where a map has been adopted, it shall be used unless a site hydraulic study under Chapters 3 and 5 shows a larger extent. Where no map has been adopted, the Applicant shall determine the 1% AEP extent by a hydraulic study for:

- (a) any site in a flood-sensitive catchment (Chapter 1);
- (b) any site of 1 ha or more adjoining a waterbody; and
- (c) any site where the Authority so requires.

For other sites, the fixed-width setback of Clause 7.4.6 may be used.

7.3.5 **Datum.** All levels in this chapter shall be stated to one datum, stated on every drawing. Where WRD records use a local or older datum, the conversion to the project datum shall be surveyed and stated.

> **Commentary.** No 1% AEP flood map of any Chhattisgarh city was found, and no design return period for urban drains has been published for any city except Nava Raipur, whose development plan asks for a 5-year intensity (Naya Raipur Development Plan-2031, §10.3.1.1). No State law, rule or WRD order zoning the floodplains of the

rivers was found, and Chhattisgarh is not among the States reported to have enacted the model floodplain zoning bill (as reported). The planning process asks for flood information but supplies none. For land next to a nala, canal, river or watercourse, the T&CP checklist of 2011 asks for "जल संसाधन विभाग से उच्च बाढ़ बिन्दु की जानकारी", the high flood point from WRD, and for the no-objection of WRD or the Revenue Department (No. 1569/स्टेनो/नग्रानि./2011); it is the only flood-level requirement found in the planning process. The T&CP site-report format of the same year asks whether the land is a waterlogging area and whether it lies beside a canal, nala or river, and at what distance, and records the area left for nalas (No. 1680/TCR-2/नग्रानि./2011); and the site-inspection form of the amendment of 6 October 2018 asks whether a major nala is adjacent to the plot and its nearest distance, to comply with a rule of the CGBVN 1984 whose number the Gazette leaves blank (Appendix Q, item 9). The inherited 1984 text asks that a site plan show "the approximate location and size of any water body, flood affected areas and contours at an interval of 0.3 meters" where the site is steeper than 1 in 20 (r. 17(4)(j), as extracted; Chhattisgarh text [TBC]), but "flood affected areas" are mapped by no agency. Few rivers that receive the cities' drains have a published flood level: the CWC gives warning and danger levels in Chhattisgarh only for the Indravati at Jagdalpur, and the highest levels recorded at its other sites, such as 281.68 m on the Kharun at Pathardihi on 29 August 2020, are known only through transcriptions (as reported; Chapter 5). The survey of all the water bodies of Raipur district that the State Wetland Authority directed on 8 May 2023 was reported in 2025 to have been carried out only at Karbala Talab (as reported), and no level, area or capacity of any of Raipur's talabs or of the lakes of Nava Raipur Atal Nagar was found on an official record. Clause 7.3.4 lets the Authority adopt flood maps once they are issued; until then the Applicant must produce the 1% AEP extent for larger and flood-sensitive sites, and the high flood point that WRD gives under the checklist is a minimum that the 1% AEP extent of this Manual, with the climate factor, may exceed but never reduce. Levels shall be stated to one datum (Clause 7.3.5), because no source read for this Manual states the datum of the CWC levels or of the full reservoir levels in WRD's tank gauge reports. Clause 7.3.3 makes the larger of the recorded and surveyed extents govern, because the Revenue boundary and the surveyed water edge often differ, and either may have been reduced by encroachment. The 100 m search band in Clause 7.3.2 is set by this Manual.

7.4 Setbacks

7.4.1 **Waterbodies to which setbacks apply.** Setbacks apply to:

- (a) every waterbody recorded under Clause 7.3.1 or found under Clause 7.3.3, whatever its size; and
- (b) every natural channel, whether recorded or not, that drains **8 ha** or more at the site.

7.4.2 **Two zones.** The setback has two zones, shown in Figure 7.1:

- **Zone 1 (waterbody protection zone)**, next to the waterbody; and
- **Zone 2 (flood fringe setback)**, outside Zone 1.

7.4.3 **Zone 1 width.** Zone 1 is measured outward from the FTL contour (tanks, lakes and ponds) or from the top of bank (channels). Where both apply, use whichever gives the wider zone. Zone 1 extends to the **greatest** of:

- (a) **15 m**, the minimum set by this Manual (no statutory distance from water was read in the Chhattisgarh text of the CGBVN 1984: its Rule 50 on unsuitable sites is inherited from a 1984 text that sets 9 m from the highest water mark and, for a major watercourse, 9 m from the average high flood mark or 15 m from the defined boundary, whichever is more, and whether the Chhattisgarh text keeps these distances is to be confirmed [TBC]; the Chhattisgarh State Wetland Authority applies 50 m from the highest flood level around the talabs and wetlands it treats as protected (as reported); this Manual applies 15 m to every waterbody; see the Disclosure below);
- (b) the outer edge of the floodway (Clause 7.6.1); and
- (c) for a tank, the MWL contour.

7.4.4 **Zone 2 width.** Zone 2 extends from the outer edge of Zone 1 to the **1% AEP flood extent plus 10 m**. Where the 1% AEP flood extent lies inside Zone 1, Zone 2 is 10 m wide.

7.4.5 **Statutory and approval setbacks.** Where a statute, a statutory rule, a condition of planning permission or the approval of WRD for a work on or next to one of its rivers, dams, reservoirs, barrages or canals, or the high flood point and no-objection of WRD or the Revenue Department that the T&CP checklist of 2011 asks for land next to a nala, canal, river or watercourse, the views or permission of the Chhattisgarh State Wetland Authority for a work within 50 m of the highest flood level of a talab or wetland that it treats as protected, or for a regulated activity at Kopra Jalashay or in a notified wetland, the consent of the mine lessee and of the DGMS for a work that drains to, or lies next to, a mine void, pit lake or working, the consent of the operator for a work next to an ash pond or ash dyke, or a direction of the High Court of Chhattisgarh or the National Green Tribunal (such as the order on the ponds of Baloda listed in the Disclosure to Clause 7.4) sets a setback wider than Zone 1 or Zone 2, the wider setback governs for the uses it restricts.

7.4.6 **Fixed-width setback where no 1% AEP extent is available.** Where Clause 7.3.4 does not require a hydraulic study and no map has been adopted, the setback (Zone 1 plus Zone 2) shall be not less than the following, measured as in Clause 7.4.3:

Table 7.1 — Fixed-width setback where no 1% AEP extent is available

Waterbody	Total setback (Zone 1 + Zone 2)	Of which Zone 1
Channel draining 8 ha to 65 ha	25 m	15 m
Channel draining 65 ha to 20 km²	40 m	15 m
Channel draining more than 20 km², and rivers	50 m	20 m
Tank, lake or pond	MWL contour + 25 m	to MWL contour, and not less than 15 m from FTL
Marsh, estuary or backwater	as notified by the Authority, and not less than 50 m	15 m

This table is the alternative S-3 of the Policy Supplement (Annex 1C) and may be adopted under Clause 1.7.

7.4.7 **Minimum in all cases.** No setback under this chapter, and no variance under Clause 1.10, shall reduce Zone 1 below 15 m (Clause 1.10.3(c)).

> **Commentary.** The 15 m minimum is set by this Manual. No statutory distance from water was read in the Chhattisgarh text of the CGBVN 1984, whose base or consolidated text was not found. Rule 50 is still the State's rule on the unsuitability of a building site: the T&CP Directorate's report format of 2011 asks whether a site is unsuitable under it, whether the land is a waterlogging area, and whether it lies beside a canal, nala or river and at what distance (letter No. 1680/TCR-2/नग्रानि./2011 of 23 June 2011). The 1984 text inherited from Madhya Pradesh bars a building site within 9 m of the highest water mark and, for a major watercourse, within "9 meters from average high flood mark or 15 meters from the defined boundary of water course, whichever is more" (r. 50(b), as extracted); whether the Chhattisgarh text keeps these distances is to be confirmed [TBC]. For land next to a nala, canal, river or watercourse, the T&CP checklist of 2011 asks for the high flood point from WRD and the no-objection of WRD or the Revenue Department, but sets no distance (No. 1569/स्टेनो/नग्रानि./2011). Around the talabs and wetlands it treats as protected, among them Karbala Talab and Narhaiya Talab at Raipur and the Jhanjh wetland at Nava Raipur Atal Nagar, the Chhattisgarh State Wetland Authority applies the bar of the Wetlands (Conservation and Management) Rules, 2017 on work within 50 m of the highest flood level (as reported). No distance from a nala as such was found, and Chhattisgarh has no floodplain zoning law. This Manual's 15 m, the greater distance of the inherited Rule 50(b) applied to every talab, nala and watercourse, gives

every waterbody a hydraulic corridor, and is this Manual's proposal, not a statutory width (see the Disclosure below). Extending the setback to the mapped 1% AEP extent plus a margin follows the Kansas City Metro APWA Section 5600 (2025.01) two-zone structure:

- APWA Zone 1 (streamside) is measured from top of bank, and only utility and road crossings are allowed in it.
- APWA Zone 2 is the 1% AEP flood extent plus 10 ft (about 3 m), for streams draining 20–40 acres.
- APWA lowered the trigger for stream setbacks from 40 acres to 20 acres (about 8 ha) in 2025.01.

(MARC, APWA 5600 Policy Supplement, Nov 2025.) Clause 7.4.1(b) adopts the 8 ha trigger. This Manual's 10 m margin is wider than APWA's 10 ft and has no direct precedent. This Manual sets it because Chhattisgarh flood maps are mostly coarser than US maps, the kanhar and dorsa lowlands around the city talabs of the Chhattisgarh plain, where Raipur, Durg–Bhilai and Bilaspur are growing, are flat, with slopes of less than 2 to 3% (so a small error in level moves the flood edge a long way), and the margin gives room for maintenance access along the waterbody.

The fixed-width table in Clause 7.4.6 is set by this Manual. Its channel widths are scaled from the 2006 APWA default stream buffers, which were 40 ft, 60 ft, 100 ft and 120 ft (12, 18, 30 and 37 m) for drainage areas of less than 40 acres, 40–160 acres, 160–5,000 acres and more than 5,000 acres (KC APWA 5600, 2006, §5605.3). They are rounded up and floored at the 15 m minimum. APWA offered a fixed-width table for the same reason: communities objected that the 1% extent is often unmapped for small drainage areas (MARC Supplement, 2025). The table should be replaced by mapped extents as soon as the Authority adopts them.

URDPFI (2015) reserves the area flooded at a 10-year frequency for parks and playgrounds, and allows residential use in the 25-year flood zone only on stilts, with minimum plinth levels and no basements. This chapter is stricter. It keeps habitable buildings out of the whole 1% AEP extent plus margin, because the 1% AEP (100-yr) event is the major-system standard of this Manual (Chapter 6).

Chhattisgarh's building rules are still the 1984 rules inherited from Madhya Pradesh in 2000, retitled the Chhattisgarh Bhumi Vikas Niyam 1984 and amended since by the Housing and Environment Department under s.85(1) read with s.24(3) of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973; no base or consolidated Chhattisgarh text was found, in print or in the archived pages of the T&CP Directorate. Rule 50 is still the State's rule on the unsuitability of a building site: the Directorate's site-report format asks "नियम 50 के अंतर्गत अनुपयुक्तता तो नहीं है", whether the land is unsuitable under Rule 50 (letter No. 1680/TCR-2/नग्रानि./2011 of 23 June 2011). In the inherited 1984 text, the rule bars a site "within a distance of 9 meters of the highest water mark" and, for a major watercourse, within "9 meters from average high flood mark or 15 meters from the defined boundary of water course, whichever is more" (r. 50(b), as extracted from the Madhya Pradesh text); **whether the Chhattisgarh text keeps these distances was not confirmed [TBC]**, and no amendment from 2008 to 2023 read for this Manual changes Rule 50. The rules show that a nala distance exists without showing it: the site-inspection form of the amendment of 6 October 2018 asks "Is Nalah (Major Drain) adjacent to plot or not. If yes, then nearest distance of nalas from plot. (Proposal should comply with in rule of Chhattisgarh Bhumi Vikas Niyam, 1984)", with the rule number left blank (No. F 7-27/2017/32, Appendix Q, item 9). The other lines in the planning law have no width. For land next to a nala, canal, river or watercourse, T&CP asks for the no-objection of WRD or the Revenue Department and the high flood point from WRD (checklist No. 1569/स्टेनो/नग्रानि./2011); integrated townships may not be laid out on land beside a perennial river (Appendix ण(2)(iii), 17 December 2008), nor may permission be given on such land for agricultural land in a planning area without a development plan (Housing and Environment Department order No. F 7-35/2012/32 of 2 January 2013, para 2.4(iii)); and construction on the bed or edge of a reservoir or tank, or on a natural drain, may not be regularised (Chhattisgarh Act of 2002 on the regularisation of unauthorised development, s. 7(1)(iv)). The Municipal Corporation Act, 1956 as adapted in Chhattisgarh was not read; its Madhya Pradesh text adds no width, but lets the Commissioner have a building over a drain vested in the Corporation pulled down if it was built without permission (s. 198) and keeps drains and latrines a hundred feet from a tank used for public water supply (s. 199) [TBC]. The widest line in use is the wetland line. Around the talabs and wetlands of more than 2.25 ha that it treats as "deemed" wetlands under the Supreme Court's rulings in *M.K. Balakrishnan* (2017) and *Anand Arya* (2024), the Chhattisgarh State Wetland Authority applies the bar of

rule 4 of the Wetlands (Conservation and Management) Rules, 2017 on work within 50 m of the highest flood level: it halted works of about ₹1.5 crore at Karbala Talab in Raipur on 30 September 2025 because paver blocks had been laid within 50 m of the highest flood level; a plan for a toy train in the buffer of Narhaiya Talab at Tikrapara led to an inquiry by the Collector as chair of the District Wetland Conservation Committee (2025); and on 3 March 2025 it directed NRDA to withdraw two tenders for plotted development, floating recreation and branded residences next to the Jhanjh wetland at Nava Raipur Atal Nagar, which NRDA had said was not a protected zone (all as reported). Mining leases carry their own lines: the appraisal record of the environmental clearance of 13 August 2024 for a coal mine in Chhattisgarh records a DGMS distance of 15 m from a river bank against a 100 m buffer required by the appraisal committee (as recorded in the clearance). Clause 7.4.7 makes 15 m the floor of Zone 1 everywhere.

Disclosure. Chhattisgarh has **no confirmed statutory setback in metres from water**. Rule 50 of the CGBVN 1984 is the State's rule on unsuitable building sites, but its Chhattisgarh text was not found; the 1984 text it inherits from Madhya Pradesh sets **9 m from the highest water mark and, for a major watercourse, 9 m from the average high flood mark or 15 m from the defined boundary, whichever is more**, and **whether the Chhattisgarh text keeps these distances is to be confirmed [TBC]**. The one distance applied in practice is the **50 m from the highest flood level** that the Chhattisgarh State Wetland Authority applies, under the Wetlands (Conservation and Management) Rules, 2017, to the talabs and wetlands it treats as protected (as reported). **No floodplain zoning law, no notified flood zone and no WRD demarcation of a river's floodplain was found**, and no distance from a nala as such. The **15 m minimum** Zone 1 of Clauses 7.4.3 and 7.4.7, and every value derived from it in Table 7.1, is **set by this Manual** as a hydraulic corridor, to give every talab, nala and watercourse the greater width of the inherited Rule 50(b). So are the setback zones of Clause 7.4, the 1% AEP extent that governs Zone 2, this Manual's readings of the "highest water mark", "high flood mark" and "highest flood level" and of the monsoon water line, and the conditions on mine voids, pit lakes and ash ponds (definitions after Clause 7.2). They are **this Manual's proposal, not a statutory width or rule**, and no government body has adopted or endorsed them. The setback is applied as **the larger of the corridor and any statutory, plan, wetland or court line**, so the Manual is **stricter than statute** along talabs, nalas and watercourses, and no less strict anywhere. What does exist, and governs wherever it is wider or stricter (Clause 7.4.5), is:

- **Rule 50 of the CGBVN 1984 (inherited text; Chhattisgarh text [TBC]).** No building site within 9 m of the highest water mark, or, along a major watercourse, within 9 m of the average high flood mark or 15 m of its defined boundary, whichever is more, and no building on a site that is not properly drained or cannot be well drained. Where the Chhattisgarh text confirms these distances, the 15 m equals Zone 1's floor and the 9 m is narrower and never governs alone; a wider distance in the Chhattisgarh text or in a development plan governs where it applies.
- **The WRD high flood point and no-objection.** For land next to a nala, canal, river or watercourse, T&CP asks for the high flood point from WRD and the no-objection of WRD or the Revenue Department (checklist No. 1569/स्टेनो/नग्रानि./2011). It sets no width, but the high flood point is a level below which Zone 2 never ends (definitions after Clause 7.2).
- **Rivers, reservoirs and natural drains in the planning law.** No integrated township on land beside a perennial river (Appendix ण, 17 December 2008), and no permission on such land for agricultural land in a planning area without a development plan (order of 2 January 2013); and no regularisation of construction on the bed or edge of a reservoir or tank or on a natural drain (Chhattisgarh Act of 2002 on the regularisation of unauthorised development, s. 7(1)(iv)).
- **Wetlands and Kopra.** No work within 50 m of the highest flood level of a talab or wetland that the Chhattisgarh State Wetland Authority treats as protected, applied at Karbala Talab and Narhaiya Talab in Raipur and at the Jhanjh wetland in Nava Raipur Atal Nagar (2025, as reported); no Chhattisgarh wetland was found notified under the Rules. Kopra Jalashay, the State's first Ramsar site (210 ha), is protected to its Ramsar boundary.
- **Tanks in the Revenue records.** Village tanks with rights of irrigation or nistar vest in the State (Chhattisgarh Land Revenue Code, 1959, s. 251), the Collector may set land apart for tanks for nistar (s. 237), and occupation of a pond recorded in the land records is removed as unauthorised occupation (s. 248), as at

Baloda on the order of the National Green Tribunal (O.A. 78/2024, order of 14 November 2024). These protect the waterbody, not a flood corridor; the whole recorded and monsoon extent is a waterbody for this chapter.

- **WRD, mine and ash-dyke lines.** Works on or next to WRD rivers, dams, reservoirs, barrages and canals need its approval; works next to a mine void, pit lake or working need the lessee's and the DGMS's consent, and an environmental clearance may set a wider buffer from a river (as in a clearance of 2024 for a coal mine in the State, which records a DGMS distance of 15 m and a 100 m buffer required by the appraisal committee); works next to an ash pond or ash dyke need its operator's consent.

This Manual sets 15 m as the floor because no width has been confirmed for talabs and nalas in the Chhattisgarh text of the rules; because 15 m is the greater distance of the inherited Rule 50(b); because it is above the smallest APWA default stream buffer (12 m) from which Table 7.1 is scaled; and because it leaves room for the maintenance track and desilting machinery that the nalas need. The reasons for the corridor are the record: Raipur, reported to have had about 130 talabs, is reported to keep about 37 to 40, "all … polluted and heavily silted" (as reported; the counts conflict); in July 2026 five ponds overflowed into homes at Chaubey Colony in Raipur, and drains were reported narrowed from seven or eight feet to four or five by encroachment (as reported); the Raipur Municipal Corporation recorded in 2015 that its storm drains covered 49% of the city, that "Capacity of drain is not sufficient", and that 11 road stretches flooded two or three times a year (AMRUT service-level improvement plan); and the State Wetland Authority found works within the 50 m line at Karbala Talab and at the Jhanjh wetland (2025, as reported). Until a statute, rule, notification, development plan or court adopts it, the 15 m minimum is this Manual's recommended standard, and the statutory, plan, wetland and court controls above remain the only legal minimum. Approving authorities should state in each approval which was applied.

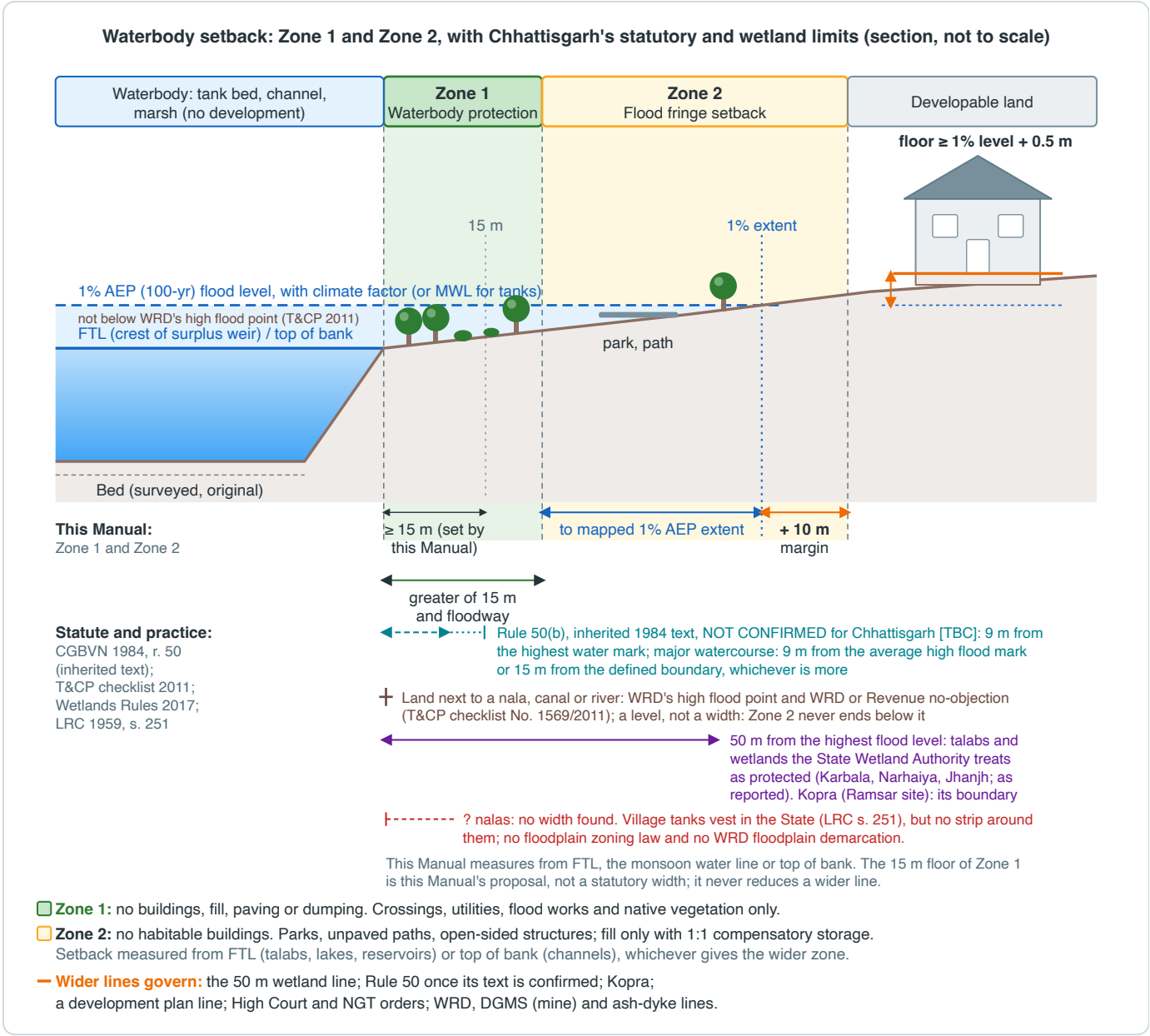

Figure 7.1 — Waterbody setback zones: section through a tank or channel showing FTL, the 1% AEP flood level, the 15 m minimum line, Zone 1 and Zone 2. Original drawing for this Manual, adapted from Kansas City Metro APWA, Section 5600 Version 2025.01 and Policy Supplement (MARC, 2025).

7.5 Uses permitted in each zone

7.5.1 **Zone 1.** No building, fill, paving, retaining wall, boundary wall, compound wall, dumping of earth, debris or solid waste, storage of materials, or construction-stage stockpile shall be placed in Zone 1. Only the following are permitted, each subject to the custodian's NOC:

- (a) road, rail and utility crossings that meet Clause 7.10.3;
- (b) flood and drainage works: outfalls, inlet structures, gross pollutant traps, sluices, surplus weirs, bund strengthening and bank protection;
- (c) native vegetation, planting and habitat works;
- (d) unpaved or permeable footpaths and maintenance tracks at existing ground level; and
- (e) waterbody restoration under Clause 7.9.

7.5.2 **Zone 2.** No habitable building, basement, school, hospital, fuel store or hazardous-material store shall be placed in Zone 2. The following are permitted:

- (a) all uses permitted in Zone 1;
- (b) parks, playgrounds, open-air sports fields and urban agriculture;
- (c) open-sided structures (shelters, pavilions) with no walls that obstruct flow;
- (d) surface parking with permeable surfacing at existing ground level, where the 1% AEP flood depth is not more than 0.3 m;
- (e) retention, detention and water quality facilities under Chapters 4 and 8, provided they are outside the floodway and their storage is not counted twice (Clause 7.6.5); and
- (f) fill, only for (a) to (e) or for essential public infrastructure, and only with compensatory storage under Clause 7.6.4.

7.5.3 **Fences.** Fences in Zone 1 and Zone 2 shall be open (chain-link or railing) with no plinth wall above existing ground level, so that they do not obstruct flow or trap debris.

7.5.4 **Open space reservation.** Zone 2 land may be counted toward the community open space required by the Chhattisgarh Bhumi Vikas Niyam 1984, where the Authority agrees. Zone 1 land shall be handed over to the custodian or kept free of private use, as the Authority decides.

> Commentary. The permitted uses follow APWA 2025.01: in Zone 1, only utility and road crossings; in Zone 2, linear utilities, trails and open-sided structures; no clearing, grading, fill or paving in either zone without approval (MARC Supplement, 2025). Clause 7.5.2 adds parks and playgrounds, the uses URDPFI (2015) allows in the 10-year flood zone. The ban on walls in Clauses 7.5.1 and 7.5.3 responds to urban Chhattisgarh practice: compound walls along channels and tanks act as dams in floods, and are often the first step toward encroachment. The 0.3 m depth limit for parking in Clause 7.5.2(d) has no direct precedent and is set by this Manual.

7.6 Floodway, flood fringe and compensatory storage

7.6.1 **Floodway delineation.** The floodway shall be delineated by a hydraulic model of the 1% AEP flood (with the climate factor), using the encroachment method: the conveyance of the flood fringe is removed equally from both banks until the 1% AEP water level rises by 0.1 m. Where the floodway has not been delineated, the whole 1% AEP flood extent shall be treated as floodway. For a tank, all land below MWL is floodway.

7.6.2 **No fill in the floodway.** No fill, building, embankment, raised road or other obstruction shall be placed below the 1% AEP flood level within the floodway, except the works permitted in Zone 1 under Clause 7.5.1(a) and (b). Those works shall be shown by hydraulic analysis to raise the 1% AEP water level by no more than 0.01 m, upstream or downstream, outside the Applicant's land.

7.6.3 **Fill in the flood fringe.** Fill below the 1% AEP flood level in the flood fringe is permitted only:

- (a) for uses permitted under Clause 7.5.2, or outside the setback where the Authority has notified a developed-fringe area under Clause 7.6.6; and
- (b) with compensatory storage under Clause 7.6.4.

7.6.4 **Compensatory storage.** Where fill is placed in the flood fringe, compensatory storage shall be provided that (Figure 7.2):

- (a) replaces at least **1.0 m³ of storage for each 1.0 m³ of fill** placed below the 1% AEP flood level;
- (b) is provided at **equivalent levels**: the volume lost and the volume replaced shall be balanced in each elevation band of not more than **0.3 m**, from the normal water level (or the 50% AEP level, where higher) up to the 1% AEP flood level;

- (c) is in the same reach of the same waterbody, hydraulically connected to it, and not further downstream than the fill;
- (d) drains freely by gravity back to the waterbody as the flood recedes, with no pumping and no permanent pool below the normal water level;
- (e) is built before, or at the same time as, the fill is placed; and
- (f) is secured by a covenant or handover so that it is never filled later.

7.6.5 **No double counting.** Compensatory storage shall not be counted as detention or retention storage under Chapter 4. Detention and retention storage placed within the 1% AEP flood extent shall be counted only for the volume above the 1% AEP flood level, unless a model of the joint event (Clause 2.12) shows that it is available.

7.6.6 **Developed-fringe areas.** The Authority may notify, by map, areas of existing built-up land within the 1% AEP flood extent but outside the floodway, where it is impractical to apply Zone 2 to redevelopment. In a notified developed-fringe area:

- (a) redevelopment may be approved without the Zone 2 restriction;
- (b) the lowest habitable floor and all openings shall be at least 0.5 m above the 1% AEP flood level (Chapter 6);
- (c) basements are not permitted;
- (d) the redevelopment shall cause no net loss of flood storage below the 1% AEP flood level (Clause 7.6.4), and stilt construction may be used for this; and
- (e) Zone 1 continues to apply in full.

Commentary. The 1:1 compensatory storage at equivalent levels is set by this Manual. A simple volume balance is not enough: storage excavated below the normal water level is full before the flood arrives and gives no flood benefit. Storage gained high up does not replace storage lost low down, because the low storage is used in every flood and the high storage only in the largest. Balancing volume within each elevation band avoids both problems. The 0.3 m band, the 0.01 m allowance for crossings, and the use of the encroachment method in Clause 7.6.1 are set by this Manual; the encroachment method is the standard way to delineate a floodway in a one-dimensional hydraulic model. The 0.1 m limit on the rise in the 1% AEP water level is tighter than the 1 ft (about 0.3 m) used by the US National Flood Insurance Program. The kanhar and dorsa lowlands and paddy land of the Chhattisgarh plain, the margins of its city talabs and the floodplains of the Seonath, the Kharun and the Arpa are flat, so a small rise in level spreads flooding over a wide area, and many small encroachments along the same channel add up.

In Chhattisgarh the lines around talabs and wetlands are measured from a high level, not from the dry-season water line: the inherited text of Rule 50(b) of the CGBVN 1984 measures from "the highest water mark" and the "average high flood mark" (Chhattisgarh text [TBC]), the State Wetland Authority measures its 50 m from the highest flood level, and T&CP asks WRD for the high flood point of land beside a nala or river. The difference matters on the plain, where a talab's dry-season pool can be much smaller than its monsoon spread: Raipur's remaining talabs are reported to cover about 69 ha together, and to be heavily silted (as reported), and no authoritative area was found even for Budha Talab, reported at about 30 ha (as reported), so a line drawn from the shrunken summer edge would leave much of a talab's own storage open to building. This Manual gives the same rule a level for every talab, bandh and lake: its monsoon water line, and its FTL, are part of its boundary (definitions after Clause 7.2), and the foreshore up to MWL, often kanhar paddy land, is its flood storage. Treating all land below MWL as floodway (Clause 7.6.1) means that no new building or fill is approved in a tank foreshore.

The developed-fringe mechanism of Clause 7.6.6 is set by this Manual. Large parts of Raipur, such as the colonies around its talabs, where five ponds overflowed into homes at Chaubey Colony in July 2026, and the low ground along the Kharun at Mahadev Ghat, overtopped in September 2026 (both as reported), of Bilaspur along the Arpa, and of Korba along the Hasdeo below the barrage at Darri, where houses at Motisagarpada, Seetamani and Imliduggu were flooded when the Bango and Darri gates were opened in August 2024 (as reported) already

lie within the 1% AEP extent. A blanket Zone 2 ban on redevelopment there would stop the replacement of older, lower buildings with buildings raised above flood level. The mechanism follows the URDPFI (2015) approach for the 25-year flood zone (stilts, minimum plinth, no basements), applied here to the 1% AEP extent.

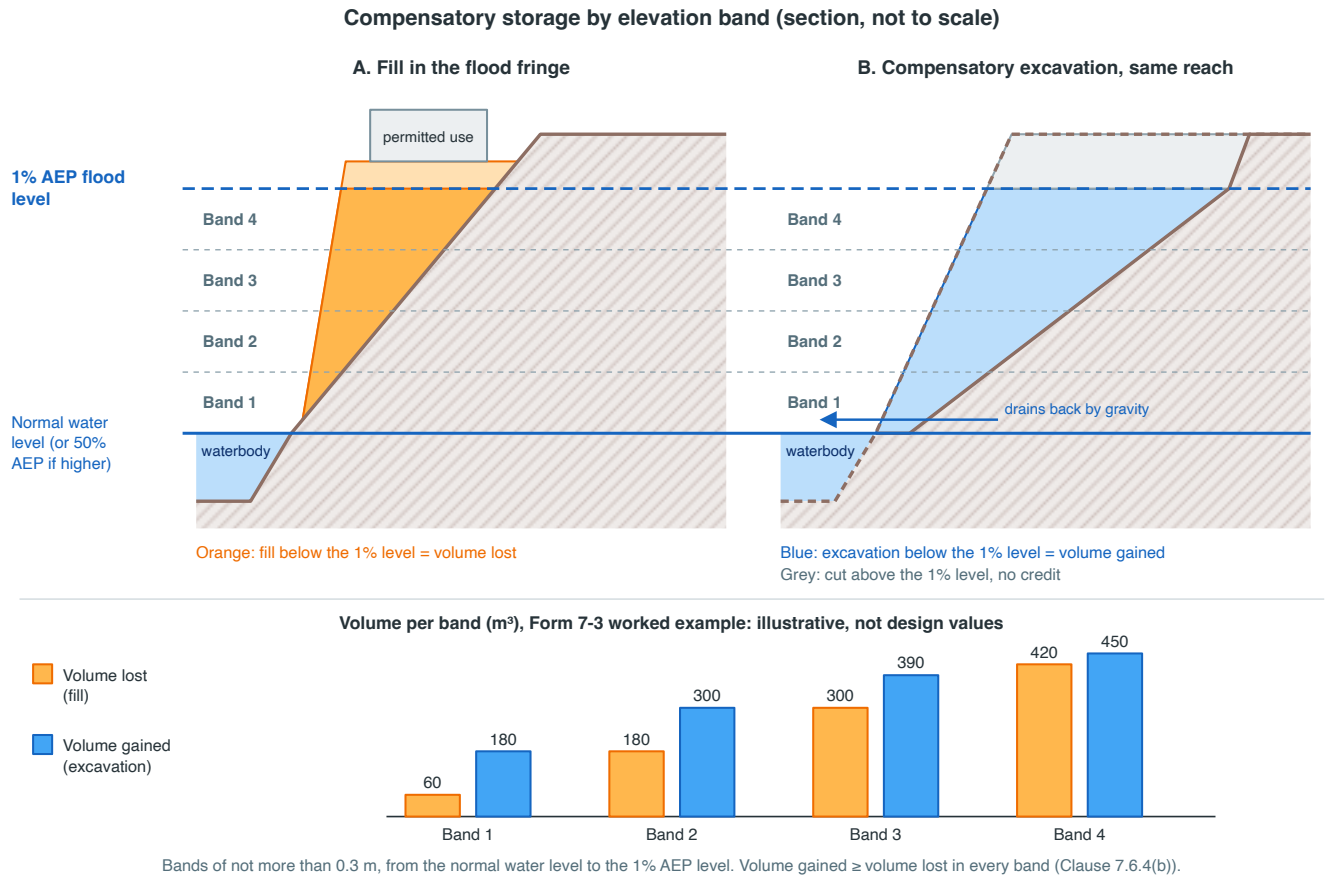

Figure 7.2 — Compensatory storage by elevation band: section through the flood fringe showing fill placed below the 1% AEP flood level and an equal or greater volume excavated within each band of not more than 0.3 m, from the normal water level (or 50% AEP level) up to the 1% AEP level, with the excavation draining back to the waterbody by gravity. Band-by-band calculation on Form 7-3. Original drawing for this Manual, illustrating the method of Clause 7.6.4, which is set by this Manual.

7.7 Tank cascades as design storage

Chhattisgarh has no tank Act, and its storages range from village dabris a few metres across to river reservoirs held by gated dams. The storages that matter for urban drainage are of five kinds. The first is the urban and village talab, bandh, dabri or kund of the plain, usually with no outlet structure, recorded in the Revenue records and vested in the State (Chhattisgarh Land Revenue Code, 1959, s. 251) or, in a town, in the local body. Raipur's are the clearest case: a cluster of talabs, such as Budha Talab (Vivekananda Sarovar, reported at about 30 ha), Telibandha, Kankali, Maharajbandh, Karbala and Narhaiya, linked by colony nalas that drain to the Kharun (all as reported), with a storm-drain project for Raja Talab among the works of the Municipal Corporation's plan of 2015 (AMRUT service-level improvement plan). These are the tanks for which Figure 7.3 and Clause 7.7.2 matter most, and no level, capacity or operating rule for any of them was found. The second is the network of lakes and reservoirs of Nava Raipur Atal Nagar, reported to number 55, among them the Sendh, Jhanjh and Khandawa lakes, into which the development plan makes the parks "storm water carriers" and asks that rainwater be stored "into dug up or natural lakes/ depressions, from the storm water drainage system" (Naya Raipur Development Plan-2031, §10.3.2; the count as reported); they are held by NRDA, and the

Jhanjh wetland among them is treated by the State Wetland Authority as protected (as reported). The third is WRD's reservoirs and barrages, such as Gangrel (Ravishankar Sagar), Minimata Bango, the Hasdeo barrage at Darri, Kharang, Maniyari, the Kelo dam, Mongra and the Arpa barrage at Bhaisajhar, and the smaller tanks near Raipur, such as Kumhari and Pendrawan, whose full reservoir levels WRD reports (tank gauge report of 10 October 2024); their releases enter a design as inflow and tailwater (Chapter 5), and they are not tanks for these clauses unless their storage is counted. Kopra Jalashay near Bilaspur, built by WRD in 1993 and the State's first Ramsar site, overflows to the Ghonga, which joins the Maniyari and the Seonath (Ramsar Information Sheet). The fourth is the dabris and farm ponds of the villages on the urban fringe, modelled as existing features of a catchment but not counted as storage (definitions after Clause 7.2). The fifth is the mine voids, pit lakes and ash ponds of the coalfields and power plants of Korba, Raigarh and Surguja; no inventory of the voids of the Hasdeo-Arand and other coalfields was found, and they are treated under the definitions after Clause 7.2. Clauses 7.7 and 7.8 apply to any of these whose storage is counted; any city talab, Nava Raipur lake, reservoir or pit lake whose storage is to be counted is subject to Clause 7.7.1 in full, with the local body, NRDA, WRD, the lessee or other custodian, and the custodian of each talab and lake shall be confirmed for the inventory (Annex 7A).

7.7.1 **When tank storage may be counted.** The storage of a tank or tank cascade may be counted as design storage (in a basin plan, a regional facility or a fee-in-lieu scheme under Clause 7.11) **only** where all of the following are met:

- (a) **Surveyed levels.** FTL, MWL, TBL, the surplus weir crest and length, and the sluice sill levels have been surveyed to the datum of Clause 7.3.5 within the previous 5 years.
- (b) **Stage–storage curve.** A stage–storage curve has been derived from a bathymetric and topographic survey of the waterspread and foreshore within the previous 5 years, or after the last desilting, whichever is later.
- (c) **Operating rule.** A documented operating rule has been agreed with the custodian (Clause 7.8).
- (d) **Maintained structures.** The surplus weir, sluices and bund are maintained under an O&M plan (Chapter 10), and the latest pre-monsoon inspection found them serviceable.
- (e) **Custodian concurrence.** WRD, or the other custodian, concurs in writing.

Storage in a tank that fails any of (a) to (e) shall not be counted, though the tank shall still be modelled as an existing feature (Chapter 3).

7.7.2 **Modelling.** Tank cascades shall be modelled under Clause 3.12.5 (level-pool routing of each tank, with surplus weir and sluice ratings) and Clause 3.13 (continuous simulation). The whole cascade, from the uppermost tank whose surplus can reach the site or facility to the next river, major drain or sea outfall, shall be modelled together, including, on the Chhattisgarh plain, every talab of a city cluster such as Raipur's that the colony nalas and overflow channels link, with its FTL or spill level, MWL and overflow, the nala it overflows to and the river that nala reaches, such as the Kharun at Raipur, the river level being the tailwater of the nala, and the paddy lowland on the cluster's fringe, which stores flood water and is not to be filled, as shown in Figure 7.3.

7.7.3 **Starting level.** The starting level in every tank shall be **FTL** for the design-storm analysis. A lower starting level may be used only under a flood-cushion operating rule that meets Clause 7.8.

7.7.4 **Design maximum level.** For the 1% AEP inflow with the climate factor, routed through the cascade:

- (a) the computed maximum level shall be not higher than MWL (as defined in Clause 7.2); and
- (b) the freeboard from the computed maximum level to the lowest point of the bund (TBL) shall be not less than WRD's freeboard standard for the tank.

Where (a) or (b) fails, the tank storage above FTL shall not be counted until the surplus weir capacity is increased or the bund raised under Clause 7.7.6.

7.7.5 **Creditable volume.** The creditable design storage of a tank is the volume between the starting level (Clause 7.7.3) and the computed maximum level of Clause 7.7.4, less any volume already committed to another facility or scheme.

7.7.6 **Changes to FTL, weirs and bunds.** No surplus weir crest shall be raised or lowered, no shutters or falling boards shall be added, and no bund shall be raised, cut or breached except under a design approved by the custodian that:

- (a) routes the cascade under Clause 7.7.2;
- (b) shows the effect on flood levels in the foreshore and upstream, and on flows in the surplus course and downstream; and
- (c) checks the bund under Clause 7.14.

> Commentary. This Manual allows tank cascades to be used as design storage subject to three basic conditions (surveyed FTL, a documented operating rule and maintained surplus weirs). Clause 7.7.1 adds the stage–storage survey and the custodian's concurrence, because the storage a tank actually offers depends on its silt level and on who controls the sluices. The 5-year survey interval is set by this Manual.
>
> The FTL starting level follows Clause 3.12.5. Chhattisgarh's worst floods have come when the reservoirs and talabs were already full. On 4 and 5 July 2026 five ponds overflowed into homes at Chaubey Colony in Raipur, with water standing in Adarsh Nagar, DD Nagar, Shankar Nagar and Tatibandh (as reported); in the statewide flood of 24 to 27 September 2026, with all 14 gates of Gangrel open, the Kharun overflowed at Mahadev Ghat, Amleshwar and Khudmuda and about 2,000 people were shifted at Abhanpur (as reported); about 3 October 2025 Gangrel, at 348.68 m and 99.49% full, released about 55,000 cusec through 8 gates and 72 riverside villages were alerted (as reported); and on 24 to 26 August 2024 Minimata Bango released about 30,000 cusec through 6 gates and the Darri barrage about 20,000 cusec, and houses were flooded in Korba (as reported). No record of how long Raipur's talabs or Nava Raipur's lakes stay at or above their spill level in a wet spell was obtained for this Manual. While a talab is full, it stores nothing more below FTL, the colony drains and nalas that flow into it discharge against its spill level, and its overflow adds to the flow in the nala below it, which itself discharges against the river. A tank cascade passes floods from one tank to the next, so a design that counts storage in one tank and ignores the surplus arriving from the tank above it will overstate the benefit. That is why Clause 7.7.2 requires the whole cascade to be modelled.

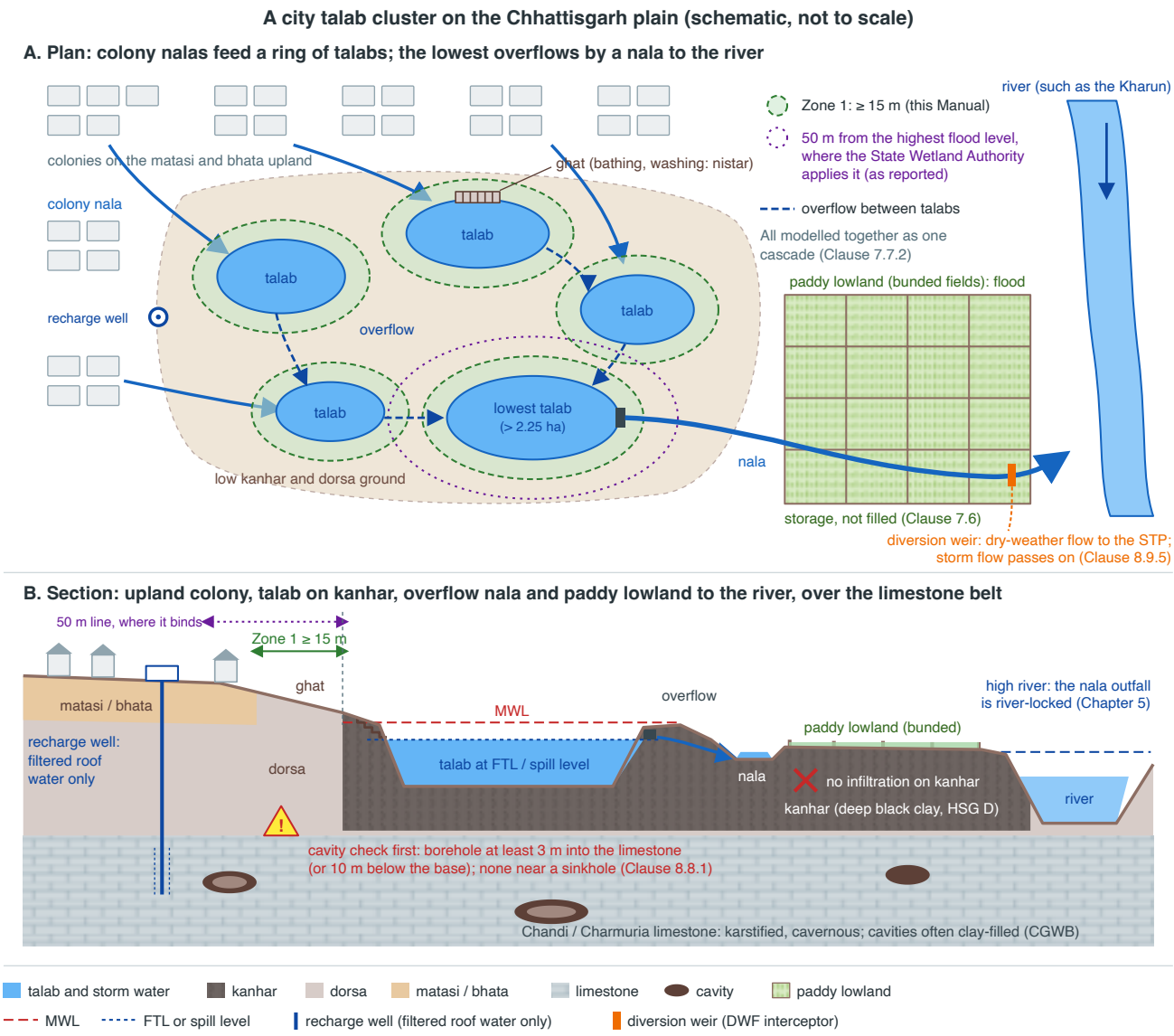

Figure 7.3 — A city talab cluster on the Chhattisgarh plain (schematic, not to scale; Raipur-type, but no particular talab, nala or colony is shown). A: in plan, the colony drains (nali) and colony nalas lead into a ring of talabs on the low kanhar and dorsa ground; the talabs spill one to another through overflow channels, and the lowest overflows to a nala that runs past a paddy-lowland fringe to the river, such as the Kharun at Raipur. The talabs, the nalas and the river are modelled together as a cascade (Clause 7.7.2); each talab's FTL or spill level and monsoon water line are part of its boundary, from which Zone 1 (at least 15 m, this Manual's proposal) and, where it binds, the State Wetland Authority's 50 m line from the highest flood level are measured (definitions after Clause 7.2). The ghats used for bathing and washing (nistar) stay part of the talab. The paddy lowland is flood storage and is not filled (Clause 7.6). B: in section, a full talab stores nothing more below its spill level, its inflow nalas discharge against that level, and the nala below it discharges against the river; while the river is high the nala's outfall is river-locked (Chapter 5). No infiltration facility is placed on kanhar. On the matasi or bhata upland above the limestone of the Raipur–Durg–Rajnandgaon belt, which is karstified and cavernous (CGWB), a recharge well takes filtered roof water only, and only after a cavity check (Clause 8.8.1). The cluster, the 15 m minimum and the cavity check are this Manual's proposals; the inherited Rule 50(b) distances, once confirmed for Chhattisgarh, the State Wetland Authority's line and the WRD high flood point govern where they say more. Original drawing for this Manual.

7.8 Operating rules and pre-monsoon flood cushion

7.8.1 **Operating rule content.** Every tank whose storage is counted under Clause 7.7 shall have a written operating rule, agreed by the custodian and kept with the O&M plan (Chapter 10). It shall state:

- (a) the levels and dates for filling, holding and drawdown through the year;
- (b) the sluice and regulator settings at each level;
- (c) who operates the sluices and regulators, and who may order a release;
- (d) the maximum release rate, set so that the release, added to the flow from the downstream catchment, does not cause flooding of buildings downstream;
- (e) the forecast and warning triggers (IMD district warnings, CGSDMA alerts, and city forecast systems where available) and the actions taken at each trigger;
- (f) communication with downstream ULBs and residents before any release; and
- (g) the records to be kept: daily levels in the SW monsoon, releases, and gate settings.

7.8.2 **Flood cushion.** A tank may be operated with a flood cushion (a level below FTL during part of the SW monsoon), and the cushion counted under Clause 7.7.3, only where:

- (a) the tank has a gated sluice, regulator or other controllable outlet able to draw the tank down to the cushion level in the time stated in the rule;
- (b) the operating rule fixes the cushion level and the dates it applies;
- (c) continuous simulation (Clause 3.13) shows that the cushion is available at the start of a design storm in at least **90%** of SW monsoon seasons in the record, allowing for the rain that usually comes before the peak storm;
- (d) the custodian accepts any loss of irrigation or water-supply yield, and any agreed compensation is recorded; and
- (e) the release needed to create or restore the cushion meets Clause 7.8.1(d).

7.8.3 **No storage credit without control.** Where a tank has no controllable outlet, no flood cushion shall be assumed, whatever the past practice.

7.8.4 **Coordination.** For tanks under WRD control, the operating rule shall be WRD's rule. This Manual does not authorise any other body to operate a WRD tank.

> **Commentary.** No operating rule for a Chhattisgarh talab, city lake or Nava Raipur reservoir that keeps a flood cushion was found, and most talabs have no controllable outlet, so Clause 7.8.3 applies to them. The controlled flows that matter in the towns are the releases of WRD's reservoirs and barrages. WRD reports the full reservoir level, live storage and spillway release of its reservoirs and barrages in a daily tank gauge report (as on 10 October 2024), but no rule fixing the levels, dates and rates at which their gates are opened was found. Gangrel was at 99.49% of its storage, and about 0.02 m below its full reservoir level of 348.70 m, when it released about 55,000 cusec in October 2025, and its 14 gates were all open in the flood of September 2026; Minimata Bango and the Darri barrage released together in August 2024 and flooded houses in Korba (all as reported). Without an agreed rule with dates, levels, release rates and a named operator, storage in a talab or reservoir cannot be relied on, and a release made without warning to the colonies downstream is itself a flood. Clause 7.8.2 turns that principle into a rule that can be checked, with a reliability test, because a cushion that is usually refilled by the early monsoon rain gives no protection in a July to September monsoon depression from the Bay of Bengal. The 90% reliability in Clause 7.8.2(c) has no direct precedent and is set by this Manual.
>
> Most talabs have other users. The vesting of a village tank in the State leaves the rights of irrigation and nistar in it with the village (Chhattisgarh Land Revenue Code, 1959, s. 251), and those uses, such as bathing, washing and watering cattle at a talab's ghats, continue wherever a talab serves them. Drawing a talab down before a storm has a cost to those who use it, which is why Clause 7.8.2(d) requires the custodian to accept it.

7.9 Restoration, desilting and deepening

7.9.1 **Pre-works survey.** Before any desilting or restoration, the proponent shall:

- (a) survey the waterspread, foreshore, bund, weirs and sluices (Clause 7.7.1(a) and (b));
- (b) find the original bed level by test pits or augering on a grid, and from historical records where available; and
- (c) where the tank receives urban runoff or sewage, test the silt for heavy metals and other contaminants before disposal is planned.

7.9.2 **Desilting limit.** Desilting shall remove accumulated sediment down to the original bed level. It shall not deepen the tank below the original bed, except under Clause 7.9.3.

7.9.3 **Deepening.** Deepening below the original bed is permitted only where a hydrogeological report shows that:

- (a) it will not cut through a clay or silt layer that holds water in the tank or protects an aquifer from polluted water;
- (b) in coastal areas, it will not draw saline water into the tank or the aquifer; and
- (c) the extra storage will be refilled and used, and will not stand stagnant below the sluice sill.

Storage created below the lowest sluice sill shall not be counted as flood storage.

7.9.4 **Safe edges.** Where the public has access to the waterbody edge:

- (a) side slopes shall be no steeper than **1V:4H** from 0.5 m above FTL down to 1.2 m below FTL, or a level safety bench at least 3 m wide shall be provided between 0.3 m and 0.5 m below FTL; and
- (b) no isolated deep pit or vertical cut shall be left in the bed.

7.9.5 **Storage not to be reduced.** Restoration shall not reduce the storage below MWL. Islands, walkways, bund widening into the waterspread, ring roads, parking, buildings and "lake-front" platforms within the waterspread or foreshore shall be counted as lost storage and shall be offset under Clause 7.6.4.

7.9.6 **Silt disposal.** Desilted material shall not be placed within the setback, on the bund (except for approved bund strengthening), or on any flood path. Silt found to be contaminated shall be disposed of as the Chhattisgarh Environment Conservation Board (CECB) directs.

7.9.7 **Inlet treatment.** Every restoration of a tank that receives urban runoff shall include sediment and gross pollutant control at each inlet: a sediment forebay, a constructed wetland inlet zone, or a gross pollutant trap (Chapter 8), so that the restored storage does not silt up again.

7.9.8 **Timing.** Desilting shall be completed, and the waterbody made safe, by **31 May** each year (Chapter 10).

> Commentary. No statewide order of the Urban Administration and Development Department for the pre-monsoon desilting of nalas or talabs, and no dated order of the Raipur Municipal Corporation, was found. After the Raipur flooding of July 2026, residents blamed inadequate desilting, and drains were reported narrowed from seven or eight feet to four or five by encroachment (as reported); at Bhilai, the Commissioner of the Municipal Corporation warned the Bhilai Steel Plant in July 2025 that the drains of its township had not been cleaned by the deadline of 15 June, and water hyacinth from them had blocked the culverts at Junwani and Kosanala (as reported). The date of 31 May in this Manual (25 May in the Bastar division) is set by this Manual, before the monsoon's normal onset in the State (about 11 to 13 June at Jagdalpur, 16 June at Raipur and 17 June at Bilaspur, IMD new normal dates, 2020; IMD's Meteorological Centre at Raipur uses 10 June). Excavation to "increase capacity" is governed by Clause 7.9.3; on the limestone of the Raipur–Durg–Rajnandgaon belt, which the CGWB describes as karstified and cavernous, deepening a talab can also open its bed to the cavities below (Clause 8.8.1). The 31 May deadline matches the pre-monsoon inspection date of Chapter 10. The limits on deepening are set by this Manual. Deepening tanks to "increase capacity" is common, but it can breach the silt and clay layer that holds water in the tank, it can let polluted urban water reach the aquifer, it can draw salt water toward the coast, and it creates

storage below the sluice sill that never empties and gives no flood benefit. Deep pits with steep sides are also a drowning hazard. The slope and bench values in Clause 7.9.4 are adapted from common pond-safety practice; they have no Chhattisgarh precedent.

7.10 Supply channels, surplus courses, inlets and outlets

7.10.1 **Part of the major system.** Supply channels and surplus courses are part of the major drainage system (Chapter 6). They shall be protected as waterbodies, with Zone 1 and Zone 2 setbacks under Clause 7.4.

7.10.2 **No closure or covering.** No supply channel or surplus course shall be closed, filled, diverted, covered, culverted along its length, or converted into a road or a closed drain, except under a design approved by the custodian that keeps or increases its 1% AEP capacity and its access for maintenance.

7.10.3 **Crossings.** Every road, rail, utility or access crossing of a waterbody, supply channel or surplus course shall:

- (a) pass the 1% AEP flow (with the climate factor), or the surplus of the upstream tank at its MWL, whichever is greater, with the freeboard required by Chapter 9;
- (b) not reduce the waterway width at the crossing below the full channel width as recorded in Revenue records or as surveyed, whichever is greater;
- (c) keep the channel bed level and grade; and
- (d) not raise the 1% AEP water level by more than the limit in Clause 7.6.2.

7.10.4 **Capacity.** The design capacity of a surplus course shall be the greater of the surplus weir discharge at MWL and the 1% AEP flow from its own catchment, combined as the cascade model (Clause 7.7.2) shows.

7.10.5 **Protection at structures.** Outlets of surplus weirs and sluices, and drain outfalls into waterbodies, shall have energy dissipation and scour protection designed under Chapter 9, so that the flow leaving them does not erode the bund, the bank or the channel bed.

7.10.6 **Drain outfalls into waterbodies.** A storm drain outfall into a tank, channel or wetland shall:

- (a) have its invert at or above the normal water level of the receiving waterbody, unless the drain is designed for submerged outfall under Chapter 5;
- (b) be located and aligned so that it does not cut through or weaken a tank bund;
- (c) have a gross pollutant trap and first-flush treatment (Chapter 8); and
- (d) not carry sewage or dry-weather flow (Chapter 8).

Commentary. This Manual protects supply channels and surplus courses as part of the major system. In urban areas they are often the first to be lost. Surplus courses are narrowed by culverts and compound walls, and supply channels are covered and turned into roads, which cuts the tank off from its source or leaves its surplus with nowhere to go. In Chhattisgarh the town nalas vest in the local body, but the split between natural nalas and municipal drains has not been written down. The Municipal Corporation Act, 1956 and the Municipalities Act, 1961, as adapted in Chhattisgarh, were not read in their Chhattisgarh text; in the Madhya Pradesh text of the 1956 Act, public streams, drains and watercourses vest in the Corporation (s. 82), the Commissioner may "enlarge, arch over, discontinue or close up" a drain, with a substitute for lawful users (s. 193), and a building put up over a drain vested in the Corporation without permission may be pulled down (s. 198) [TBC]. No rule or order dividing natural nalas from municipal drains, or the local bodies' drains from WRD's rivers and canals, was found for this Manual; Chapter 6 gives this Manual's working assignment, under which the rivers, dams, reservoirs, barrages and canals lie with WRD, the nalas and drains within a town with its local body, the streams and drains of Nava Raipur Atal Nagar with NRDA, the drains of the Bhilai Steel Plant's township with the plant, the drains of a scheme with its builder until they are handed over, and the mine and ash-pond drains with the lessee or operator. The planning law

knows the nala but gives it no width: the T&CP site report records "नाला क्षेत्र में छोड़ा गया क्षेत्र", the area left for nalas (2011), the site-inspection form asks for the distance of a major nala from the plot (amendment of 6 October 2018, Appendix Q, item 9), and construction on a natural drain may not be regularised (Chhattisgarh Act of 2002 on the regularisation of unauthorised development, s. 7(1)(iv)). The record shows nalas being lost. Raipur's storm drains covered 49% of the city in 2015, and "Capacity of drain is not sufficient In rainy season most of the drains are overflowed" (Raipur Municipal Corporation, AMRUT service-level improvement plan, 2015); in July 2026 drains in Raipur were reported narrowed from seven or eight feet to four or five by encroachment (as reported); Raipur's domestic sewage is reported to reach the Kharun through eight large nalas (as reported), which are therefore sewers as well as drains (Clause 8.9); and at Bhilai water hyacinth from the township drains blocked the Junwani and Kosanala culverts in 2025 (as reported). The inherited Rule 50 keeps a building off an undrained site, and the 2018 form records the nala distance; neither keeps a nala at its recorded width once it has been piped, narrowed or covered, which is the failure Clauses 7.10.2 and 7.10.3 are written to prevent. Clause 7.10.3(b) prevents a crossing from narrowing a channel to its current, encroached width.

7.11 Fee-in-lieu and regional tank storage

7.11.1 Where a basin plan has been notified, and the Authority has adopted alternative S-5 of the Policy Supplement (Clause 1.7), a development may pay a fee in lieu of on-site detention under Chapter 4. The fee shall be applied to restoring or creating storage in a tank, pond or regional facility in the same basin.

7.11.2 Tank storage funded by fee-in-lieu shall:

- (a) meet Clause 7.7.1 in full before it is counted;
- (b) be downstream of the contributing development and upstream of the flood-prone area it is meant to protect;
- (c) be created, or under construction, before the development it serves is occupied, or within a period the basin plan fixes; and
- (d) be recorded in the register of Clause 10.17 with the developments that funded it, so that the same volume is not sold twice.

7.11.3 Fee-in-lieu does not replace:

- (a) the retention requirement of Chapter 4, which is always met on site; or
- (b) the water quality requirement of Chapter 8, which is met on site or in a regional facility treating the same runoff before it reaches the tank.

Commentary. Chapter 4 provides for fee-in-lieu toward restored talab or tank storage in master-planned basins. Malaysia's 2008 DID checklist allowed on-site detention only for small sites and required community ponds for larger areas (DID, 2008). In Chhattisgarh no shared form of storage is required by the building rules: the rainwater harvesting of the CGBVN 1984 is plot-level (rule 78(4)) or, in an integrated township, township-level (Appendix ण), and the rules have no detention, peak-flow or "zero runoff" rule. The nearest existing form of shared storage is the lake network of Nava Raipur Atal Nagar, into which the development plan asks that rainwater be stored "into dug up or natural lakes/ depressions, from the storm water drainage system" (Naya Raipur Development Plan-2031, §10.3.2), and, in Raipur, the city talabs themselves, with the water-body rejuvenation reported among the components of the urban flood risk management project approved for Raipur in October 2025 (as reported). None of these sets a technical standard for counting storage. Clause 7.11.2(b) matters because a tank downstream of the flood-prone area protects nothing. Clause 7.11.3 keeps retention on site: it is the volume-control layer of this Manual, and a regional tank cannot recharge the aquifer under the developed site.

7.12 Wetlands, marshes and estuaries

7.12.1 **Scope.** This clause applies to marshes, swamps, backwaters, estuaries, and tidal creeks and lagoons, including:

- (a) wetlands notified under the Wetlands (Conservation and Management) Rules, 2017 or listed by the Chhattisgarh State Wetland Authority;
- (b) Kopra Jalashay near Bilaspur, the State's first Ramsar site (Site No. 2583; 210 ha; designated on 8 August 2025 according to the Ramsar Information Sheet, and announced in December 2025), to its Ramsar boundary, and any wetland notified under the 2017 Rules;
- (c) the talabs and wetlands of more than 2.25 ha that the Chhattisgarh State Wetland Authority treats as protected, such as Karbala Talab and Narhaiya Talab at Raipur and the Jhanjh wetland at Nava Raipur Atal Nagar (as reported), the lakes and reservoirs of Nava Raipur Atal Nagar, and the marshy low land and depressions of the river floodplains, such as those of the Mahanadi, the Seonath, the Kharun, the Arpa and the Indravati, within the 1% AEP extent of Clause 7.3.4 (the river channel itself is a waterbody under the rest of this chapter); and
- (d) any other marsh, wetland, talab, lake or pit lake that the Authority notifies, that the wetland authority lists, or that the High Court of Chhattisgarh or the National Green Tribunal directs to be protected.

7.12.2 **No drainage of wetlands.** No drain, channel, cut or pump shall be designed to drain a wetland, to lower its normal water level, or to reduce the period for which it is wet, except under a plan approved by the wetland authority.

7.12.3 **Inflows.** Every drain discharging into a wetland in Clause 7.12.1 shall:

- (a) meet Clause 7.10.6;
- (b) pass through water quality treatment for the WQv of its catchment (Chapter 8) before entering the wetland; and
- (c) where practicable, spread its flow across a vegetated margin rather than discharge at one point.

7.12.4 **Hydraulic connectivity.** Roads, embankments and utility corridors across or along a wetland shall have openings that:

- (a) pass the 1% AEP flow without raising the 1% AEP water level by more than the limit in Clause 7.6.2; and
- (b) keep the wetland's normal water exchange, including tidal exchange in estuaries, with openings spread along the crossing so that no part of the wetland is cut off.

7.12.5 **Storage counted at basin scale only.** Wetland storage shall be included in basin and regional models. It shall not be counted as detention for any individual development.

7.12.6 **Tidal wetlands and estuaries.** For estuaries and tidal wetlands:

- (a) tailwater shall be taken from Chapter 5 (Clause 2.12 joint cases);
- (b) works shall also comply with the Coastal Regulation Zone Notification in force; and
- (c) mouth-opening works, training walls and dredging are outside the scope of this Manual and need their own studies.

7.12.7 **No dumping.** No solid waste, construction debris or dredged material shall be placed in a wetland or its setback.

> **Commentary.** Chhattisgarh's wetlands are the concern of the Chhattisgarh State Wetland Authority, reconstituted by an order of the General Administration Department of 17 December 2021 according to its report to the National Green Tribunal in O.A. 351/2019 (another source gives its establishment as 1 July 2020), with the Forest and Climate Change Department as its nodal department and the State Biodiversity Board as its secretariat (as seen in an extract of that report); in each district the Collector chairs a District Wetland Conservation Committee (as reported). No wetland notified under rule 7 of the 2017 Rules and no published inventory of wetlands was found; the Authority treats talabs and wetlands of more than 2.25 ha as "deemed" wetlands under the Supreme Court's rulings (as reported). It is the working regulator of Raipur's talabs: it halted works at Karbala Talab on 30

September 2025, ordered an inquiry over a plan for a toy train in the buffer of Narhaiya Talab (2025), and directed NRDA on 3 March 2025 to withdraw two tenders next to the Jhanjh wetland at Nava Raipur Atal Nagar; a Forest Division inquiry report of July 2023 on Karbala Talab is reported never to have reached the Authority, with similar findings on Budha Talab, Telibandha and Maharajbandh, and its directive of 8 May 2023 to survey every water body in Raipur district is reported to have been carried out only at Karbala (all as reported). Kopra Jalashay, a reservoir of 210 ha in Takhatpur block of Bilaspur district, about 10 km west of Bilaspur, built in 1993 and managed by WRD, is the State's first Ramsar site (Site No. 2583): its designation date is 8 August 2025 in the Ramsar Information Sheet, and it was announced on 12 December 2025 (as reported); the Sheet lists agricultural runoff and the growing urbanisation and development of the surrounding area among its threats, and the State is reported to aim at 20 Ramsar sites by 2030 (as reported). A drain outfall, desilting, diversion or gate operation in or near Kopra Jalashay, a notified wetland or a talab that the Authority treats as protected therefore needs the Authority's permission or views where the Rules or its directions reach it, as well as compliance with this chapter; for the talabs and lakes it does not reach, this chapter and the revenue and municipal law are the only controls. Clause 7.12.2 targets a specific risk: drains built to relieve flooding next to a marsh can drain the marsh itself, and once it is dry it is encroached. Sewage reaches Chhattisgarh's talabs and rivers through the nalas (Clause 8.9, Commentary). Budha Talab in Raipur is reported to be polluted by "several drain lines", Raipur's remaining talabs are reported to be "polluted and heavily silted", and the city's domestic sewage is reported to reach the Kharun through eight large nalas (all as reported); in January 2021 the CECB told the National Green Tribunal that only Bilaspur had a sewerage system; the High Court heard in January 2025 that about 40% of Bilaspur's sewage would still reach the Arpa untreated (as reported); and the National Green Tribunal ordered in January 2026 that the treatment plants on the Shankar and Pulgaon nalas, which carry most of Durg's sewage to the Seonath, be finished within six months (as reported). Clauses 7.10.6(d) and 7.12.3 therefore cannot yet be met by many existing outfalls. In Chhattisgarh they apply in full to every new outfall, and every existing outfall to a talab, lake, wetland or river that carries dry-weather flow comes under the annual survey and the interception of Clauses 8.9.4 and 8.9.5 until its sources are removed.

7.13 Encroachment interfaces

7.13.1 **Boundary.** For design under this Manual, the boundary of a waterbody is the larger of its Revenue record boundary and its surveyed FTL (or top of bank), as in Clause 7.3.3. Setbacks are measured from that boundary.

7.13.2 **No reliance on encroached land.** No design shall:

- (a) count, as storage or conveyance, land within a waterbody boundary that is occupied by encroachment, unless its removal is ordered and funded; or
- (b) treat the encroached width of a channel as its design width.

7.13.3 **Public works in encroached reaches.** Where a public drainage work passes through an encroached reach:

- (a) it shall be designed for the full waterbody width (Clause 7.13.1);
- (b) where the full width cannot be cleared before the work is built, the design report shall state the interim capacity and the flood risk until it is cleared; and
- (c) the work shall be built so that the full width can be restored later without rebuilding it.

7.13.4 **Boundary marking.** Every development adjoining a waterbody shall, before the completion certificate:

- (a) mark the outer edge of Zone 1 on the ground with permanent survey markers at intervals of not more than 30 m and at every change of direction; and
- (b) show the setback zones on the approved layout plan and in the sale documents of every plot that includes setback land.

7.13.5 **Retaining walls.** No retaining wall, bund or embankment shall be built along the edge of a waterbody, within Zone 1, to protect or reclaim land, except to protect existing public infrastructure under the custodian's approval.

> **Commentary.** Chhattisgarh's main controls on filling and building in tanks are revenue law, the planning law and the courts. Village tanks vest in the State, with the rights of irrigation and nistar preserved (Chhattisgarh Land Revenue Code, 1959, s. 251), and a person who occupies land without authority is penalised and may be evicted summarily by the Tehsildar (s. 248). The route works when a court or tribunal drives it: in Nagar Panchayat Baloda (Janjgir-Champa district), on land "recorded as a pond, a water body", among them the Babu, Noni and Ramsagar ponds and Andhari Talab, the National Green Tribunal (Central Zone) directed the Collector on 14 November 2024 "to ensure the removal of encroachments within the reasonable time", and the Tahsildar reported that "legal proceedings have been initiated under Section 248 of the Chhattisgarh Land Revenue Code, 1959, against individuals who have unlawfully encroached upon ponds" (O.A. 78/2024, report of 27 December 2024). Construction on the bed or edge of a reservoir or tank, or on a natural drain, may not be regularised (Chhattisgarh Act of 2002 on the regularisation of unauthorised development, s. 7(1)(iv)); whether the later regularisation Acts and rules of 2016 and 2022 keep this bar was not checked. In the towns, public tanks vest in the local body under the Municipal Acts as adapted (Chhattisgarh texts not read [TBC]). The losses are reported, not measured: Raipur is reported to have had about 130 ponds, of which half had gone by 2001 and about 37 to 40 remain (as reported; the counts conflict, and a figure of about 200 is also reported); a petition on the encroachment of Madho Talab at Tifra in Bilaspur was before the High Court (W.P.(PIL) No. 135 of 2017, as reported); and in April 2026 the High Court dismissed a petition on the pollution of a Raipur pond with costs of ₹50,000 as privately motivated (as reported). The Chhattisgarh State Wetland Authority has stopped works within 50 m of the highest flood level at Karbala Talab and next to the Jhanjh wetland (2025, as reported), but its survey of Raipur district's water bodies was not carried out (as reported). The Municipalities Act, 1961, which governs the smaller towns, and the other encroachment sections of the Land Revenue Code were not read for this Manual. This Manual cannot evict anyone. It can stop designs from treating encroachment as permanent (Clause 7.13.2), make public works restorable to full width (Clause 7.13.3), and make the setback visible on the ground and in property documents so that new buyers know it is there (Clause 7.13.4). The 30 m marker interval is set by this Manual.

7.14 Safety of tank bunds and embankments

7.14.1 **Dam Safety Act screening.** For every tank, and every embankment built or altered under this Manual, the designer shall determine whether it is a "specified dam" under the Dam Safety Act, 2021. Under the Act, a specified dam is a dam:

- (a) more than 15 m high, measured from the lowest point of the general foundation area to the top of the dam; or
- (b) between 10 m and 15 m high, where the crest length is not less than 500 m, or the reservoir capacity is not less than one million m³, or the maximum flood discharge dealt with by the dam is not less than 2,000 m³/s, or the dam has specially difficult foundation problems, or the dam is of unusual design.

Where it is a specified dam, the Act and the requirements of the State Dam Safety Organisation govern, in addition to this Manual.

7.14.2 **Hazard class.** Every tank bund and detention embankment that is not a specified dam shall be given a hazard class:

- **High hazard:** a breach could flood a habitable building, a school, a hospital, an arterial road or critical infrastructure.
- **Low hazard:** any other case.

7.14.3 **High-hazard embankments.** For high-hazard tank bunds and embankments:

- (a) the surplus weir or spillway shall pass the 1% AEP inflow at MWL with the freeboard of Clause 7.7.4(b), and shall be checked for the 0.2% AEP (500-yr) inflow without overtopping the bund (as for detention basins in Chapter 4);
- (b) a breach analysis shall map the area flooded by a breach at MWL;
- (c) an emergency action plan shall be prepared, with the warning and evacuation arrangements for the breach area; and
- (d) the design shall be certified and independently checked under Clause 10.6.

7.14.4 **Inspection.** Every tank whose storage is counted under Clause 7.7, and every high-hazard embankment, shall be inspected before the SW monsoon (by 31 May) and after every heavy-rainfall event (Chapter 10) for seepage, slips, cracks, animal burrows, trees on the bund, and blockage of the weir and sluices.

7.14.5 **No unauthorised cutting.** No person shall cut, lower or breach a tank bund or surplus weir during a flood except under the custodian's emergency procedure.

> Commentary. The definition of a specified dam is quoted from the Dam Safety Act, 2021 (Act 41 of 2021). The Act defines a "dam" as an artificial barrier constructed across rivers or tributaries to impound or divert water. It excludes flood embankments, dikes and guide bunds. The banks of most talabs and dabris are low and are not expected to make them specified dams, but each bund shall be screened under Clause 7.14.1. The bunds of the larger city talabs and bandhs, such as Budha Talab, Telibandha and Maharajbandh at Raipur, and the dams of the Nava Raipur lakes, are of heights that were not found for this Manual; each shall be screened. Where the storage of a talab, bandh, lake or pit lake is counted under Clause 7.7, its bund, dam or retaining bench is a tank bund for Clause 7.14.2 unless the Act applies; the highwall and benches of a pit lake are checked for stability as well (definitions after Clause 7.2). An ash dyke is never counted as storage, but where it lies upstream of a drain, nala or colony that a design relies on, its breach path is mapped as a hazard (definitions after Clause 7.2; Chapter 5). Whether a tank bund that is not across a river or tributary is a "dam" under the Act is not clear from its text. In Chhattisgarh the embankments that matter to the towns are the dams and barrages of WRD's reservoirs and the ash dykes of the power plants. In September 2026 all 14 gates of Gangrel were open in a statewide flood in which eight people died and about 16,000 were rescued (as reported); in October 2025 Gangrel released about 55,000 cusec and 72 riverside villages were alerted (as reported); and in August 2024 the releases of Minimata Bango and the Darri barrage flooded houses in Korba (as reported). The ash dykes have failed: the NTPC ash dyke at Dhanras near Korba is reported to have breached in July 2024, damaging about 16 ha of paddy; the dyke of CSPGCL's Korba West plant near Jhabu and Nawagaon breached on 19 April 2026, with one death; and slurry from BALCO's ash dam is reported to have reached the Belgari nala and the Hasdeo (all as reported; to be verified with the CECB). No breach map or emergency action plan for a talab bund, a city reservoir or an ash dyke was found, and no State disaster management plan for urban floods was found. A failed or overtopped bund floods the colonies below it, a breached ash dyke sends slurry down the nalas, and an unannounced release floods the colonies too, which is why Clause 7.14.3 asks for breach mapping and an emergency action plan for high-hazard embankments. The hazard classes, the breach analysis and the emergency action plan for high-hazard tanks are set by this Manual. They extend the Chapter 4 requirement for detention basins (1% AEP spillway with outlet blocked, 0.2% AEP for high-hazard basins) to tanks that are counted as design storage.

7.15 Submittal requirements

Every drainage report for a development or work under Clause 7.1.2 shall include:

1. the **Waterbody Sheet (Form 7-1)** under Clause 7.3.2, with the setback zones and their basis (mapped extent, hydraulic study or fixed width);

2. the hydraulic study of the 1% AEP extent and floodway, where Clause 7.3.4 requires it;
3. the **Compensatory Storage Calculation (Form 7-3)**, by elevation band, where fill is proposed in the flood fringe;
4. for any tank storage counted: the **Tank Storage Credit Sheet (Form 7-2)**, with the survey, stage–storage curve, operating rule, cascade model results and custodian concurrence of Clause 7.7.1;
5. for any crossing, outfall or work in Zone 1: the custodian's NOC and the hydraulic analysis of Clause 7.6.2 or 7.10.3;
6. for restoration or desilting: the pre-works survey, silt tests and hydrogeological report of Clause 7.9; and
7. for any embankment: the Dam Safety Act screening and hazard class of Clause 7.14.

7.16 Review and update

7.16.1 This chapter shall be reviewed with the rest of the Manual (Chapter 1). The fixed-width table of Clause 7.4.6 shall also be reviewed within 2 years of the Authority adopting 1% AEP flood maps for its area.

7.16.2 Where the Authority adopts new 1% AEP flood maps, they apply to new submittals **6 months** after adoption, unless the Authority notifies otherwise.

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Annexes and forms

Annex / Form	Content
7A	Statewide waterbody GIS layer and inventory fields
7B	Fixed-width setback table (alternative S-3, with Annex 1C)
7C	Template tank operating rule, with flood cushion
7D	Desilting and restoration specification
7E	Glossary of Hindi and Chhattisgarhi waterbody and soil terms and regional variants
Form 7-1	Waterbody Sheet
Form 7-2	Tank Storage Credit Sheet
Form 7-3	Compensatory Storage Calculation (by elevation band)

CHAPTER 8

Water Quality and Recharge

8.1 Purpose and scope

8.1.1 This chapter sets the requirements for the quality of stormwater leaving a site or a public drainage system, and for groundwater recharge. It covers:

- (a) the water quality volume (WQv) and the water quality design flow;
- (b) the treatment train, and the selection and design of best management practices (BMPs);
- (c) the criteria for infiltration and recharge;
- (d) the exclusion of sewage and dry-weather flow from storm drains;
- (e) gross pollutant traps, first-flush treatment and screens at outfalls to waterbodies;
- (f) how rainwater harvesting (RWH) fits with retention and detention;
- (g) erosion and sediment control during construction; and
- (h) monitoring.

8.1.2 This chapter applies to every development and public work to which Chapter 4 applies (Chapter 1). Clauses 8.9 (sewage) and 8.12 (construction erosion and sediment control) apply to every development that needs a building permit or layout approval, and to every public drainage work, whatever its size.

8.1.3 Retention under Chapter 4 is the first and preferred form of water quality control. Runoff retained on site is treated in full. The rest of this chapter applies to the part of the WQv that is not retained.

> **Commentary.** CPHEEO (2019) is the only Indian manual that covers stormwater quality in any detail. Its Chapter 9 reproduces US event mean concentrations, but cautions that "in an Indian context the values would be much higher" because drains receive night soil, urine and food waste, and it sets no Indian numeric criteria (CPHEEO 2019, Part A §9.5). Its Chapter 10 describes water-sensitive urban design, LID and SuDS treatment trains (CPHEEO 2019, Part A Ch. 10). This chapter turns that guidance into requirements. It follows the treatment approach of the MARC/APWA BMP Manual (2012), now merged into APWA 5600 (2025.01), with the Malaysian 3-month ARI water quality storm (MSMA 2, 2012) and the Georgia water quality volume (GSMM, 2016) as tropical and US benchmarks.

8.2 Definitions

Term	Meaning in this Manual
Water quality volume (WQv)	The runoff volume that must be retained or treated, from the water quality rainfall depth (Clause 8.4).
Water quality rainfall depth (P_wq)	The greater of the retention depth (Annex 2E, Clause 2.10) and the 24-hour 3-month ARI depth (Annex 8A).

Term	Meaning in this Manual
3-month ARI depth	The 24-hour rainfall depth equalled or exceeded on average four times a year, from a partial-duration series of the long-term daily record.
Water quality design flow (Q_wq)	The peak flow from the 3-month ARI storm, used to size flow-through devices and diversion structures (Clause 8.4.5).
Runoff coefficient for WQv (Rv)	The volumetric runoff coefficient for small storms, Rv = 0.05 + 0.009·I.
BMP	A best management practice: a structural or non-structural measure that reduces pollutant loads or runoff volume.
Treatment train	Two or more BMPs in series, from source to site to regional scale (Clause 8.5).
Pretreatment	A BMP that removes coarse sediment, litter and oil before runoff reaches a filter or infiltration BMP.
First flush	The early part of a runoff event, which usually carries the highest concentrations of sediment, litter and oil.
Gross pollutant trap (GPT)	A device that captures litter, plastics, vegetation and coarse sediment, typically above about 5 mm.
Dry-weather flow (DWF)	Flow in a storm drain when it has not rained for at least 72 hours. In a separate storm system, DWF is evidence of sewage, sullage, groundwater infiltration or illicit discharge.
Cross-connection	Any pipe, channel or opening that lets sewage or sullage (grey water from kitchens, bathrooms and laundries) enter a storm drain.
Infiltration facility	A facility designed to store runoff below the ground surface, or in a depression, and let it soak into the soil: infiltration trenches and basins, unlined bioretention, permeable pavement with an open base, soakaways, recharge pits and recharge wells.
Seasonal high groundwater table	As defined in Clause 3.10.2: the highest level in the SW monsoon season.
Hotspot	A land use with a high risk of spills or contaminated runoff (Clause 8.8.8).
Level of Service (LoS) / Value Rating (VR)	The scoring system of Clause 8.6, from the MARC/APWA BMP Manual (2012).

8.3 Objectives and pollutants of concern

8.3.1 Stormwater treatment under this Manual shall aim to:

- (a) remove litter, plastics and other gross pollutants before they reach a waterbody;
- (b) remove suspended sediment, and the metals, nutrients and oil carried on it;
- (c) keep sewage and sullage out of storm drains and waterbodies;
- (d) recharge groundwater where soils and groundwater depth allow, without polluting it; and
- (e) prevent erosion and sediment discharge during construction.

8.3.2 The design shall address the following pollutants for every site, and the hotspot pollutants of Clause 8.8.8 where they apply:

Table 8.1 — Pollutants of concern, sources and primary controls

Pollutant	Main sources in Chhattisgarh towns	Primary control
Gross pollutants (plastics, litter, vegetation)	Streets, markets, open dumping in drains	Source collection; GPTs and screens (Clause 8.10)

Pollutant	Main sources in Chhattisgarh towns	Primary control
Suspended sediment	Construction sites, unpaved roads and verges, road wear, drain silt	Erosion and sediment control (Clause 8.12); pretreatment; filtration
Oil, grease and fuel	Roads, parking, fuel stations, workshops, bus depots	Oil-grit separators at hotspots; bioretention
Pathogens and organic load (BOD)	Sewage and sullage cross-connections, animal waste	Exclusion and DWF interception (Clause 8.9)
Nutrients (N, P)	Sewage, fertiliser, food waste, detergents	Exclusion; bioretention; wetlands
Heavy metals	Roads, roofs, industrial yards	Filtration; sedimentation

> **Commentary.** Gross pollutants and sewage come first in Clause 8.3 because they dominate the pollution of urban waterbodies in Chhattisgarh, and because no treatment BMP works for long if they are not removed. CPHEEO (2019) notes that Indian drains carry night soil, urine and food waste (Part A §9.5). In its WSUD procedure it lists TSS, total nitrogen, total phosphorus and gross pollutants as the key pollutants (Part A Ch. 10). AMRUT 2.0 funds the harvesting of rainwater "through storm water drains into water body (which is not receiving sewage/effluent)" (Lok Sabha unstarred question AU1419), which only works if sewage is first kept out of the drains.

8.4 Water quality volume and design flow

8.4.1 **Water quality rainfall depth.** The water quality rainfall depth is:

$$P_{wq} = \max(P_{ret}, P_{3m})$$

where:

- P_{ret} = the retention depth for the site's zone (Annex 2E, Clause 2.10), mm; and
- P_{3m} = the 24-hour 3-month ARI depth for the site's zone (Annex 8A), mm.

8.4.2 **Water quality volume.** The WQv shall be computed as:

$$WQv = (P_{wq} / 1000) \times Rv \times A$$

where:

- WQv is in m³;
- A = the contributing area, m²;
- $Rv = 0.05 + 0.009 \cdot I$; and
- I = the impervious percentage of the contributing area (0–100), after the connectivity rules of Clause 3.9.

The Rv form above may be used only for sites under 0.5 ha and under the Tier B simplified method (Clause 4.14). This is the same limit as for the retention volume (Clause 4.4.2). For all other sites the WQv shall be computed with the SCS-CN method, as the retention volume is: WQv (m³) = 10 × Q_{wq} × A, where Q_{wq} (mm) is the SCS-CN runoff from P_{wq} for each sub-area and A is in ha.

8.4.3 **Climate factor.** The climate factor of Clause 2.8 shall not be applied to P_wq. P_ret and P_3m are frequency statistics of ordinary rainfall, not AEP design depths.

8.4.4 **WQv met by retention.** The volume retained on site under Chapter 4 counts toward the WQv in full. Where the retained volume is equal to or greater than the WQv, no further treatment is required for that area, except:

- (a) pretreatment of inflows to each retention facility (Clause 8.5.3);
- (b) hotspot controls (Clause 8.8.8); and
- (c) the outfall requirements of Clause 8.10, where the site discharges to a waterbody.

8.4.5 **Water quality design flow.** Flow-through devices (swales, filter strips, oil-grit separators, GPTs) and diversion weirs for off-line BMPs shall be sized for Q_wq. Q_wq shall be computed from the 3-month ARI intensity at the time of concentration of the contributing area:

- (a) by the Rational Method, where Chapter 3 allows it (a single element draining no more than 16 ha); or
- (b) as the peak of a hydrograph method using the 3-month ARI 24-hour depth with the alternating-block pattern of Clause 2.9.1.

8.4.6 **Bypass.** Flows above Q_wq shall bypass flow-through and off-line treatment devices without washing out the material already captured. The bypass shall be part of the minor or major system (Chapter 6).

8.4.7 **Drawdown.** Where the WQv is held in an extended-detention BMP (wet pond, constructed wetland or dry extended-detention basin), it shall be released over **24–48 hours** (Chapter 4).

> Commentary. The WQv depth is the runoff from the 3-month ARI storm or the retention depth, whichever is greater. The 3-month ARI is the Malaysian water quality design storm (MSMA 2, 2012). The Malaysian 2008 checklist gave its depth as about 40 mm and said it captures 90% or more of 24-hour storm runoff volume (DID, 2008). The Chhattisgarh value must be computed (Annex 8A). Georgia sets its WQv as the runoff from 1.2 inches (about 30 mm), the depth that covers 85% of storms in an average year, and requires 80% removal of the average annual post-development TSS load (GSMM 2016, Vol 1, Standard #4). Kansas City's water quality storm is the 1.37-inch (about 35 mm) event that produces up to 90% of the runoff from all 24-hour storms in a year (MARC BMP Manual, 2012).
>
> The Rv formula is from the MARC/APWA BMP Manual (2012), which computes WQv = P × Rv with Rv = 0.05 + 0.009·I. Rv is used instead of the SCS curve number because the CN method greatly understates runoff from small storms on pervious ground. Not applying the climate factor (Clause 8.4.3) has no direct precedent; this Manual does so because it treats P_wq like the retention depth, as a statistic of ordinary rainfall. Bypass of larger flows and the 24–48 hour drawdown follow Chapter 4; MARC (2012) required at least 40 hours of extended detention of the WQv.
>
> **Illustrative — not design values.** For a 1.0 ha site with 80% impervious cover and an assumed P_wq of 50 mm:
>
> - Rv = 0.05 + 0.009 × 80 = 0.77
> - WQv = (50 / 1000) × 0.77 × 10,000 = 385 m³

8.5 Treatment train and pretreatment

8.5.1 **Treatment train.** Water quality control shall be designed as a treatment train, in the order shown in Figure 8.1:

- (a) **source controls** on the plot: roof runoff to RWH, rain gardens, permeable paving, disconnection of impervious areas, and silt traps at inlets;

- (b) **site controls**: bioretention, swales, filter strips, infiltration, oil-grit separators at hotspots, wet ponds and extended detention; and
- (c) **regional controls**, for public drainage: GPTs and DWF interceptors at outfalls, constructed wetlands at tank inlets, and regional ponds.

8.5.2 A development shall meet the WQv on site, through (a) and (b). Regional controls under (c) may count toward a development's WQv only where a regional facility treating the same runoff has been built under a notified basin plan (Clause 7.11).

8.5.3 **Pretreatment.** Every bioretention cell, infiltration facility, permeable pavement receiving run-on, wet pond and constructed wetland shall have pretreatment at each concentrated inflow. Pretreatment shall be a sediment forebay, grass filter strip, silt trap or proprietary device that holds at least **10% of the WQv** of its contributing area, or that is sized for Q_wq for flow-through devices.

8.5.4 Pretreatment shall be accessible for cleaning without entering or disturbing the main BMP.

> **Commentary.** The treatment train follows CPHEEO (2019, Part A §10.2.3, SuDS treatment train) and CIRIA C753 *The SuDS Manual* (2015). It is also the order of priorities of this Manual: preserve, retain, detain, convey (Chapter 1). The 10% forebay volume has no Indian precedent; it is a common sizing rule for forebays in US manuals. In Chhattisgarh, unpaved verges, construction sites and drain silt deliver a heavy sediment load. Bioretention and infiltration facilities without pretreatment clog, often within a year or two.

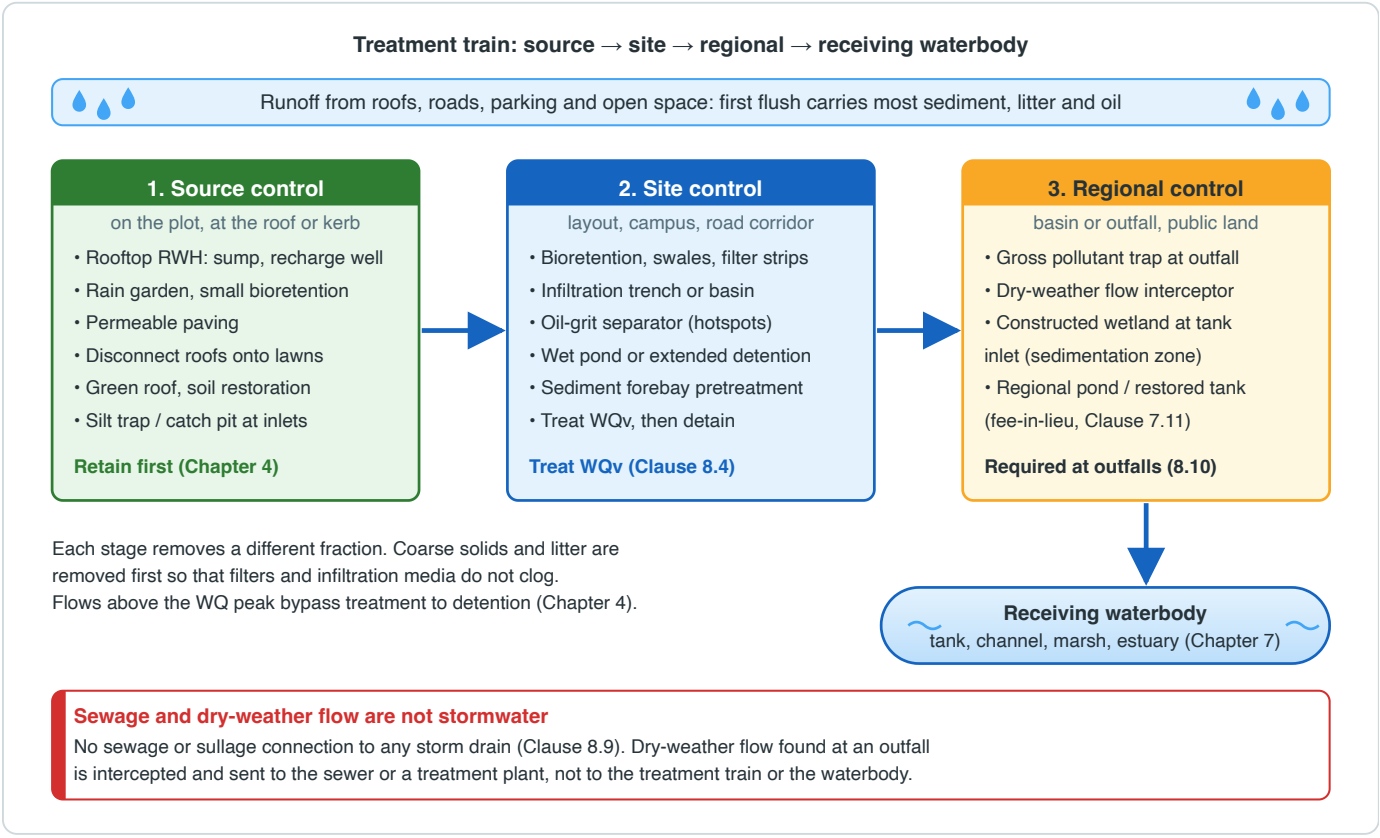


Figure 8.1 — Treatment train: source, site and regional controls before the receiving waterbody, with sewage and dry-weather flow kept out of the system. Original drawing for this Manual, adapted from CIRIA, The SuDS Manual (C753), 2015, and MARC/APWA, Manual of Best Management Practices for Stormwater Quality, 2012.

8.6 BMP selection: Level of Service and Value Rating

8.6.1 **Method.** In Tier A areas, the BMPs for each site shall be selected by the Level of Service (LoS) method of this Clause. In Tier B areas, the simplified route of Clause 8.6.5 may be used.

8.6.2 **Required Level of Service.** For a site that is not redevelopment, the required LoS depends on the change in curve number (ΔCN) from pre- to post-development (Chapter 3):

Table 8.2 — Required Level of Service for sites that are not redevelopment

ΔCN	Impact	Required LoS
17 or more	High	8
7 to 16	Moderate	7
4 to 6	Low	6
1 to 3	Minimal	5

For redevelopment (existing impervious cover of 20% or more of the site), the required LoS shall be taken from Annex 8B.

8.6.3 **Value Rating.** Each BMP treating the WQv has a Value Rating (VR) from Table 8.3. The site's composite VR is the area-weighted average of the VRs of the BMPs serving each part of the site. Treatment trains get a composite VR under Annex 8B. The composite VR shall be equal to or greater than the required LoS.

8.6.4 **Conditions for a Value Rating.** A BMP earns its VR only if:

- (a) it is sized for the WQv (or Q_wq for flow-through devices) of the area credited to it;
- (b) it meets the design requirements of Clause 8.7 and, for infiltration, Clause 8.8; and
- (c) it has pretreatment under Clause 8.5.3.

8.6.5 **Tier B simplified route.** In Tier B areas, the requirement of Clause 8.6.3 is deemed met where the whole WQv of the site is either:

- (a) retained on site under Chapter 4; or
- (b) treated by a bioretention cell, rain garden or infiltration facility designed under Clauses 8.7 and 8.8, with pretreatment.

Table 8.3 — BMP Value Ratings and suitability in Chhattisgarh

BMP	VR (MARC 2012)	Main pollutants removed	Volume reduction / recharge	Matasi uplands, river sands	Laterite / hard rock	Kanhar or dorsa lowland, mine-disturbed ground or ash / high water table	Chhattisgarh notes
Native vegetation (preserved)	9.25	All	High	Suited	Suited	Suited	Counts only for preserved, undisturbed area
Rain garden / infiltration	9.0	Sediment, metals, oil, nutrients	High	Suited if 1.0 m separation met	Conditional (test)	Not suited	Mosquito control: drain within 48 h
Bioretention	8.5	Sediment, metals, oil, nutrients	Medium–high (unlined); low (lined)	Suited	Suited, with underdrain	Lined, with underdrain	Media source and phosphorus content to be specified

BMP	VR (MARC 2012)	Main pollutants removed	Volume reduction / recharge	Matasi uplands, river sands	Laterite / hard rock	Kanhar or dorsa lowland, mine-disturbed ground or ash / high water table	Chhattisgarh notes
Permeable pavement	7.5	Sediment, metals, oil	Medium–high	Suited	Conditional	Lined only	Not where run-on carries heavy silt
Extended detention wetland	7.0	Sediment, nutrients, metals	Low	Conditional (water balance)	Suited	Suited	Good at tank inlets (Clause 7.9.7)
Media / sand filter	6.0	Sediment, metals	Low	Suited	Suited	Suited (lined)	Needs frequent cleaning
Extended wet detention (wet pond)	5.0	Sediment, metals, some nutrients	Low	Conditional (water balance)	Suited	Suited	Dry-season water balance (Clause 8.7.8)
Vegetated filter strip	5.0	Sediment	Low–medium	Suited	Suited	Suited	Mostly as pretreatment
Native vegetation swale	4.0	Sediment, oil	Low–medium	Suited	Suited	Suited	Not where Q_wq velocity is high
Extended dry detention	4.0	Coarse sediment	Low	Suited	Suited	Suited	Combine with detention (Chapter 4)
Proprietary / hydrodynamic devices	3.0–5.0	Coarse sediment, oil, litter	None	Suited	Suited	Suited	VR from verified test data only
Signage (public education)	+0.25	—	—	—	—	—	Added to composite VR

The VR column is from the MARC/APWA BMP Manual (2012), Table 4.4. The suitability columns and notes have no direct precedent and are set by this Manual.

> **Commentary.** The LoS and VR method is from the MARC/APWA Manual of Best Management Practices for Stormwater Quality (2012), Tables 4.2 and 4.4. The VRs combine a water quality score, based on effluent TSS concentrations in the International Stormwater BMP Database, with scores for volume reduction, temperature, and oils and floatables. The method suits Chhattisgarh for two reasons. It rewards volume reduction and preservation, which also serve recharge. It can also be checked by a reviewer without water quality modelling. Its weakness is that the scores come from US data. Chhattisgarh stormwater carries more litter and more sewage than the US runoff behind the scores (CPHEEO 2019, §9.5), so this Manual requires GPTs and DWF exclusion separately (Clauses 8.9 and 8.10) and does not let a VR stand in for them.

8.7 BMP design summaries

8.7.1 **General.** Each BMP shall be designed to the requirements below and to Annex 8D (BMP design sheets). The dimensions given are **recommended starting values** where marked; the designer shall justify any departure. Every BMP shall be designed for safe passage of flows above Q_wq (Clause 8.4.6) and shall have access for maintenance (Chapter 10).

8.7.2 **Bioretention and rain gardens** (Figure 8.2).

- (a) Surface ponding depth not more than **300 mm** above the mulch, with an overflow at the maximum ponding level.
- (b) Surface ponding drained within **48 hours**.
- (c) Filter media 600–900 mm deep: an engineered sandy loam with low fines and low phosphorus content (Annex 8D), with a 50–75 mm mulch layer.
- (d) A transition (choker) layer and a gravel drainage layer below the media.
- (e) A perforated underdrain where the design infiltration rate of the native soil (Clause 8.8.4) is below the rate needed to meet (b), or where the cell is lined.
- (f) Unlined cells shall meet the infiltration criteria of Clause 8.8. Cells that cannot meet them, and cells at hotspots, shall be lined.
- (g) Plants: native or naturalised species tolerant of both waterlogging and long dry spells.

8.7.3 **Vegetated swales.**

- (a) Trapezoidal or parabolic section, with side slopes no steeper than 1V:3H.
- (b) Longitudinal slope 0.5% to 4%, with check dams where steeper.
- (c) Flow depth at Q_{wq} not more than the height of the grass, and velocity at Q_{wq} not more than 0.3 m/s.
- (d) The 1% AEP flow (or the bypass) shall not erode the swale (Chapter 9 permissible velocities).
- (e) Where longitudinal slopes are below 1% or the subsoil is poorly drained, an underdrained ("dry") swale with filter media.

8.7.4 **Vegetated filter strips.**

- (a) Receive sheet flow only, with a level spreader where flow arrives concentrated.
- (b) Length in the direction of flow at least 5 m, and slope 1% to 8%.
- (c) Used mainly as pretreatment for bioretention, infiltration and swales.

8.7.5 **Permeable pavement.**

- (a) Run-on from impervious areas not more than twice the permeable area, and no run-on from unpaved or eroding areas.
- (b) Base reservoir of clean, open-graded aggregate, sized to hold its share of the WQv in its voids.
- (c) An open base (infiltrating) only where Clause 8.8 is met; otherwise lined with an underdrain, and then counted as filtration only.
- (d) Structural design for the traffic loading by the road authority's standards.
- (e) Not at hotspots, and not where the surface will be resurfaced with impermeable material.

8.7.6 **Infiltration trenches and basins** (Figure 8.3).

- (a) Pretreatment under Clause 8.5.3 in every case.
- (b) Trenches filled with clean washed aggregate, with a void ratio confirmed by test, wrapped in geotextile on the sides and top, on a sand base.
- (c) An observation well to the base of each trench.
- (d) Storage volume = WQv (or the retention volume of Chapter 4, where greater) held in the voids; emptied within 72 hours at the design infiltration rate.
- (e) An overflow to the minor or major system at grade.
- (f) Not brought into service until the contributing area is stabilised (Chapter 10).

8.7.7 **Wet ponds (extended wet detention).**

- (a) A permanent pool of at least the WQv, plus extended detention of the WQv above the pool, released over 24–48 hours.
- (b) A sediment forebay at each inlet (Clause 8.5.3).
- (c) Length-to-width ratio of the flow path at least 3:1, with no short-circuiting.

- (d) Permanent pool depth 1.0–2.5 m, with a safety bench (Clause 7.9.4).
- (e) A dry-season water balance (Clause 8.7.8).
- (f) The detention storage of Chapter 4 may sit above the extended detention level.

8.7.8 **Water balance for permanent pools.** Wet ponds and wetlands shall be shown, by a monthly water balance using IMD rainfall and pan evaporation, to keep at least 50% of the permanent pool volume (or the minimum wetland depth) through the driest year in 10. They shall not be topped up with groundwater, potable water or sewage. Where the balance fails, a dry extended-detention basin or a constructed wetland with a smaller permanent pool shall be used instead.

8.7.9 **Constructed wetlands.**

- (a) An inlet sedimentation pond (forebay) followed by a shallow vegetated macrophyte zone.
- (b) Extended detention of the WQv above the normal water level, released over 24–48 hours, with an extended detention depth not more than 0.5 m.
- (c) A flow path length-to-width ratio of at least 3:1, with deep zones across the flow path to spread the flow.
- (d) A high-flow bypass so that floods do not scour the macrophyte zone.
- (e) Where built at a tank inlet, the wetland shall sit within the tank's setback but outside its waterspread, and its storage shall not be counted as tank storage.

8.7.10 **Gross pollutant traps.**

- (a) Sized to treat Q_wq, with all flows up to the 1% AEP passing without washing out captured material.
- (b) Capture of gross pollutants down to about 5 mm.
- (c) Head loss included in the hydraulic grade line of the drain (Chapter 9), with the screen assumed **50% blocked**.
- (d) Storage for captured material sized for the cleaning interval of the O&M plan, and not less than the 3-monthly cleaning of Chapter 10.
- (e) Vehicle access for cleaning by suction or grab.

8.7.11 **Oil-grit separators.**

- (a) Required at the hotspots of Clause 8.8.8 that involve fuel or oil: fuel stations, vehicle workshops and washing bays, bus and truck depots, and parking areas of more than 50 spaces.
- (b) Sized for Q_wq, with a high-flow bypass.
- (c) Not to be used as the only treatment: they shall be followed by a filtering BMP where the site needs a VR above 5.

8.7.12 **Silt traps and catch pits.**

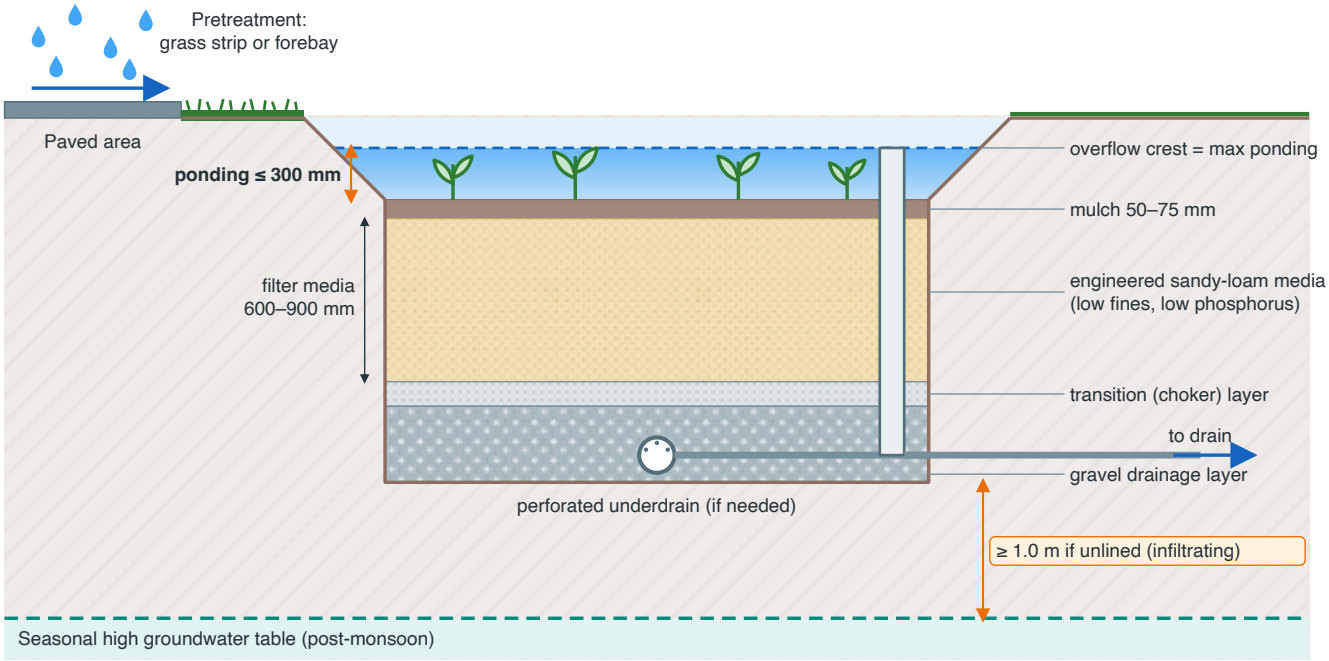
- (a) At every inlet that receives runoff from unpaved ground, and at the inlet of every drain discharging to a BMP or waterbody.
- (b) A sump at least 300 mm below the outlet invert.
- (c) Silt traps are pretreatment only and earn no VR.

> **Commentary.** The design summaries draw on the MARC/APWA BMP Manual (2012), the Georgia Stormwater Management Manual (2016, Vol 2), CIRIA C753 *The SuDS Manual* (2015) and PUB Singapore's ABC Waters Design Guidelines. The dimensions are typical of those manuals but have not been calibrated in Chhattisgarh. Three local issues drive changes from the source manuals:
>
> - **Mosquitoes.** Dengue is endemic in Chhattisgarh. Surface ponding in bioretention and rain gardens must drain within 48 hours (Clause 8.7.2(b)), well inside the time mosquito larvae need to develop. This is consistent with the 24–48 hour WQv drawdown of Clause 8.4.7.
> - **Dry seasons.** Most of Chhattisgarh's rain falls in the south-west monsoon: the State's normal is 1,268.1 mm a year, of which about 90% falls from June to September and July and August alone bring nearly 58%, and the district normals run from about 1,010–1,040 mm in Kabirdham, Bemetara and Baloda Bazar to 1,575.7

mm in Bastar and 1,839.3 mm in Dantewada, with 1,129.3 mm in Raipur, 1,061.5 mm in Durg, 1,110.5 mm in Bilaspur and 1,354.1 mm in Korba (IMD, *Climate of Chhattisgarh*, 2015, normals of 1961–2010). The monsoon now sets in at Jagdalpur on about 11 to 13 June, at Raipur on about 16 June and at Bilaspur on about 17 June, and withdraws from Pendra Road by about 7 October, from Raipur by about 9 October and from Jagdalpur by about 12 to 14 October (IMD, new normal dates, 2020). From October to the monsoon, talabs and dabris fall unless they are fed, and in the towns the feed of a talab is often sewage, because the nalas carry it (Clause 8.9, Commentary). A wet pond or constructed wetland whose water balance (Clause 8.7.8) cannot keep its permanent pool through the dry months on its own catchment's runoff is not to be used; on the bhata and matasi uplands and in the drier west of the plain, the dry forms of Clause 8.7 (bioretention, swales, recharge where Clause 8.8.1 allows it, and extended dry detention) are the practical choice. Clause 8.7.8 applies to BMPs, which shall not be topped up with sewage, with pumped mine water or with ash-pond water; talabs, bandhs, dabris, lakes, reservoirs and pit lakes are waterbodies under Chapter 7 and are not to be used as treatment for a development. Wet ponds that dry out turn into stagnant pools or are used for dumping. Clause 8.7.8 requires a water balance and bans topping up with sewage, which is a common practice for keeping urban ponds "full".

- **Litter and silt.** GPTs and silt traps are treated as essential, not optional, because Chhattisgarh drains carry heavy litter and silt loads. The 50% blockage factor for screens matches the grate clogging factor of Chapter 6.

The 50-space threshold for oil-grit separators, the 5 mm GPT capture size, the 0.3 m/s swale velocity and the other dimensions in this Clause have no direct precedent and are set by this Manual.



Lined version (no infiltration): impermeable liner below the gravel layer. Required at pollution hotspots and wherever the 1.0 m separation to seasonal high groundwater cannot be met.

Dimensions as set in Clause 8.7.2.

Figure 8.2 — Bioretention cell (rain garden) in section: pretreatment, surface ponding, filter media, transition and gravel layers, underdrain, overflow, and the 1.0 m separation to seasonal high groundwater for an unlined cell. Original drawing for this Manual, adapted from MARC/APWA, Manual of Best Management Practices for Stormwater Quality, 2012, and CIRIA, The SuDS Manual (C753), 2015.

8.8 Infiltration and recharge criteria

8.8.1 **Groundwater separation.** An infiltration facility is permitted only where the **seasonal high groundwater table is at least 1.0 m below the base** of the facility. There is no exception in Chhattisgarh, which has no coastal sand areas (Clause 8.8.9). For this Clause the seasonal high groundwater table is the highest post-monsoon level (September to November); in 2023 the post-monsoon water table stood within 5 m of the ground in about 76% of the State's monitoring wells (CGWB). **Recharge first where the aquifer is stressed.** Statewide, groundwater extraction was 47.32% of the extractable resource in the assessment of 2024, but five blocks were critical, among them Dharsiwa in Raipur district at 93.51%, with Gurur, Nawagarh, Bemetara and Berla nearby, and 21 semi-critical, among them Durg, Dhamdha and Rajnandgaon (CGWB). In those blocks, and in the urban areas of Raipur and Durg–Bhilai generally, recharge, with pretreatment, is the first means of meeting retention wherever this Clause is met. **Limestone belt.** The Chandi and Charmuria limestone of the Raipur–Durg–Rajnandgaon belt, which supplies Raipur, is described by the CGWB as karstified and cavernous, with solution cavities along its bedding, often filled with clay, and a field permeability of 4 to 65 m a day; water entering it reaches the aquifer fast, with whatever it carries, and concentrated recharge can wash out the clay in a cavity. In the belt (Raipur, Durg, Bemetara, Baloda Bazar, Rajnandgaon and Khairagarh districts, and wherever a borehole finds limestone), no recharge well, recharge shaft, borewell recharge or infiltration facility deeper than 1.5 m is built without a **cavity check**: a borehole log at the facility, carried at least 3 m into the limestone or to 10 m below the base of the facility, whichever is less, or a geophysical survey interpreted by a hydrogeologist, showing no open or clay-filled cavity within that depth, and no facility of any kind within 30 m of a known sinkhole or collapse; every such facility receives only filtered roof water or runoff that has passed through a filtering BMP (Clause 8.8.10(b)), and no runoff from a fuel station, vehicle workshop, cement plant, quarry, industrial yard or road with heavy traffic is led to recharge in the belt. **No recharge.** In the following, no infiltration facility, recharge pit, recharge well or recharge shaft shall receive runoff of any kind, the exception for roof water in Clause 8.11.3(c) does not apply, and no infiltration or recharge is credited toward retention or the WQv; retention there is met by storage for use, surface measures and detention under Chapter 4: (i) **kanhar**, the deep black clay of the lowlands (HSG D), whatever its depth to water, because it cracks when dry, seals when wet and lies where the water table is shallow; (ii) river floodplains, and the beds and foreshores of talabs, bandhs, dabris, lakes and reservoirs; (iii) dorsa and puddled paddy lowland where the seasonal high groundwater table is within this separation of the base; (iv) mine-disturbed ground (overburden dumps, backfilled or reclaimed mine land, compacted mine benches and haul roads, and subsidence ground), ash ponds, ash dykes and land filled with ash, the dewatering zone of a working mine as the lessee's mining plan or hydrogeological report shows it (around the Gevra and Kusmunda mines of the Korba coalfield the drawdown is reported to reach about 1.2 km), and land within 50 m of the edge of a mine void unless a geotechnical check shows that it is stable; (v) ground where groundwater flows at the surface, as from the flowing wells of the Gondwana areas of Raigarh, Korba, Surguja and Koriya; and (vi) any other site where the seasonal high groundwater table is within this separation of the base. **Quality.** Fluoride above 1.5 mg/L was found in samples from Janjgir-Champa, Kanker, Korba, Koriya, Mahasamund, Raigarh, Rajnandgaon and Surajpur districts, nitrate above 45 mg/L in about 11% of samples across 20 districts, among them Raipur, Durg and Bilaspur, and arsenic in samples from Korba, Koriya and Raigarh (CGWB); arsenic is high in the wells of Kaudikasa in Ambagarh Chowki block, formerly of Rajnandgaon district (CGWB). In the fluoride districts a recharge well is used only after a check of the groundwater quality at the site, and water drawn back from a recharged aquifer is tested before any drinking use; in Ambagarh Chowki block, untreated runoff of any kind, including roof water, shall not be led into a recharge well, borewell, recharge shaft or injection well unless a hydrogeologist's report confirms that the target aquifer is not arsenic-affected and will not be harmed; and no mine water, ash pond water, industrial runoff or sewage-laden runoff is led to any recharge structure. Where the recharge is through a well or shaft into fractured or cavernous rock, a hydrogeologist's report shall confirm the depth to water, the receiving zone, and that no drinking-water well within 30 m will be harmed. These limits, and the cavity check, are set by this Manual. The seasonal high groundwater table shall be determined under Clause 3.10.2.

8.8.2 **Other separation.** The base of an infiltration facility shall also be at least 1.0 m above bedrock or any impermeable layer, unless the facility is designed to recharge a fractured or weathered rock aquifer under Clause 8.8.10.

8.8.3 **Infiltration tests.** The infiltration rate shall be measured at the proposed base level of the facility:

- (a) by a double-ring infiltrometer test (ASTM D3385) for basins, bioretention and permeable pavement;

- (b) by a falling-head borehole or trial-pit soakage test (for example BRE Digest 365) for trenches, soakaways, recharge pits and recharge wells;
- (c) with at least **2 tests per facility**, and at least 1 test per 100 m² of infiltration base area or per 50 m of trench length; and
- (d) by a person competent in geotechnical testing, with the results reported on the Infiltration Test Record (Form 8-2).

Tests made in the dry season shall be reported with the groundwater level at the time of test and the seasonal high level of Clause 8.8.1.

8.8.4 **Design infiltration rate.** The design infiltration rate is the measured rate divided by a safety factor:

- (a) **2.0** where the tests meet Clause 8.8.3(c) and the results vary by no more than a factor of 2; and
- (b) **3.0** otherwise.

8.8.5 **Minimum rates.**

- (a) Full infiltration (no underdrain) only where the design rate is at least **13 mm/h**.
- (b) Partial infiltration, with an underdrain, where the design rate is 3 mm/h to 13 mm/h. Only the volume that infiltrates within 72 hours is credited as retention.
- (c) No infiltration credit where the design rate is below 3 mm/h.

8.8.6 **Setbacks for infiltration facilities.** Infiltration facilities receiving runoff from roads, parking or other paved surfaces shall be at least:

- (a) **3 m** from building foundations, and **5 m** from a basement;
- (b) **1.5 m** from a property boundary, unless the neighbour agrees in writing;
- (c) **30 m** from a drinking-water well or borewell;
- (d) **15 m** from a septic tank or soak pit; and
- (e) **6 m** from the crest of a slope steeper than 1V:3H, unless a geotechnical report shows the slope remains stable.

Roof-only runoff to recharge structures is subject to (a) and (b) only, and to Clause 8.11.3.

8.8.7 **Drawdown and verification.** Every infiltration facility shall empty within **72 hours** at the design rate (Chapter 4), and shall be tested after construction (Clause 10.9.3).

8.8.8 **Hotspots.** Runoff from the following shall not be infiltrated without treatment, and shall not be infiltrated at all where (d) or (e) applies:

- (a) fuel stations;
- (b) vehicle workshops, washing bays, and bus and truck depots;
- (c) industrial yards and loading areas where hazardous materials are handled or stored;
- (d) solid waste transfer stations and landfill areas; and
- (e) land known or suspected to be contaminated.

Treatment for (a) to (c) shall include an oil-grit separator and a lined filtering BMP before any infiltration.

8.8.9 **Sandy coastal aquifers.** This Clause applies only to coastal sand aquifers. Chhattisgarh has none, so the Clause has no effect in the State; it is kept only so that the clause numbers stay unchanged. The sands of the river beds, such as those of the Mahanadi, the Seonath, the Kharun and the Arpa, and the alluvium along the larger rivers are not coastal sand areas: Clause 8.8.1 applies there in full, and the reduced separation of item (d) does not. In coastal sand areas:

- (a) infiltration is the preferred means of meeting retention and the WQv wherever Clause 8.8.1 is met;
- (b) storm drains shall not be designed to lower the groundwater table: inverts below the seasonal high groundwater table shall be watertight, with no weep holes or subsoil drains that discharge groundwater to the outfall, unless a hydrogeological report shows no harm to the freshwater aquifer;

- (c) no long-term pumping of groundwater into storm drains shall be approved (for example basement dewatering); and
- (d) in notified coastal sand areas, **roof-only runoff** may be infiltrated with a separation of not less than **0.5 m** to the seasonal high groundwater table, provided a groundwater mounding check shows that the mound stays at least 0.3 m below the facility base and does not reach adjacent foundations or basements. No other runoff may use this reduced separation;
- (e) where neither the 1.0 m separation nor clause (d) can be met, the site shall use surface retention that does not infiltrate below ground (for example shallow vegetated depressions, disconnection of roofs onto lawns, and reuse), and detention under Chapter 4.

8.8.10 **Laterite and hard-rock areas.** In laterite and hard-rock terrain (for example the granite-gneiss and laterite of the Bastar plateau, as around Jagdalpur and Kanker, and of the northern hills, as around Ambikapur and Jashpur Nagar, the bhata uplands of the Chhattisgarh plain, thin gravelly soil over murram and ferricrete, the Gondwana sandstone and shale of the Korba, Raigarh and Surguja coalfields, and the limestone and shale of the Chhattisgarh plain, as around Raipur, Durg–Bhilai and Rajnandgaon, where the Chandi and Charmuria limestone is karstified and cavernous (Clause 8.8.1), and where groundwater is held mainly in the weathered zone, the fractures and the solution cavities):

- (a) tests shall be made at the facility base, below any surface crust or hardpan;
- (b) recharge into weathered or fractured rock is permitted only for roof runoff, or for runoff that has passed through a filtering BMP, because fractures give little treatment; and
- (c) infiltration on or above slopes shall be checked for slope stability.

8.8.11 **Delta clays and black cotton soils.** In the kanhar and dorsa lowlands and puddled paddy land of the Chhattisgarh plain, the silted beds of talabs, bandhs, dabris and reservoirs, the floodplains of the rivers, ash ponds, and overburden dumps, compacted mine benches and other mine-disturbed ground, and in expansive black cotton soils:

- (a) infiltration facilities will usually fail Clause 8.8.5 and need not be tested where the soil survey (Chapter 3) shows HSG D; and
- (b) where infiltration is used near buildings on expansive soils, the setback of Clause 8.8.6(a) shall be doubled.

> Commentary. The 1.0 m groundwater separation is a core requirement of this Manual. The other values in this Clause (the 1.0 m bedrock separation, test numbers, safety factors, minimum rates and setbacks) are typical of US and UK practice and have no Chhattisgarh precedent. The well setback in Clause 8.8.6(c) applies to runoff from paved surfaces, not to roof water. Chhattisgarh RWH practice routes roof water into open wells and recharge wells, and Clause 8.11 keeps that practice.
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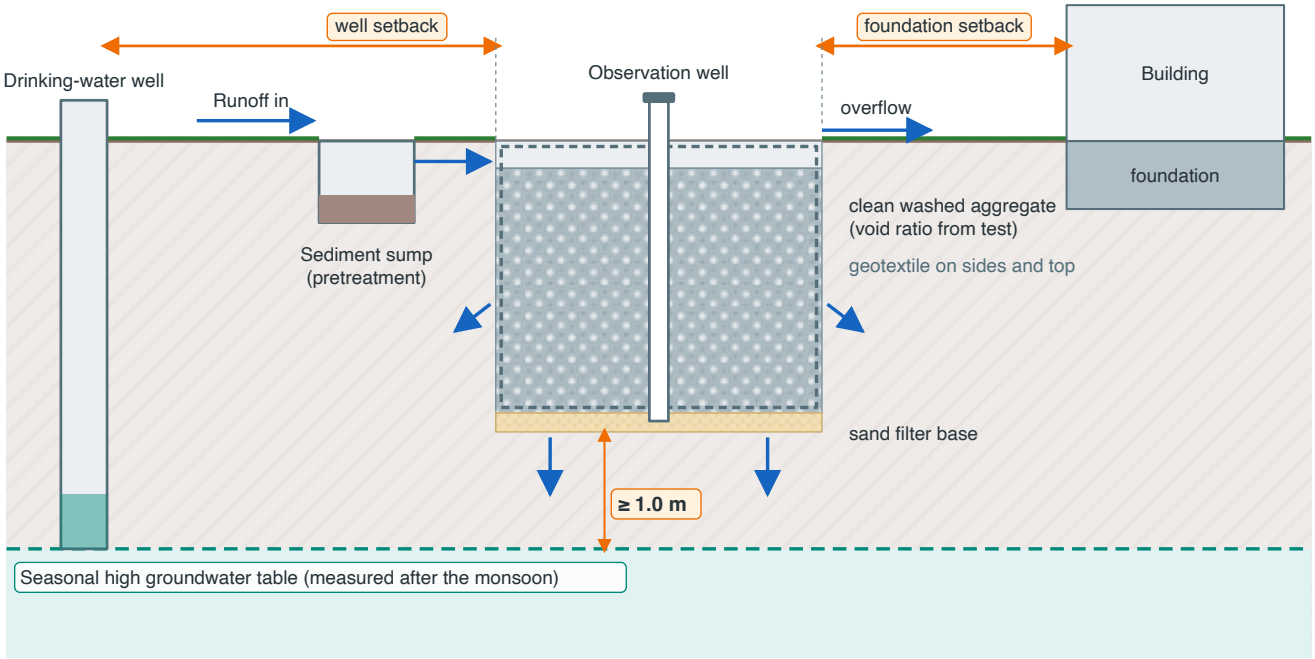
> No statewide map of hydrologic soil groups for Chhattisgarh was found, and every infiltration design needs the tests of Clause 8.8.3; the soils, geology, depths and categories below are from agronomy papers, the CGWB's assessments, factsheet and district reports for the State, and one field study. About 87% of the State is underlain by consolidated (hard) rock (CGWB, 2024): granite-gneiss under the Bastar plateau and the northern hills, the Gondwana sandstone, shale and coal of Korba, Raigarh, Surguja and Koriya, where groundwater sometimes flows at the surface and can be as hot as 50 °C, and, under the Chhattisgarh plain, the limestone, dolomite and shale of the Chhattisgarh Supergroup, whose limestones the CGWB describes as "Karstified and Cavernous", with a field permeability of 4 to 65 m a day. In the Raipur–Durg–Rajnandgaon belt, "solution cavities along the bedding plane is very common and these cavernous zones are filled with clay material" in the Charmuria limestone, the Chandi limestone holds gypsum in its cavities, and the Gunderdehi shale "acts as an aquiclude" (CGWB, Rajnandgaon district report); the Chandi limestone is the main aquifer that supplies Raipur. On the plain the soils follow the slope, from the thin bhata on the crests through matasi and dorsa to the deep kanhar clay of the lowlands; in Durg district matasi covers about 44% of the land, dorsa about 41%, kanhar about 9% and bhata about 5% (Mandal et al., 2018). Kanhar is "heavy soil with clay content of 40 to 50 percent … The permeability is generally low" (CGWB, Rajnandgaon); on a sandy loam upland field of the agricultural university at Raipur, double-ring tests gave basic infiltration rates of 7.3 and 7.6 mm/h (Ranjan and Singh, 2018), against a design loss of 1.5 to 3.5 mm/h in the CWC Flood Estimation Report for subzone 3(d). In 2023 the water table stood within 5 m of the ground after the

monsoon in about 76% of the State's monitoring wells, and within 2 m in about 18% (CGWB). The State as a whole draws less than half of its groundwater (47.32% in the assessment of 2024), but the Seonath plain is stressed: Dharsiwa in Raipur district (93.51%), Gurur, Nawagarh, Bemetara and Berla were critical, and Durg, Dhamdha and Rajnandgaon among 21 semi-critical blocks (CGWB); a report that no block was critical in 2026 was not verified. In the Korba coalfield, inflows to six opencast mines are reported at about 7,500 to 13,700 m³ a day, with drawdown reaching about 1.2 km from the Kusmunda and Gevra mines (Singh et al., 2023, as summarised), and the Hasdeo is reported to be connected to the mine water through a fault (2018 isotope study, as summarised). This Manual's interim soil groups follow from these: kanhar D, dorsa C (D where vertic or puddled paddy), matasi B (C where shallow or puddled), bhata C (D where rock or ferricrete is within 50 cm, never A), the deep plateau soils of Bastar B and its hill soils D, the red and yellow soils and laterite of the northern hills B or C, urban fill C or D, and overburden dumps, mine-disturbed ground and ash ponds D for every storm, with no infiltration credit (Chapter 3). **The State therefore needs opposite rules in different places.** In the critical and semi-critical blocks of the plain, recharge comes first, with pretreatment (Clause 8.8.1); but the limestone that receives it is cavernous and supplies Raipur's wells, so recharge there takes only filtered water and follows a cavity check. On kanhar, on the lowland paddy and talab beds, on floodplains, on mine-disturbed ground and ash, in a mine's dewatering zone and wherever the seasonal water table is within the separation of Clause 8.8.1, no infiltration is used, and retention is met by storage for use and surface measures, and detention. The Chhattisgarh Ground Water (Management and Regulation) Act, 2022 lets the State enforce rainwater harvesting and recharge "in over-exploited/critical blocks and stressed urban areas" once notified (s. 7), but no notification bringing it into force, or notifying an area under it, was found. Everywhere, runoff from roads and open land goes to surface systems and never directly to a recharge well, borewell or shaft (Clause 8.11.3(b)), and fluoride, arsenic and nitrate are checked where Clause 8.8.1 says so. Clause 8.8.9 has no effect in Chhattisgarh, which has no coast.

Infiltration trench: section, not to scale



Test infiltration at the proposed base level. Design rate = measured rate ÷ safety factor. Empty within 72 h.
Setback distances and test methods: Clauses 8.8.3 to 8.8.6. Road and parking runoff is never piped into a well.
Sandy coastal soils: high rates, shallow groundwater. Laterite, hard rock: low rates. Delta clay: usually unsuitable.

Figure 8.3 — Infiltration trench in section: pretreatment sump, aggregate-filled trench in geotextile, observation well, overflow, the 1.0 m minimum separation to the seasonal high groundwater table, and setbacks to wells and foundations. Original drawing for this Manual, adapted from the Georgia Stormwater Management Manual, 2016 (Vol

2), and CIRIA, The SuDS Manual (C753), 2015.

8.9 Sewage cross-connections and dry-weather flow

8.9.1 **No sewage in storm drains.** No sewage or sullage (grey water from kitchens, bathrooms and laundries), septic tank effluent, or trade effluent shall be connected to, or discharged into, any storm drain, by any pipe, channel or opening.

8.9.2 **Condition of approval.** Before the completion certificate, the Applicant shall show, by a dye or smoke test witnessed by the Authority or the sewerage agency, that every sewage and sullage outlet on the site discharges to a sewer or to an approved on-site system, and that none discharges to a storm drain. Any existing cross-connection on the site shall be removed as a condition of approval, including on redevelopment and on additions to existing buildings.

8.9.3 **Where no sewer exists.** Where no sewer is available, sewage and sullage shall go to an approved on-site or decentralised treatment system. The treated effluent of such a system may be discharged to a storm drain only with the written consent of the Chhattisgarh Environment Conservation Board (CECB) and the drain owner.

8.9.4 **Public drains: DWF survey.** Every ULB in a Tier A area shall survey its storm drain outfalls to waterbodies for dry-weather flow at least once a year, before the SW monsoon, and record:

- (a) the outfalls with DWF;
- (b) the estimated DWF rate; and
- (c) the upstream sources found by tracing, CCTV, dye or smoke testing.

The results shall be published in the register of Clause 10.17.

8.9.5 **DWF interception.** Until the sources are removed, every outfall to a waterbody that carries DWF shall have a DWF interceptor that:

- (a) diverts the DWF to a sewer or a treatment plant;
- (b) does not reduce the capacity of the storm drain for the minor or major event (Chapter 6), with its weir or diversion head losses included in the hydraulic design (Chapter 9);
- (c) has a screen ahead of the diversion; and
- (d) is not a substitute for removing the source connections.

8.9.6 **Sewage overflows.** No new sewage overflow, emergency bypass or pumping station overflow shall be designed to discharge into a storm drain or waterbody, except where the sewerage agency shows that no alternative exists, and then only with screening and with the overflow frequency stated.

> **Commentary.** The prohibition of sewage connections and the removal of existing cross-connections as a condition of approval are core requirements of this Manual, and Clause 1.10.3 allows no variance from them. Sewage in storm drains is the single largest water quality problem of urban drainage in Chhattisgarh. It defeats every other measure in this chapter: BMPs designed for stormwater are overloaded, GPTs fill with sludge, and the receiving tanks and channels become sewage lagoons. CPHEEO (2019) notes that Indian drains carry night soil and urine (Part A §9.5). DWF interception (Clause 8.9.5) is an interim measure. It protects the waterbody while the sources are traced and removed, but it can become permanent if the source work is not done, which is why the annual survey and publication of Clause 8.9.4 are required. The annual survey frequency is set by this Manual.
>
> In Chhattisgarh the drains carry the sewage. In January 2021 the CECB told the National Green Tribunal that sewage generation could be estimated only for Bilaspur, "as the sewerage system exists in Bilaspur only", and that its three sewage treatment plants, of 73.1 MLD, were treating 6.1 MLD; elsewhere septic effluent and sullage run in the open nalas to the Kharun, the Seonath and the Arpa (CECB, monthly progress report of January 2021, filed in O.A. 593/2017). The State's answer has been interception at the nala: at Raipur, whose domestic sewage is

reported to reach the Kharun through eight large nalas (as reported), the treatment plants at Chandanidih (75 MLD), Kara (35 MLD) and Nimora (90 MLD) were being built with diversion weirs on the nalas (CECB, 2021), and some of them are reported not to work, so that untreated sewage still enters the river (March 2026, as reported); at Durg, the National Green Tribunal ordered on 16 January 2026 that the treatment plants for the Shankar and Pulgaon nalas, which carry most of the town's sewage to the Seonath, be finished within six months, with interim treatment until then (as reported); and at Bilaspur the High Court heard in January 2025 that about 40% of the city's sewage would still reach the Arpa untreated after the new plants (as reported). The Municipal Corporation Act, 1956 as adapted in Chhattisgarh is understood to keep the Madhya Pradesh provisions that let the Commissioner require separate drains for sewage and for rain-water (ss. 196 and 204 of the Madhya Pradesh text), but its Chhattisgarh text was not read [TBC]. Raipur's diversion weirs are DWF interceptors under Clause 8.9.5, and Clause 8.9.5(b) applies to them in full: each is sized from the measured dry-weather flow of its nala, does not reduce the nala's capacity for the minor and major events or raise the flood level behind it, and passes the storm flow above its capacity on down the nala to the river, never to the treatment plant or back into the colonies.

8.10 Outfalls to waterbodies: first flush, gross pollutant traps and screens

8.10.1 **Requirement.** Every new storm drain outfall to a waterbody, and every existing outfall on a drain that is rebuilt, widened or extended, shall have:

- (a) a GPT designed under Clause 8.7.10; and
- (b) first-flush treatment: treatment of the WQv (or Q_wq) of the contributing catchment by the BMPs of Clause 8.7, either on the contributing sites or at the outfall.

8.10.2 **Where treatment at the outfall is impracticable.** Where land is not available at a public outfall, first-flush treatment may be provided by:

- (a) a proprietary first-flush or hydrodynamic device sized for Q_wq; or
- (b) a treatment wetland or forebay within the waterbody setback (Clause 7.5.1(b)), outside the waterspread.

8.10.3 **Screens and trash racks.** Screens at outfalls, culvert inlets and pump sumps shall:

- (a) be designed with **50% blockage** for hydraulic capacity;
- (b) have an overflow or bypass path so that a fully blocked screen does not flood property;
- (c) have bar spacing chosen for the hazard: closer spacing for litter capture, wider spacing where blockage would cause flooding; and
- (d) be cleanable from a safe platform, with vehicle access.

8.10.4 **Existing outfalls.** Each ULB shall prepare a programme to fit GPTs and DWF interceptors to its existing outfalls to waterbodies, starting with outfalls to tanks used for drinking water supply and to notified wetlands (Clause 7.12), and shall report progress in the register of Clause 10.17.

Commentary. This Manual requires first-flush treatment and gross pollutant traps at outfalls to waterbodies. Gross pollutants are the most visible form of pollution in Chhattisgarh waterbodies, and plastics also block drains and culverts, which links water quality to flooding. The 50% screen blockage matches the grate clogging factor of Chapter 6. Clause 8.10.3(c) warns against fine screens on large drains, which block in the first hour of a storm and cause the flooding they were meant to prevent.

8.11 Rainwater harvesting integration

8.11.1 **RWH mandate continues.** The rainwater harvesting required by the Chhattisgarh Bhumi Vikas Niyam 1984 (rule 78(4), reported to make rain or roof water harvesting mandatory "for all types of buildings having plot size more than 500 sq. mt." in towns with planning areas notified under the 1973 Adhiniyam (Rajya Sabha answer of 8 July 2019), by the systems of Appendix ढ (N); the amendment of 17 February 2009 substitutes the figures "500" and "300" in the rule, lets the Authority levy a penalty of up to ₹1,000 a year for each 100 m² of built-up area where the owner does not build and maintain the system, changes the filter of Appendix ढ-2 to a layer of fine stone and fine sand, and adds a storage tank as Appendix ढ-4, but the rule's Chhattisgarh text was not read and the threshold is to be confirmed [TBC]; and the rainwater harvesting that Appendix ण makes mandatory in an integrated township), together with that required in State and public-sector buildings with roofs of more than 1,500 sq ft (new) or 5,000 sq ft (existing) by the General Administration Department order of 31 May 2016, and at every development in Nava Raipur Atal Nagar by the Naya Raipur Development Plan-2031 (§10.3.2), continues to apply, subject to the limits of Clause 8.8.1 (Chapter 1). This Clause sets how RWH structures are counted under this Manual.

8.11.2 **Three different functions.** The designer shall keep apart, on the drawings and in the calculations:

- (a) **harvesting**: storage for use (sumps and tanks);
- (b) **recharge**: recharge pits, trenches, shafts and wells that put water into the ground; and
- (c) **detention**: storage with an orifice outlet that is empty at the start of a storm (Chapter 4).

8.11.3 **Recharge structures.**

- (a) Roof runoff may be led to recharge wells, open wells, recharge pits and shafts after a filter or settling chamber.
- (b) Runoff from roads, parking and other paved areas shall not be led directly into any well, borewell or recharge shaft. It may be recharged only through an infiltration facility that meets Clause 8.8, with pretreatment.
- (c) Recharge structures shall meet the groundwater separation of Clause 8.8.1, measured from the base of the pit or the bottom of the filter media, except recharge wells that receive filtered roof water only.

8.11.4 **Credit toward retention and WQv.** A recharge structure counts toward the retention volume of Chapter 4 and the WQv only where:

- (a) its infiltration rate has been tested under Clause 8.8.3;
- (b) it empties within 72 hours at the design rate; and
- (c) it has pretreatment.

A harvesting sump counts toward retention only where its contents are used, by a demand stated in the drainage report, within 72 hours (Chapter 4).

8.11.5 **No credit as detention.** RWH storage does not count as detention unless it is a dedicated volume that is empty when a storm arrives, with an outlet. RWH sumps are assumed **full** at the start of every SW monsoon (JJAS) design storm (Chapter 4).

8.11.6 **Overflow.** Every RWH structure shall have an overflow that discharges at grade to the site drainage, never onto a neighbouring plot or into a sewer.

8.11.7 **Surface-runoff recharge in public spaces.** Recharge trenches, shafts and percolation pits in roads, medians, parks and other public spaces shall meet Clause 8.8 in full, including the hotspot rules for roads with heavy traffic, and shall have a GPT or silt trap at each inlet.

> Commentary. The rule that RWH storage does not count as detention is set in Chapter 4. Chhattisgarh's building rules make rainwater harvesting mandatory, but none of them sets a detention rule. Rule 78(4) of the CGBVN 1984 is reported to provide "for mandatory implementation of Rain / Roof Water Harvesting measures for all

types of buildings having plot size more than 500 sq. mt." in towns with planning areas notified under the 1973 Adhiniyam (Rajya Sabha answer of 8 July 2019); a letter of the Housing and Environment Department of 23 November 2009 confirms that rule 78(4) sets out the planning standards for rainwater harvesting in detail (No. 2940/1445/32/2009). The rule's Chhattisgarh text was not read. The amendment of 17 February 2009 substitutes the figures "500" and "300" in rule 78(4) without saying what they replace, so whether the plot threshold is 500 m² or 300 m² cannot be read from it [TBC]; it adds that the Authority may levy a penalty of up to ₹1,000 a year for each 100 m² of built-up area where the owner does not build and maintain the system as provided, replaces the "round pebbles" of Appendix ढ-2 with a layer of fine stone to 50% of the depth at the bottom and fine sand to 20% in the middle, and adds as Appendix ढ-4 an underground tank in the open margin of the house into which rainwater is led, for washing, cleaning floors and gardening (No. 318/1193/32/2006, items 25 to 27). The options of Appendix ढ are fixed recipes, not a sizing method. The completion inspection form of 2018 asks "whether Rain water harvesting system installed at site as per rule" (Appendix Q2, item 14). Integrated townships must provide rainwater harvesting and the reuse of recycled water (Appendix ण(1)(च), 2008). A draft of 30 March 2017 would have applied rainwater harvesting to buildings on plots above 100 m², a "Reduction of Hardscape" to larger plots, and an unpaved area of at least 20% of the recreational open spaces (draft rules 79-C to 79-G); whether those clauses survived in the final rules was not found. For State and public-sector buildings, the General Administration Department ordered rainwater harvesting in every new building with a roof of more than 1,500 sq ft and every existing pucca roof of more than 5,000 sq ft, from the departments' own budgets (order No. F 9-22/2016/1/5 of 31 May 2016). In Nava Raipur Atal Nagar, "Rain Water Harvesting is mandatory at individual development level" (Naya Raipur Development Plan-2031, §10.3.2). The Raipur Municipal Corporation is reported to have asked developers for at least 50 recharge pits in each township, and the district to have acted against 18 builders and 7 industrial units in 2016 for not installing rainwater harvesting (as reported). The Ground Water (Management and Regulation) Act, 2022 would let the State enforce rainwater harvesting and recharge in notified critical blocks and stressed urban areas (s. 7), but it has not been found in force. The rules set no test of the recharge pit, no protection of the aquifer from road runoff and no storage to be empty before a storm. In the critical and semi-critical blocks of the plain, Dharsiwa (Raipur), Durg and Dhamdha among them, recharge is the main function of RWH (Clause 8.11.2(b)), through a pit or well that has passed the cavity check of Clause 8.8.1 where it reaches limestone; on kanhar, on floodplains and talab beds, on wet lowland, and on mine-disturbed ground, ash or a dewatering zone, harvesting for use comes first, and recharge is not used (Clause 8.8.1). RWH serves water supply and recharge. It is usually full, or its soil saturated, by the time a July to September monsoon depression from the Bay of Bengal arrives after weeks of rain, which is why it cannot be counted as detention.

Clause 8.11.3(b) has no direct precedent; this Manual sets it because road runoff carries oil, metals and pathogens. A recharge shaft or borewell bypasses the soil that would otherwise filter it and delivers it straight to the aquifer that supplies wells nearby. The CGWB *Manual on Artificial Recharge of Ground Water* (2007) gives recharge structure types and filter designs; this Manual adds the source-water restrictions.

8.12 Construction-stage erosion and sediment control

8.12.1 **ESC plan.** Every development with land disturbance of **0.1 ha** or more, and every public drainage or road work, shall have an Erosion and Sediment Control (ESC) plan, approved with the drainage report. It shall show:

- (a) the sequence of clearing, earthworks and stabilisation;
- (b) the area exposed at each stage;
- (c) the measures of Clause 8.12.3, with their locations and sizes;
- (d) the protection of infiltration and retention areas (Chapter 10);
- (e) stockpile, concrete washout and fuel storage locations; and
- (f) the inspection and maintenance schedule.

8.12.2 **Monsoon timing.** Earthworks shall be planned so that:

- (a) the area exposed during the SW monsoon (June to September) is as small as practicable;
- (b) all completed earthworks are stabilised by vegetation, mulch, matting or paving by **31 May**; and
- (c) any area exposed between 10 June and 20 October has sediment controls in place before it is cleared.

8.12.3 **Measures.**

- (a) **Silt fences** along the downslope edge of disturbed areas receiving sheet flow, with the area draining to each fence limited as set in Annex 8E.
- (b) **Sediment traps** for concentrated flow from disturbed areas of up to 2 ha.
- (c) **Sediment basins** for disturbed areas of more than 2 ha. The storage volume below the outlet shall be at least the greater of the runoff from the 50% AEP (2-yr) 24-hour storm and **250 m³ per hectare** of drainage area, with a dewatering outlet that draws water from the surface.
- (d) **Inlet protection** at every existing drain inlet that receives runoff from the site.
- (e) **Stabilised construction exits**, with wheel washing where vehicles enter public roads.
- (f) **Stockpiles** outside Zone 1 of any waterbody setback (Chapter 7), at least 10 m from any drain, and covered during the SW monsoon.
- (g) **Dewatering** discharges passed through a sediment basin, settling tank or filter bag, never pumped directly into a drain or waterbody.
- (h) **Concrete washout** in a lined pit, never into a drain.

8.12.4 **Design storm for temporary works.** Temporary ESC measures use the rainfall of Chapter 2 with a climate factor of 1.0 (Clause 2.8.2).

8.12.5 **Inspection.** ESC measures shall be inspected weekly and within 24 hours after every heavy-rainfall event (as defined in Chapter 10), repaired within 48 hours, and cleaned when half full of sediment.

8.12.6 **Removal.** Temporary measures shall be removed only after the area draining to them is stabilised. Sediment shall be removed from any permanent facility used for sediment control before its as-built survey (Chapter 10).

> Commentary. The 0.1 ha threshold is the applicability threshold of Chapter 1. The sediment basin volume of 250 m³/ha is the US EPA Construction General Permit minimum of 3,600 ft³ per acre drained, converted to SI (US EPA, 2022 CGP); the CGP allows the alternative of the 2-year, 24-hour runoff. This Manual requires the greater of the two, because Chhattisgarh's 2-year 24-hour depths are high and construction sites are commonly left exposed through the SW monsoon. The 2 ha division between traps and basins, the 31 May stabilisation date, the 10 m stockpile distance and the inspection frequency have no direct precedent and are set by this Manual. The 31 May date matches the pre-monsoon deadline of Chapter 10. Construction sediment is a direct flood hazard as well as a pollutant: it fills drains, culverts and tanks, and in Chhattisgarh it settles in the city talabs, the Nava Raipur lakes and the reservoirs that the nalas feed (Clause 7.9, Commentary). In the coalfields of Korba, Raigarh and Surguja the load is heavier, from overburden dumps, haul roads and mine benches, and it is governed by the mine's environmental clearance: a clearance of 13 August 2024 for a coal mine in the State requires garland drains "designed keeping at least 50% safety margin over and above the peak sudden rainfall and maximum discharge in the area adjoining the mine sites", which is not a return period, a sump with "adequate retention period to allow proper settling of silt material of the surface runoff", that "the drains shall be regularly desilted particularly after monsoon", and that the major water channels around the site not be altered (conditions 3.6 to 3.8). The ash ponds of the power plants add a second load: their dykes are reported to have breached at Dhanras in 2024 and at Korba West in April 2026, and slurry from BALCO's ash dam is reported to have reached the Belgari nala and the Hasdeo (as reported; to be verified with the CECB). No measurement of the quality of runoff or seepage from Chhattisgarh's mines, dumps or ash ponds was read for this Manual, so acid drainage and leaching from ash are neither assumed nor ruled out; where runoff or seepage from a mine, a dump, a pit lake or an ash pond is found to be acidic or contaminated, its treatment and discharge are for the lessee or operator under its consent from the CECB, and it is kept out of every storm drain, BMP and recharge structure.

8.13 Monitoring

8.13.1 **Outfall monitoring.** Each Tier A authority shall monitor water quality at a representative set of its outfalls to waterbodies, including every outfall to a drinking-water tank or notified wetland. Monitoring shall include:

- (a) dry-weather sampling (Clause 8.9.4), at least once a year before the SW monsoon; and
- (b) wet-weather sampling of at least three events each SW monsoon at a set of index outfalls.

8.13.2 **Parameters.** At minimum: TSS, BOD, COD, faecal coliform, total nitrogen, total phosphorus, oil and grease, and, for industrial catchments, heavy metals. Results shall be compared with the CPCB water quality criteria for the designated best use of the receiving waterbody.

8.13.3 **Gross pollutant records.** The mass or volume of material removed from each GPT and screen at each cleaning shall be recorded in the O&M records (Chapter 10).

8.13.4 **BMP performance monitoring.** The State Stormwater Technical Committee shall run a BMP performance calibration programme and shall revise Table 8.3 and Annex 8C from its results.

8.13.5 **Publication.** Monitoring results shall be published in the register of Clause 10.17.

> Commentary. No Chhattisgarh programme of stormwater quality monitoring was found during research for this Manual, and CPHEEO (2019) sets no Indian targets. The monitoring in Clause 8.13 is the minimum needed to calibrate Table 8.3 and to show whether DWF interception and source removal are working. The parameter list follows the key pollutants named in CPHEEO's WSUD procedure (TSS, nitrogen, phosphorus and gross pollutants), with BOD and faecal coliform added because of sewage contamination. The monitoring frequencies are set by this Manual.

8.14 Submittal requirements

Every drainage report under this chapter shall include:

1. the **Water Quality Sheet (Form 8-1)**: P_ret, P_3m and P_wq; the WQv and Q_wq for each drainage area; the volume retained under Chapter 4; and the BMP serving each area;
2. for Tier A: the LoS calculation (ΔCN, required LoS) and the composite VR (Clause 8.6);
3. the design of each BMP to Clause 8.7 and Annex 8D, with pretreatment;
4. for infiltration and recharge: the **Infiltration Test Record (Form 8-2)**, the seasonal high groundwater level and its source, the design rate, the drawdown time, and the setbacks of Clause 8.8.6;
5. the sewage and sullage connection plan and the dye or smoke test record of Clause 8.9.2;
6. the GPT and first-flush design for any outfall to a waterbody (Clause 8.10);
7. the RWH layout, separating harvesting, recharge and detention (Clause 8.11); and
8. the ESC plan (Clause 8.12).

8.15 Review and update

8.15.1 This chapter shall be reviewed with the rest of the Manual (Chapter 1). Table 8.3 shall also be reviewed when the calibration programme of Clause 8.13.4 reports, and in any case within 5 years of notification.

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Annexes and forms

Annex / Form	Content
8A	3-month ARI 24-hour depth and IDF by zone
8B	Redevelopment LoS table and treatment-train composite VR
8C	Chhattisgarh BMP performance calibration
8D	BMP design sheets, media specification, plant lists
8E	Erosion and sediment control standard details
Form 8-1	Water Quality Sheet
Form 8-2	Infiltration Test Record

CHAPTER 9

Hydraulic Design of Structures

9.1 Purpose and scope

9.1.1 This chapter sets the hydraulic design methods for the structures used in the minor and major systems (Chapter 6), in retention and detention facilities (Chapter 4), at outfalls (Chapter 5) and at waterbodies (Chapter 7). It covers:

- (a) pipes and box drains (Clause 9.3);
- (b) culverts (Clause 9.4);
- (c) open channels (Clause 9.5);
- (d) energy dissipators, outlet protection and drop structures (Clause 9.6);
- (e) detention outlet structures (Clause 9.7);
- (f) trash racks and screens (Clause 9.8);
- (g) pumps and sumps (Clause 9.9);
- (h) tide gates, flap gates and non-return valves (Clause 9.10);
- (i) inlets and gratings (Clause 9.11);
- (j) manholes, junction chambers and access (Clause 9.12); and
- (k) materials and durability, including coastal and saline exposure (Clause 9.13).

9.1.2 **Structural design is outside the scope of this Manual.** Structures shall be designed structurally to the applicable Indian Standards and IRC codes, for example IS 456 (plain and reinforced concrete), IS 458 (concrete pipes), IRC:SP:13 (small bridges and culverts) and the IRC bridge codes. Where this chapter refers to a structural standard, it does so only for durability (Clause 9.13) or hydraulic performance.

9.1.3 Design flows, design AEPs and level-of-service limits come from Chapters 2 to 7. This chapter does not change them.

> **Commentary.** APWA Section 5600 places most structure hydraulics in its sections on inlets and manholes (5604), enclosed pipe systems (5606), engineered channels (5607) and detention (5608). CPHEEO (2019) spreads them across Chapters 5, 6, 8 and 11. This chapter gathers them in one place and points to the US Federal Highway Administration (FHWA) hydraulic circulars for methods that Indian codes do not cover in enough detail: HDS-5 for culverts, HEC-14 for energy dissipators, HEC-22 for inlets and access holes, and HEC-24 for pump stations. The FHWA editions cited are those listed in the FHWA hydraulics library as of October 2026.

9.2 General requirements

9.2.1 **Tailwater.** Every structure that discharges to a river, nala, talab, reservoir, barrage pond or mine void whose level can rise above the outfall invert shall be analysed with the tailwater conditions of Chapter 5: the non-tidal tailwater of Clause 5.6 for minor- and major-system design; the riverine backwater of Clause 5.8, including the flood released from any dam or barrage upstream, such as Gangrel (Ravishankar Sagar) and the barrages below it on the Mahanadi, Minimata Bango and the Hasdeo barrage at Darri on the Hasdeo, the Mongra barrage on the Seonath, the Arpa

Bhaisajhar barrage on the Arpa, Kharang (Khutaghat) and the Kelo dam, and the backwater of the Hirakud reservoir in Odisha in the low reaches of the Mahanadi and the Mand in Raigarh and Sarangarh districts; the outfall-closed check of Clause 5.10 wherever the receiving water can rise above the outfall invert, which is the case for the drains and nalas of Raipur that outfall to the Kharun or to the city's talabs, for those of Durg and Bhilai that outfall to the Seonath and its nalas, for those of Bilaspur that outfall to the Arpa, for those of Korba that outfall to the Hasdeo below Darri, for those of Raigarh that outfall to the Kelo, for those of Jagdalpur that outfall to the Indravati, and for the drains of Nava Raipur that outfall to its lakes and reservoirs; for an outfall to a reservoir, barrage pond, lake or talab, its maximum water level, or its full tank level plus the surcharge in the design flood, as Chapter 5 sets it, or, where neither is published, the full tank level plus 0.5 m (set by this Manual); for an outfall to a mine void or pit lake, which this Manual permits only with the consent of the lessee and of DGMS, the surveyed spill level of the void and the level it reaches when it holds the 72-hour design volume, never a storage assumed to be empty; the level held in a drain by any sluice, interception weir, anicut, check dam, pond outlet weir or pumping station downstream of the outfall, among them the diversion weirs that take Raipur's nalas to its sewage treatment plants, taken as a fixed control at its gate-closed level, its weir crest or the start level of its pumps; and the joint case of Clause 2.12. At Jagdalpur the levels are those of the CWC site on the Indravati (warning level 539.49 m, danger level 540.80 m; high flood level 544.891 m, 26 September 2026, which replaced 544.68 m of 1973; CWC levels, as reported); readings of a local gauge published in the press, on another zero, are never mixed with them. On the Kharun at Pathardihi, below Raipur, only a high flood level has been found, 281.68 m (29 August 2020, as reported), with no warning or danger level. No document found sets a design river, reservoir, barrage or mine-void level at any outfall in Chhattisgarh; until one is published, the levels of Chapter 5 apply.

9.2.2 **Climate factor.** Design flows shall include the climate factor of Clause 2.8.

9.2.3 **Maintenance access.** Every structure shall be accessible for inspection and cleaning by the methods in Clause 9.12 and Chapter 10, without manual entry into confined spaces wherever practicable.

9.2.4 **Design life.** Structures shall be designed for a service life of at least 50 years for concrete and masonry works, and at least 25 years for mechanical and electrical equipment, unless the approving authority agrees otherwise.

9.2.5 **Blockage.** Where a structure can be blocked by debris, it shall be checked in the blocked condition set out in this chapter, and a safe overflow route shall exist for that condition (Chapter 6, Clause 6.3.2).

> Commentary. The design-life values in Clause 9.2.4 have no direct precedent; this Manual sets them because they are needed to decide when the climate factor applies (Clause 2.8.2 applies it to elements with a design life over 25 years).

9.3 Pipes and box drains

9.3.1 **Capacity and roughness.** Pipe and box-drain capacity shall be computed by Manning's equation with the design n of Chapter 6, Table 6.6. For precast or cast-in-situ RCC box drains, the design n shall be at least 0.015.

9.3.2 **Partly full flow.** Depth, velocity and discharge in partly full circular pipes shall be computed from the hydraulic-element relationships for circular sections (for example CPHEEO 2019, Table 5.5). The minimum velocity of Clause 6.7.3 applies at the design flow.

9.3.3 **Pressure flow.** Where a pipe runs full or surcharged, the HGL shall be computed under Chapter 6, Clause 6.8, with friction and structure losses.

9.3.4 **Steep pipes.** Where the design flow in a pipe is supercritical (Froude number above 1), the designer shall:

- (a) locate any hydraulic jump and show that it occurs in a structure designed to contain it, not in a pipe run;
- (b) provide air venting where the jump can seal the pipe; and
- (c) meet the maximum velocity of Clause 6.7.3(b).

9.3.5 **Changes of size and direction.**

- (a) Pipe size shall not decrease downstream.
- (b) Pipes shall join only at manholes or junction chambers, except that inlet connections may join pipes of 1,200 mm or more through a factory-made junction.
- (c) Changes of direction greater than 90° in a single structure are not permitted.

9.3.6 **Flotation.** Pipes and box drains in high groundwater shall be checked for flotation (Chapter 6, Clause 6.7.7).

> **Commentary.** The design n of 0.015 for box drains is set by this Manual to match the "fair" concrete value in Table 6.6. Clauses 9.3.4 and 9.3.5 follow common practice in FHWA HEC-22 and other manuals. A jump inside a pipe run causes pressure surges and can blow off manhole covers.

9.4 Culverts

9.4.1 **Method.** Culverts shall be designed by the methods of FHWA *Hydraulic Design of Highway Culverts*, HDS-5, 3rd edition (2012), or by software that implements them (for example FHWA HY-8). The design headwater is the **greater** of the headwater under inlet control (HWi) and the headwater under outlet control (HWo).

9.4.2 **Inlet control.** HWi shall be computed from the HDS-5 inlet-control equations or nomographs for the barrel shape, material and inlet edge condition actually built.

9.4.3 **Outlet control.** HWo shall be computed as:

> HWo = ho + H − L·S

> H = [1 + ke + (19.63 n² L) / R^1.33] · V² / 2g (SI units, full barrel)

> ho = the greater of TW and (dc + D)/2

where ho is the outlet water depth used for control (m), H is the total head loss through the barrel (m), L is the barrel length (m), S is the barrel slope (m/m), ke is the entrance loss coefficient (Table 9.1), n is the barrel roughness, R is the hydraulic radius (m), V is the barrel velocity (m/s), TW is the tailwater depth above the outlet invert (m), dc is the critical depth (m) and D is the barrel rise (m). Part-full barrels under outlet control shall be analysed by backwater calculation as set out in HDS-5.

Table 9.1 — Entrance loss coefficients ke (outlet control, full or partly full)

Barrel and inlet	ke
Concrete pipe, projecting from fill, socket (groove) end	0.2
Concrete pipe, projecting from fill, square-cut end	0.5
Concrete pipe, headwall or headwall with wingwalls, socket end	0.2

Barrel and inlet	ke
Concrete pipe, headwall or headwall with wingwalls, square edge	0.5
Concrete pipe, headwall, rounded entrance (radius = D/12)	0.2
Concrete pipe, mitred to conform to fill slope	0.7
Concrete pipe, bevelled edges	0.2
Corrugated metal pipe, projecting from fill	0.9
RCC box, headwall parallel to embankment, square edge on three sides	0.5
RCC box, headwall parallel to embankment, three edges rounded or bevelled	0.2
RCC box, wingwalls at 30°–75° to barrel, square-edged crown	0.4
RCC box, wingwalls at 10°–25° to barrel, square-edged crown	0.5
RCC box, wingwalls parallel (extension of sides), square-edged crown	0.7

Source: FHWA HDS-5 (2012), entrance loss coefficient table (Appendix C).

9.4.4 **Tailwater.** TW shall be the higher of:

- (a) the normal or backwater depth in the downstream channel at the design flow; and
- (b) the tailwater level of Clause 9.2.1, for culverts within tidal or river backwater reach.

9.4.5 **Headwater limits.** At the design AEP of Chapter 6, Table 6.1:

- (a) HW/D shall not exceed 1.2;
- (b) the headwater shall be at least 0.3 m below the edge of the road shoulder at the upstream face; and
- (c) the headwater shall not raise water levels at any upstream building or property boundary above the limits of Chapter 6, Clause 6.12.2(d).

9.4.6 **Road overtopping.** Flow over the road in the check and 1% AEP events shall be computed with the broad-crested weir equation:

> Qr = Cd · L · h^1.5

where Qr is the overtopping flow (m³/s), L is the length of the overtopping section (m) and h is the depth of the headwater above the road crest (m). Cd shall be taken from HDS-5 for the road surface and the ratio of h to road width, and shall not exceed 1.66 (SI). The culvert and overtopping flows shall be balanced at a common headwater. The overtopping depths and hazard shall meet Chapter 6, Clause 6.12.

9.4.7 **Outlet velocity.** The outlet velocity at the design AEP and at the 1% AEP shall be computed and the outlet protected under Clause 9.6.

9.4.8 **Barrels and debris.**

- (a) Multi-barrel culverts shall be avoided in debris-prone urban drains. Where they are used, one barrel shall carry the dry-weather flow, and the inlet shall have a debris fin or deflector.
- (b) Culverts shall meet the minimum size and blockage checks of Chapter 6, Clauses 6.12.4 and 6.12.5.
- (c) Culvert inverts shall match the bed of the drain. They shall not be set above the bed, which traps silt and water upstream.

9.4.9 **Headwalls and cut-offs.** Inlets and outlets shall have headwalls, wingwalls and cut-off walls designed to prevent piping and undermining.

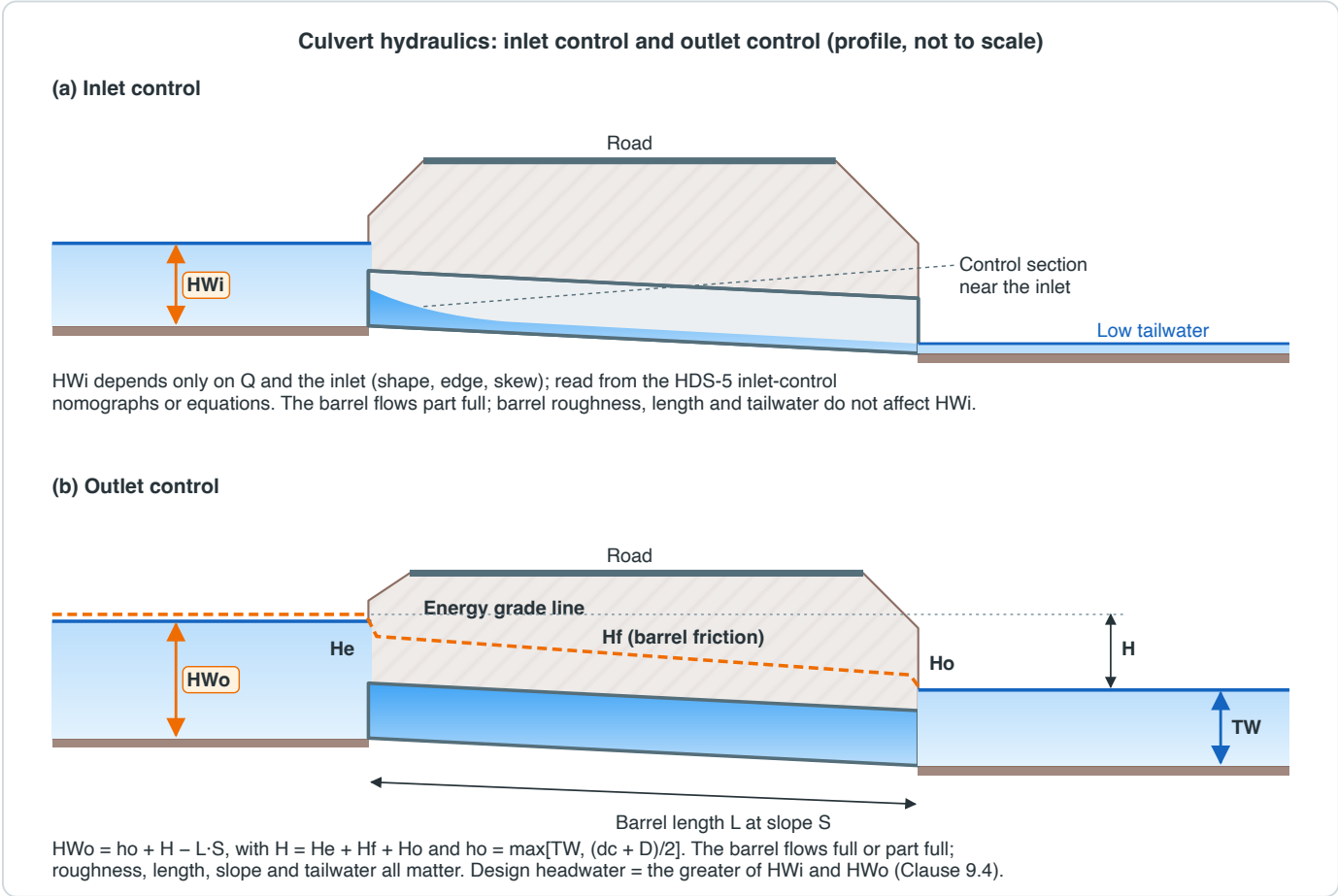

Figure 9.1 — Culvert hydraulics: inlet control and outlet control. Original drawing for this Manual, adapted from FHWA, Hydraulic Design of Highway Culverts, HDS-5 (3rd ed., 2012).

> **Commentary.** HDS-5 (3rd ed., FHWA-HIF-12-026, April 2012) merged the culvert design material of HEC-5, HEC-10 and HEC-13 and is the current FHWA culvert manual. The outlet-control equation in Clause 9.4.3 is the HDS-5 energy equation in SI units: the friction coefficient 19.63 equals 2g. The ke values in Table 9.1 are reproduced from HDS-5. The weir coefficient cap of 1.66 (SI) is the conversion of the APWA 5600 (2006) limit of C ≤ 3.0 (US units) for overtopping calculations. The headwater limits of Clause 9.4.5 are set by this Manual: HW/D ≤ 1.2 keeps the inlet from operating far into submerged-orifice flow at the design event, where small increases in flow give large increases in headwater. Clause 9.4.8(c) has no direct precedent; it is included because raised culvert inverts are common in Chhattisgarh where a road is rebuilt on top of an old culvert, and they hold standing water and silt upstream.

9.5 Open channels

9.5.1 **Flow computation.** Open channels shall be designed by Manning's equation for uniform flow, with the n of Table 6.6. Water levels shall be computed by gradually varied flow (standard step) analysis where tailwater, bridges, culverts, bends or changes of section affect them, which on the drains of Raipur that outfall to the Kharun and to the city's talabs, on the drains of Durg and Bhilai, Bilaspur and Korba that outfall to the Seonath, the Arpa and the Hasdeo below

barrages and dams whose gates set their levels, on the low outfalls of Raigarh and Sarangarh districts that the Hirakud backwater can reach, on the nalas held by anicuts, check dams and the diversion weirs of the sewage treatment plants, and on the steep nalas of the Bastar plateau and the northern hills stepped by drop structures, is the normal case.

9.5.2 **Flow regime.** At the design flow, the Froude number shall not exceed 0.8 (CPHEEO 2019, §5.3), unless the channel is designed as a supercritical chute. Flows with a Froude number between 0.8 and 1.2 are not permitted at the design flow in any channel.

9.5.3 **Permissible velocities.** Mean velocity at the design flow shall not exceed the limits of Table 9.2.

Table 9.2 — Maximum permissible mean velocities in channels

Lining or soil	Maximum velocity (m/s)	Source
Concrete lining	3.0	CPHEEO 2019 Table 5.11
Riprap lining	1.8	CPHEEO 2019 Table 5.11
Grassed lining	1.2	CPHEEO 2019 Table 5.11
Stabilised earthen channel (modified natural channel)	2.0, or the critical velocity if less	CPHEEO 2019 §5.10.3
Unlined, fine sand	0.46	Fortier and Scobey (1926), clear water
Unlined, sandy loam	0.53	As above
Unlined, silt loam	0.61	As above
Unlined, firm loam	0.76	As above
Unlined, stiff clay	1.14	As above
Unlined, fine gravel	0.76	As above
Unlined, coarse gravel	1.22	As above

Flexible linings (vegetation, riprap, geosynthetics) may instead be designed by the permissible shear stress method of FHWA HEC-15, *Design of Roadside Channels with Flexible Linings*, 3rd edition (2005).

9.5.4 **Longitudinal slope.** In engineered channels, the longitudinal slope shall not exceed 0.4% for natural linings and 0.2% for concrete linings, unless drop structures (Clause 9.6.6) or a chute are provided (CPHEEO 2019, Table 5.11).

9.5.5 **Freeboard and bends.** Freeboard shall meet Chapter 6, Table 6.7, measured above the water level that includes superelevation at bends. Bend radii shall meet Chapter 6, Clause 6.7.10.

9.5.6 **Side slopes.** Unless a structural or geotechnical design supports a steeper slope:

- (a) unlined earth channels shall have side slopes no steeper than 1V:2H; and
- (b) grassed channels that are mown shall have side slopes no steeper than 1V:3H.

9.5.7 **Low-flow channel.** Channels with a bed width of 3 m or more shall have a defined low-flow channel that carries the dry-weather flow at the self-cleansing velocity of Clause 6.7.3.

9.5.8 **Transitions.** Changes of section shall use gradual transitions. Abrupt expansions and contractions are not permitted in supercritical flow.

9.5.9 **Linings in high groundwater.** Lined channels in areas of high groundwater or tidal influence shall have weep holes with filters, or shall be designed to resist uplift with the channel empty.

9.5.10 **Natural channels.** Natural channels shall be preserved where possible (CPHEEO 2019, §5.10.3 note; Chapter 7). Where a natural channel must be modified, its capacity shall meet the design AEP of Chapter 6 and its velocities shall meet Table 9.2.

> **Commentary.** The lined-channel velocities and slopes are CPHEEO (2019) Table 5.11 values. The unlined-soil values are the classic Fortier and Scobey (1926) permissible velocities for clear water, as reproduced in Chow (1959). They are given for soils common in Chhattisgarh (the kanhar, dorsa, matasi and bhata soils of the Chhattisgarh plain, from the deep black clays of the lowlands to the thin gravelly soils over murram and ferricrete on the uplands, the red and yellow soils and laterite of the Bastar plateau and the northern hills, the sandstone and shale soils of the Gondwana coalfields of Korba, Raigarh and Surguja, with the mine-disturbed ground, overburden dumps and ash ponds on them, and the alluvium along the larger rivers). The ban on Froude numbers between 0.8 and 1.2 extends CPHEEO's advice to avoid critical flow, where the water surface "becomes unstable and wavy" (§5.3). This extension is made by this Manual. The side slopes and the low-flow channel are also set by this Manual, because many Chhattisgarh urban channels carry sewage-mixed dry-weather flow until the cross-connections of Chapter 8 are removed. A wide flat bed then deposits sludge across its whole width, and a low-flow channel keeps the dry-weather flow concentrated.

9.6 Energy dissipators, outlet protection and drop structures

9.6.1 **When required.** Outlet protection or an energy dissipator shall be provided at every pipe, culvert and channel outlet where the outlet velocity at the design AEP or the 1% AEP exceeds the permissible velocity of the receiving channel in Table 9.2, and at every outlet to an unlined channel, tank or waterbody.

9.6.2 **Method.** Energy dissipators and outlet protection shall be designed by the methods of FHWA HEC-14, *Hydraulic Design of Energy Dissipators for Culverts and Channels*, 3rd edition (2006), selecting the dissipator type by its range of application.

9.6.3 **Riprap aprons.** For a circular outlet discharging to a flat apron, the median riprap size shall be computed with the HEC-14 apron equation:

> $$D50 = 0.2 \cdot D \cdot [Q / (g^{0.5} \cdot D^{2.5})]^{(4/3)} \cdot (D / TW)$$

where D50 is the median stone size (m), D is the culvert diameter or rise (m), Q is the design discharge (m³/s), g = 9.81 m/s² and TW is the tailwater depth (m). Where TW is not known, it shall be taken as 0.4D.

Apron length and thickness shall follow the HEC-14 riprap-class table (Table 9.3). The apron width at the outlet shall be at least 3D and shall flare to the downstream channel.

Table 9.3 — Riprap classes and apron dimensions (HEC-14)

Class	D50 (mm)	Apron length	Apron thickness
1	125	4D	3.5 D50
2	150	4D	3.3 D50
3	250	5D	2.4 D50
4	350	6D	2.2 D50
5	500	7D	2.0 D50

Class	D50 (mm)	Apron length	Apron thickness
6	550	8D	2.0 D50

Source: FHWA HEC-14 (2006), §10.2 and Table 10.1, as reproduced in state-agency design worksheets. Where the computed D50 exceeds Class 6, a riprap basin or a structural stilling basin shall be used.

9.6.4 **Riprap filter and grading.** Riprap shall be placed on a granular filter or a geotextile. Stone shall be angular, durable and graded so that the D50 of Clause 9.6.3 is met.

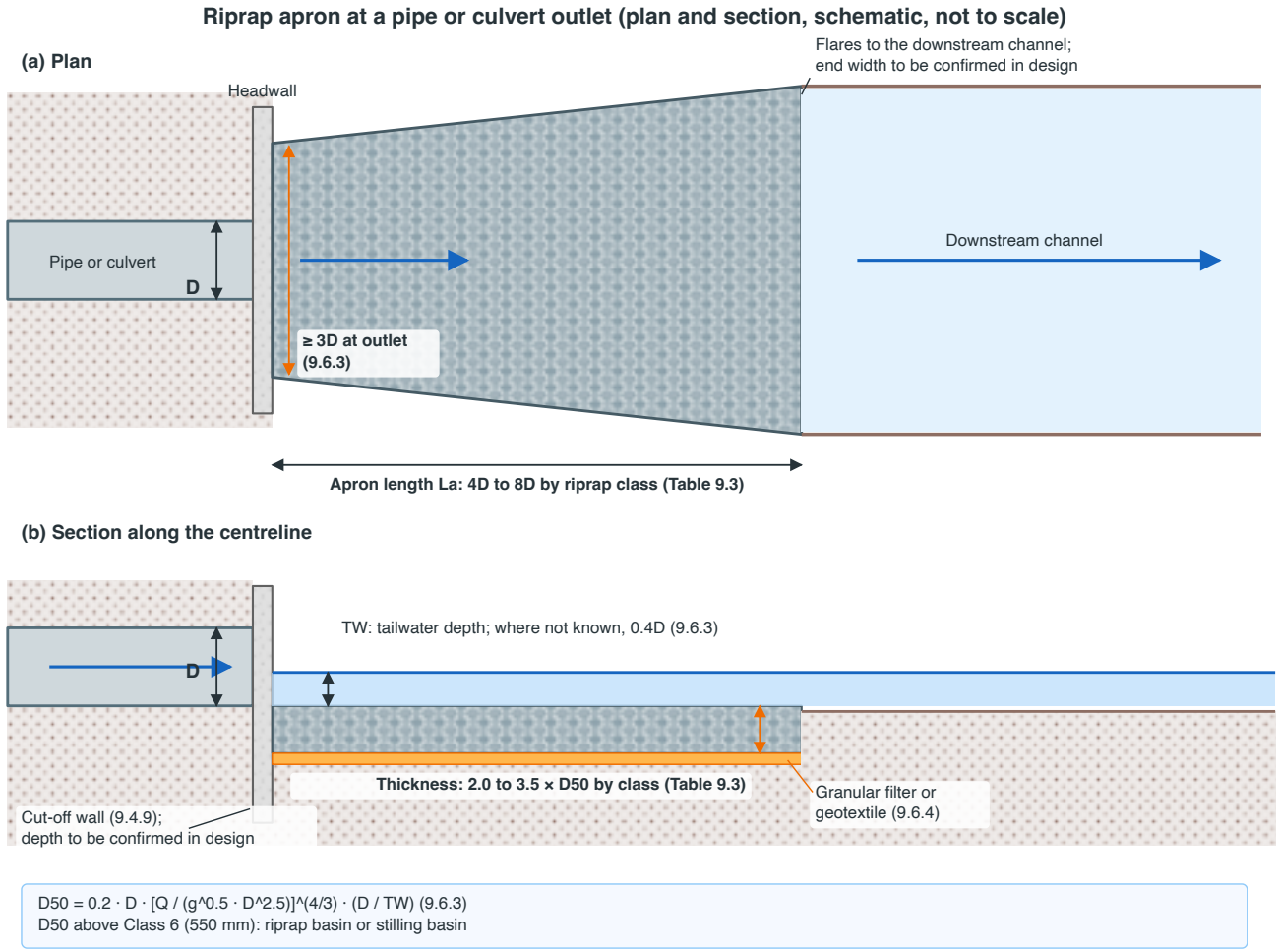

Figure 9.2 — Riprap apron at a pipe or culvert outlet: apron width at least 3D at the outlet, flaring to the downstream channel; length and thickness from Table 9.3; stone on a granular filter or geotextile (Clauses 9.6.3 and 9.6.4). Original drawing for this Manual, adapted from FHWA, Hydraulic Design of Energy Dissipators for Culverts and Channels, HEC-14 (3rd ed., 2006), §10.2.

9.6.5 **Stilling basins.** Where velocities or Froude numbers are above the range of riprap aprons and basins, a structural stilling basin or impact basin shall be designed from HEC-14 or from the US Bureau of Reclamation basin types it describes.

9.6.6 **Drop structures.** Drop structures shall be used where the channel slope would otherwise exceed Clause 9.5.4 or the velocity limit of Table 9.2.

- (a) For a straight vertical drop, the drop number $D = q^2 / (g\,h^3)$ shall be computed, where q is the unit discharge (m^2/s) and h is the drop height (m). The pool geometry shall be set from the Rand (1955) relationships: drop length Ld/h = 4.30 D^0.27; pool depth Yp/h = 1.00 D^0.22; sequent depths y1/h = 0.54 D^0.425 and y2/h = 1.66

D^0.27.

- (b) Baffled chutes and stepped drops shall be designed by HEC-14 or US Bureau of Reclamation methods.
- (c) The height of a single vertical drop in a public drain shall not exceed 1.5 m. Drops shall have edge protection under Chapter 6, Clause 6.13.8.

9.6.7 **Coastal and beach outfalls.** Outfalls onto beaches or into estuaries shall be protected against wave action and scour. Where littoral sand can close the outfall, the outfall shall be designed so that it can be reopened by machine, and the drainage system shall be checked with the outfall closed under Chapter 5.

> Commentary. HEC-14 (3rd ed., FHWA-NHI-06-086, July 2006) is the current FHWA energy-dissipator manual. The apron equation and the class table are from HEC-14 §10.2 (Equation 10.4 and Table 10.1). The equation and table are given here as reproduced in secondary design documents (a Tennessee handbook and a state-agency riprap worksheet). The 3D apron width is from the same sources. A Purdue study (JTRP) found the HEC-14 apron method somewhat conservative within its range of application. The Rand (1955) drop relationships are standard and are reproduced in Chow (1959). The 1.5 m limit on a single vertical drop has no direct precedent; this Manual sets it for public safety in urban drains.

9.7 Detention outlet structures

9.7.1 **Primary outlet.** The primary outlet of a detention facility shall:

- (a) produce the release rates and drawdown times of Chapter 4;
- (b) work without mechanical parts or operator action, except in pumped facilities (Clause 9.9);
- (c) have no orifice smaller than **50 mm**, and every orifice shall be protected by a trash screen (Clause 9.8.5); and
- (d) be accessible for cleaning from the surface.

9.7.2 **Orifices.** Orifice flow shall be computed as:

> $Q = C_d \cdot A \cdot (2 g h)^{0.5}$

where A is the orifice area (m²) and h is the head (m) on the centroid of the orifice, or the difference between the upstream and downstream water levels where the orifice is submerged. Cd shall be 0.60 for a sharp-edged orifice in a thin plate. Other edge conditions shall use published tested values.

9.7.3 **Weirs.** Weir flow shall be computed as $Q = C_w \cdot L \cdot H^{1.5}$, with:

- (a) sharp-crested rectangular weirs: Cw = 1.84 (SI), with the effective length reduced by 0.1 H for each end contraction;
- (b) V-notch weirs: $Q = 1.38 \cdot \tan(\theta/2) \cdot H^{2.5}$ (SI), where θ is the notch angle; and
- (c) broad-crested weirs and spillway crests: Cw = 1.44 to 1.70 (SI), selected from published tables for the crest shape and head.

Submerged weirs shall be corrected for submergence from published relationships.

9.7.4 **Stage–discharge rating.** For multi-stage outlets (orifices and weirs in a riser, with an outlet pipe), the designer shall compute, at each stage, the discharge under every possible control (each orifice, each weir, riser crest, and the outlet pipe under inlet and outlet control) and use the **smallest** combined value. The rating shall be tabulated from the outlet invert to the emergency spillway crest plus the design head.

9.7.5 **Tailwater.** Ratings shall be computed with the tailwater of Clause 9.2.1. Where the receiving water can rise above the outlet, the outlet shall have a flap gate or non-return valve under Clause 9.10, and the storage shall meet the outfall-closed check of Chapter 5.

9.7.6 **Emergency spillway.** The emergency spillway shall pass the 1% AEP inflow with the primary outlet assumed blocked and the basin full at the start, with **0.3 m** freeboard to the embankment crest. High-hazard basins shall also be checked for the 0.2% AEP inflow (Chapter 4).

9.7.7 **Outlet pipes through embankments.** Outlet pipes through embankments shall have a filter diaphragm or seepage collars to prevent piping along the pipe, and a flared or headwalled inlet.

9.7.8 **Anti-vortex.** Riser outlets with a design head above the riser crest greater than the riser diameter shall have an anti-vortex plate.

> Commentary. The 50 mm minimum orifice with a trash screen, the emergency-spillway criterion and the drawdown times are core criteria of this Manual (Chapter 4). The minimum orifice is smaller than the APWA 5600 (2006, 5608.4) minimum of 3 in (about 75 mm). It allows the low release rates of the Chapter 4 mixed-frequency rule on small sites, and it relies on a good screen (Clause 9.8.5). The "no mechanical parts" rule follows APWA 5600 (2006, 5608.4), which requires the primary outlet to function unattended. The orifice and weir coefficients are standard textbook values (for example Brater and King, *Handbook of Hydraulics*; Chow 1959). The 1.84 sharp-crested coefficient and the 0.1H end-contraction correction are the Francis formula in SI units. The broad-crested range 1.44–1.70 is the SI equivalent of the US range 2.6–3.1 commonly tabulated. Clauses 9.7.7 and 9.7.8 follow common dam and detention practice.

9.8 Trash racks and screens

9.8.1 **Where required.** Trash racks or screens shall be provided:

- (a) where an open drain enters a pipe, covered drain or culvert longer than 30 m (Chapter 6, Clause 6.13.4);
- (b) at every detention outlet orifice (Clause 9.7.1);
- (c) at the inlet of every pump sump (Clause 9.9); and
- (d) at outfalls to waterbodies, as part of the gross pollutant trap of Chapter 8.

9.8.2 **Conduit-entrance racks.** Racks at the entrance to conduits and culverts shall have:

- (a) clear bar spacing not less than 75 mm and not more than half the smallest internal dimension of the conduit;
- (b) a gross area at least 4 times the conduit's cross-sectional area;
- (c) a mean velocity through the rack at the design flow, with 50% of the rack blocked, of not more than 1.0 m/s; and
- (d) an inclination no steeper than 1V:2H where the rack is to be raked by hand, with a working platform and handrail.

9.8.3 **Head loss.** Head loss through a clean rack shall be computed with the Kirschmer equation:

$$h_r = \beta \cdot (t / b)^{4/3} \cdot (V^2 / 2g) \cdot \sin\theta$$

where t is the bar thickness, b is the clear bar spacing, V is the approach velocity, θ is the angle of the rack from the horizontal, and β is the bar shape factor (2.42 for sharp-edged rectangular bars; 1.79 for circular bars). The HGL and headwater shall be checked with the rack **50% blocked** (head loss computed with half the open area).

9.8.4 **Overflow when blocked.** Every rack shall have an overflow route so that a fully blocked rack does not flood buildings or exceed the limits of Chapter 6.

9.8.5 **Detention outlet screens.** Screens over detention orifices shall have openings smaller than the orifice they protect, and an open area at least 10 times the orifice area.

9.8.6 **Pump sump screens.** Bar screens at pump-station inlets shall have a clear spacing smaller than the largest solid the pumps can pass, as stated by the pump manufacturer. Screens shall be built in removable modules, and a mechanically raked screen shall be used where the design inflow exceeds 1 m³/s.

9.8.7 **Safety.** Racks at the entrance to long conduits shall prevent people from being drawn in. They shall be designed for the hydrostatic load of a fully blocked rack.

> Commentary. CPHEEO (2019, §8.3.1.3) describes pump-station bar screens with a spacing of "approximately 58 mm", built in modules, with an emergency overflow in case of clogging. Clause 9.8.6 follows that approach but links the spacing to the pump's solids-passing size, which varies by pump. The 50% blockage check matches the grate clogging factor of Chapter 6. The Kirschmer equation and its shape factors are standard (Kirschmer 1926, as reproduced in hydraulic handbooks). The rack spacing, area ratio, through-velocity, the 30 m conduit length trigger, the 10× screen area for orifices and the 1 m³/s trigger for mechanical raking have no direct precedent and are set by this Manual.

9.9 Pumps and sumps

9.9.1 **When pumping is used.** Pumping shall be used only where gravity drainage cannot meet the requirements of Chapters 5 and 6, for example at road underpasses (Chapter 6, Clause 6.10.5), basements, low-lying areas and outfalls that are tide-locked under the outfall-closed check of Chapter 5.

9.9.2 **Design inflow and storage.** The station shall be designed with the inflow hydrograph routed through the catchment and any upstream storage. Pump capacity and storage shall be chosen together:

- (a) the station shall meet the design AEP of the element it serves (for underpasses, the 2% AEP with a 1% AEP check);
- (b) where the station serves a tidal outfall, storage plus pumps shall hold the 48 h design volume with the outfall throttled for the tidal-lock duration (default: two consecutive high tides of 6 h each), as required by Chapter 5; and
- (c) the pumps shall be selected for the full range of static head between the lowest sump level against the lowest tailwater, and the highest tailwater (HAT plus surge plus 0.3 m).

9.9.3 **Wet-well volume.** The effective wet-well volume (between the pump-stop and the lowest pump-start level) shall be the greatest of:

- (a) where variable-speed drives and automatic controls are installed, 5 minutes of the minimum inflow (CPHEEO 2019, §8.3.3);
- (b) otherwise, 10 to 15 minutes of the peak inflow, or the volume needed for the pump manufacturer's minimum cycle time, from $V = Q \cdot t$ (CPHEEO 2019, Eq. 8.1); and
- (c) for constant-speed pumps, the volume $V = \Theta \cdot q / 4$ for each pump, where Θ is the minimum cycle time recommended by the motor manufacturer and q is the pump capacity (FHWA HEC-24, 2001).

9.9.4 **Duty and standby.**

- (a) Duty pumps shall be of the same size where practicable (CPHEEO 2019, §8.3.3).
- (b) Standby capacity shall be at least equal to the largest duty pump, and not less than 50% of the number of duty pumps.

- (c) Duty and standby roles shall alternate automatically.

9.9.5 **Pump type.** Pumps shall be solids-handling (non-clog) types suited to stormwater with sand and debris. Axial-flow pumps shall be used only behind a screen that meets Clause 9.8.6, because they cannot handle debris (CPHEEO 2019, §8.3.2).

9.9.6 **Sump and intake design.** Sump geometry, pump spacing, minimum submergence and approach flow conditions shall follow the pump manufacturer's requirements and ANSI/HI 9.8 (Rotodynamic Pumps for Pump Intake Design). A physical hydraulic model study shall be carried out where any pump exceeds 1 m^3/s, or where the sump does not follow standard ANSI/HI 9.8 geometry. The minimum clear space between adjacent pumps or motors is 0.6 m for small and medium units and 1.0 m for large units (CPHEEO 2019, §8.2.5).

9.9.7 **Silt and screens.** A silt trap and a bar screen shall be provided upstream of the wet well (Clause 9.8.6; Chapter 6, Clause 6.13.3).

9.9.8 **Controls and monitoring.**

- (a) Pumps shall be controlled by a primary level sensor (ultrasonic, radar or pressure) with an independent float-switch back-up.
- (b) High-level and pump-fault alarms shall be sent by telemetry to the local body's control room.
- (c) Sump levels, pump run hours and power status shall be logged and kept for at least 5 years.

9.9.9 **Power supply.**

- (a) The station shall have two independent power sources: the grid supply and a standby generator (or a second independent grid feeder) able to run all duty pumps at once (CPHEEO 2019, §2.4.2.3(j)).
- (b) Fuel for at least **72 hours** of continuous operation of all duty pumps shall be stored on site, above the flood level of Clause 9.9.10.
- (c) Changeover to standby power shall be automatic.

9.9.10 **Flood protection of the station.** The floor of the electrical and control room, the generator and the transformer shall be at least 0.5 m above the 1% AEP flood level at the station (Chapter 6, Clause 6.11). Access to the station for maintenance vehicles shall remain usable in the 1% AEP event.

9.9.11 **Discharge.** Each pump shall have its own rising main, or a non-return valve and an isolating valve where mains combine. Rising mains shall discharge into a delivery chamber or outfall fitted with a flap gate or non-return valve under Clause 9.10. Discharge piping shall be sized by a recognised friction formula (CPHEEO 2019, §8.3.9).

9.9.12 **Emergency connections.** Every station shall have a hard-standing area and a quick-coupling connection for mobile pumps to discharge into the delivery chamber.

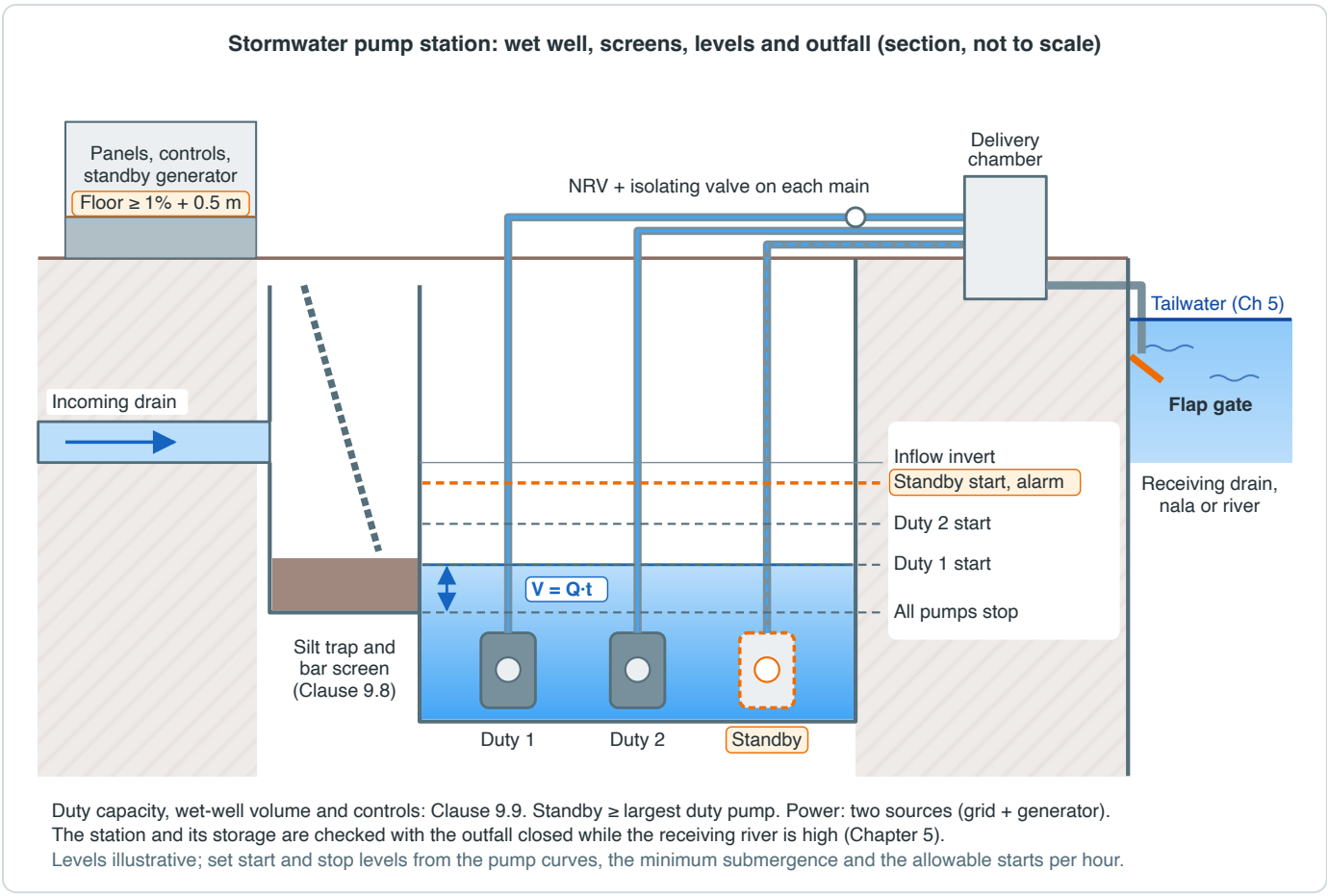

Figure 9.3 — Stormwater pump station: wet well, screens, levels and outfall. Original drawing for this Manual, adapted from CPHEEO, Manual on Storm Water Drainage Systems (2019), Chapter 8, and FHWA, Highway Stormwater Pump Station Design, HEC-24 (2001).

> **Commentary.** CPHEEO (2019) Chapter 8 covers stormwater pumping: wet-well sizing (§8.3.3), pump types (§8.3.2), trash screens (§8.3.1.3), space requirements (§8.2.5) and the need for generator sets "as standby for emergency operation during failure of electricity" (§2.4.2.3). It recommends standby units of "minimum 50% of the duty pump units". Clause 9.9.4(b) is stricter: the standby must at least equal the largest duty pump, so the station still meets its design duty with its largest pump out of service. This stricter rule is set by this Manual. FHWA HEC-24 (*Highway Stormwater Pump Station Design*, FHWA-NHI-01-007, 2001) is the FHWA pump-station manual. The 72 h fuel storage is set by this Manual to match the 72 h design storm of Chapter 2. The model-study trigger, the telemetry and data-retention requirements, and the emergency connection have no direct precedent and are set by this Manual.

9.10 Tide gates, flap gates and non-return valves

9.10.1 **Where required.** Flap gates, tide gates or non-return valves shall be provided on every outfall whose invert is below the 1% AEP still-water level of the receiving water (Chapter 5).

9.10.2 **Types.** Acceptable devices are:

- (a) hinged flap gates (cast iron, ductile iron, stainless steel, HDPE or composite);
- (b) elastomeric in-line or end-of-pipe check valves (duckbill type); and
- (c) penstocks or sluice gates, manual or motorised, as a secondary closure.

9.10.3 **Secondary closure.** Outfalls of 900 mm or more, and every outfall serving a pumped or storage system, shall have a penstock or stop-log slot upstream of the flap gate so that the gate can be maintained, and as a back-up if the gate jams open.

9.10.4 **Hydraulics.** The head loss through the device in the open position shall be taken from the manufacturer's tested data and included in the HGL (Chapter 6, Clause 6.8.7). Flap gates shall open at a small head difference (CPHEEO 2019, §6.3.10.1).

9.10.5 **Sand and debris.** At beach, estuary and creek outfalls where sand or debris can hold a gate open or shut:

- (a) the gate shall be housed in a chamber with surface access, or elastomeric check valves shall be used; and
- (b) the gate shall be inspected after every high-tide or surge event during the monsoon season (Chapter 10).

9.10.6 **Materials.** Hinges, pins, links and fixings shall be of corrosion-resistant material (CPHEEO 2019, §6.3.10.1). In coastal and tidal locations this means stainless steel grade 316, bronze or a non-metallic material (Clause 9.13).

9.10.7 **Storage behind gates.** Where gates close during a storm, the storage or pumping behind them shall meet the outfall-closed check of Chapter 5.

> **Commentary.** CPHEEO (2019, §6.3.10.1) describes flap gates and flood gates at outfalls to tidal waters. It notes that "adequate storage may be necessary if pumping is to be avoided during the closure of the gate" and that hinge pins and links "should be of corrosion-resistant materials". Chhattisgarh has no coast, so no outfall in the State is closed by the tide, and the beach, estuary, tidal and coastal provisions of Clauses 9.6.7, 9.9.2(b), 9.10.5 and 9.13.6, and the coastal and tidal rows of Table 9.5, do not arise; in Clause 9.9.2(c) the highest tailwater is the highest river, reservoir or barrage-pond level of Chapter 5. The aggressive-ground tests of Clause 9.13.3 should still be made at structures in the coalfields of Korba, Raigarh and Surguja, where mine water, the runoff from overburden dumps and the water of ash ponds can carry sulphate or be acidic, and on the Chandi limestone of the Raipur–Durg–Rajnandgaon belt, which holds gypsum in its cavities (CGWB) (this Manual's reading; no survey of mine-water or ash-pond chemistry in Chhattisgarh was found). Gates are still required below the 1% still-water level, because the State's city outfalls end in rivers that dam and barrage releases hold high, in barrage ponds and in talabs. Gangrel (Ravishankar Sagar) on the Mahanadi, whose spillway was designed for 17,230 m³/s (CWC, National Register of Large Dams, 2009), released about 55,000 cusecs (about 1,560 m³/s) through eight of its 14 gates in October 2025, with 72 riverside villages alerted, and opened all 14 gates in September 2026 (as reported); Minimata Bango on the Hasdeo (spillway 24,000 m³/s) released about 30,000 cusecs (about 850 m³/s) through six gates in August 2024, while the Hasdeo barrage at Darri above Korba (spillway 19,820 m³/s) passed about 20,000 cusecs and houses in Korba were flooded (as reported); and the Hasdeo then set new high flood levels at Katghora (313.58 m, 20 July 2026) and Bangodam (303.5 m, 17 August 2026) (CWC levels, as reported). Upstream of Durg and Bhilai the Seonath is held by the Mongra barrage (full reservoir level 337.150 m), at Bilaspur the Arpa by the Arpa Bhaisajhar barrage (302.000 m) and at Raigarh the Kelo by the Kelo dam (233.000 m) (WRD, tank gauge report of 10 October 2024), and Kharang (Khutaghat) in Bilaspur district has a spillway of 994 m³/s (CWC, 2009). The spillways and gates of these dams and barrages are WRD's works, designed to dam-safety standards and not to this chapter, but their operating levels and releases are this chapter's boundary conditions, and no release rule, spillway rating or record of gate operation for any of them was found. A city drain that outfalls below them is closed for as long as a release holds the river above the outfall, so it needs a flap gate, a penstock and either storage or a pumping station sized under Clause 9.9 and the outfall-closed check of Chapter 5; no pumping station on an urban drain in Chhattisgarh was found documented. In Raipur the drains reach the Kharun through its nalas, many by way of the city's talabs; the Kharun at Pathardihi rose to a high flood level of 281.68 m on 29 August 2020, 2.47 m above its mark of 1994, and overtopped the road at Mahadev Ghat in September 2024 and September 2026 (as reported), but no warning or danger level has been found for it. Raipur's new sewage treatment plants at Chandanidih, Kara and Nimora take the flow of the nalas through diversion weirs (CECB, report to the National Green Tribunal, January 2021), and the Goverdhan nala enters the Kharun at the anicut at Mahadev Ghat (as reported); each such weir or anicut takes part of the section and fixes a level upstream, so its crest is a fixed control in Clause 9.2.1 and its effect on the storm capacity of the nala is checked under Clause

8.9.5 and the HGL of Chapter 6. In Nava Raipur the drains and parks lead to a network of lakes and reservoirs, 55 of them reported, among them Sendh, Jhanjh and Khandawa (as reported), and the Development Plan keeps roads at least 0.5–1.0 m above the observed high flood level (NRDA, 2008); each lake outlet is a fixed control in the same way. At Jagdalpur the drains outfall to the Indravati, which reached a new high flood level of 544.891 m at the CWC site on 26 September 2026, about 4.1 m above its danger level of 540.80 m (CWC levels, as reported); a press figure of 4.26 m above danger on the same day was read on a local gauge with another zero, so the two series are never mixed, and the release rules of the Upper Indravati project in Odisha upstream were not found. In the east the Hirakud reservoir in Odisha backs water into the State, partly submerging 33 villages in Chhattisgarh at its full level (as reported), and in August 2020 the Mahanadi at Kalma and Surajgarh and the Mand at Kurubhata set new high flood levels of 202.43 m, 201.06 m and 220.4 m (CWC levels, as reported), so the low outfalls of Raigarh and Sarangarh districts are checked against the reservoir level as well as the river. On the plain the slopes are gentle, 0–5% across Nava Raipur (NRDA, 2008), and backwater and silting rather than velocity govern most channels; on the Bastar plateau and in the northern hills the nalas fall steeply over granite-gneiss and laterite, so drop structures (Clause 9.6.6), stepped or baffled chutes and stilling basins (Clause 9.6.5) are needed wherever the slope exceeds the limits of Clause 9.5.4, and crossings below the Bailadila range and the hill flanks of Surguja and Jashpur should be checked for boulders and debris under Clause 9.4.8 (this Manual's reading). Screens matter as much as gates, because the nalas carry silt, solid waste and sewage: in January 2021 Bilaspur was the only city in the State with a sewerage system (CECB); Raipur's drainage plan names the choking of drains, with their number and size, as the main cause of the city's waterlogging (RMC, 2015); after the floods of July 2026 Raipur's residents blamed inadequate desilting and drains narrowed by encroachment from about 7–8 ft to 4–5 ft (as reported); and water hyacinth from the Bhilai township drains blocked the culverts of the Junwani and Kosanala nalas in 2025 (as reported), so a rack on a Chhattisgarh nala is checked in the blocked case of Clause 9.8.4. In the coalfields of Korba, Raigarh and Surguja the mine sites and overburden dumps are the largest sources of sediment, and coal-mine clearances size garland drains with "at least 50% safety margin over and above the peak sudden rainfall" and ask for sumps that settle the silt (MoEFCC, clearance for Gare Palma Sector II, Raigarh district, 2024), which is not a return period. Where a drain of this Manual receives runoff from an overburden dump or a mine site, the lessee's garland drain, stable toe drain and sediment basin shall lie upstream of the public outfall, the basin holding at least the volume of Clause 8.12.3(c), with its outlet under Clause 9.7, its screen under Clause 9.8 and its spillway passing the 1% AEP inflow (Clause 9.7.6) (set by this Manual); no public drain shall discharge into an active mine, whose openings the Coal Mines Regulations 2017 require to stand at least 1.5 m above the high flood level (reg. 149, as reported); and a mine void or pit lake receives a public drain only with the consent of the lessee and of DGMS and a survey of its level, spill and highwall stability, never as storage assumed to be free. Mine dewatering at Korba draws about 7,500–13,700 m³ a day from six mines, with drawdown reaching about 1.2 km from the pits, and the Hasdeo has been found connected to the mine water through a fault (research papers of 2018 and 2023, as reported), so ground levels and grades near the pits are surveyed at design. Ash dykes are a hazard that this Manual flags and does not design. The ash ponds of the power stations at Korba have breached: the NTPC Dhanras dyke, about 420 ha, in July 2024, damaging about 16 ha of paddy, and the dyke of CSPGCL's Hasdeo Thermal Power Station near Jhabu and Nawagaon on 19 April 2026, with one death, while slurry from BALCO's ash dam no. 6 reached the Belgari nala and the Hasdeo (as reported; to be verified with CECB). An ash dyke, its decant and its toe drains are designed and kept by its owner to the standards for such works, not to this chapter; a public drain shall not discharge into an ash pond or count on its storage; and a drain, culvert or outfall downstream of a dyke is checked against the breach flow path mapped by the owner, as a hazard for its siting and for the BSMP of Chapter 11, not as a design inflow (set by this Manual). No inventory of the sluices, flap gates, interception weirs, anicuts, check dams, lake outlets and pumping stations on the drains of any Chhattisgarh city, with their capacities and operating rules, no operating rule for the gates of Gangrel, Minimata Bango, the Hasdeo barrage, Kharang or the Mongra, Arpa Bhaisajhar and Kelo works, and no design river, reservoir or mine-void level at any outfall was found in the public domain. Clauses 9.10.3 and 9.10.5 are set by this Manual because a flap gate, sluice or penstock that fails, or is jammed open by silt, solid waste or debris, lets a river, reservoir or talab in flood back into the drain network, and one jammed shut blocks it. The nalas of Chhattisgarh carry silt, solid waste and sewage to their outfalls: in January 2021 Bilaspur was the only city with a sewerage system (CECB, report to the National Green Tribunal), Raipur's sewage reaches the Kharun through its nalas, which the new treatment plants intercept with diversion weirs, Durg's sewage reaches

the Seonath through the Shankar and Pulgaon nalas (National Green Tribunal, 16 January 2026, as reported), Raipur's drainage plan names the choking of drains, with their number and size, as the main cause of the city's waterlogging (RMC, 2015), and water hyacinth from the Bhilai township drains blocked the culverts of the Junwani and Kosanala nalas in 2025 (as reported), so a gate, its screen and the interception weir beside it are exposed to the same debris and must be kept clear together; and because the Indravati at Jagdalpur rose about 4.1 m above its danger level in September 2026 (CWC levels, as reported), and releases from Gangrel, from Minimata Bango and the Darri barrage and from the barrages on the Seonath and the Arpa can hold the rivers high for days, a gate that leaks or jams can be exposed for the whole of a flood.

9.11 Inlets and gratings

9.11.1 **Hydraulics.** Inlet capacity, spacing and clogging shall follow Chapter 6, Clause 6.6.

9.11.2 **Load class.** Gratings, covers and frames shall be rated for the traffic at their location. Load classes shall be those of IS 1726 (cast iron manhole covers and frames) or EN 124 (gully tops and manhole tops), at least:

- (a) carriageways and medians: EN 124 class D400 or the equivalent heavy-duty IS class;
- (b) kerb-side channel inlets within 0.5 m of the kerb: class C250 or equivalent; and
- (c) footpaths and cycle tracks: class B125 or equivalent.

9.11.3 **Grate pattern.**

- (a) Grates in carriageways, cycle tracks and footpaths shall not have openings parallel to the direction of travel wider than 25 mm.
- (b) Grate types without tested capacity data shall be rated by the weir and orifice methods of FHWA HEC-22 (2024), with the clogging factors of Chapter 6.

9.11.4 **Kerb openings.** Kerb-opening inlets shall have a clear opening height of at least 100 mm and a length of at least 1.0 m. Where a kerb inlet discharges through the wall of a covered drain (Chapter 6, Clause 6.13.1), the opening shall be at road level.

9.11.5 **Security.** Gratings and covers on public land shall be hinged or locked to their frames, so that they cannot be removed without a tool.

9.11.6 **Inlet chambers.** Inlet chambers shall have the silt sump of Chapter 6, Clause 6.6.7, and shall be sized so that the sump can be emptied by a suction or grab machine.

> Commentary. NDMA (2010, §4.13) cites IS 5961 for cast-iron gratings for drainage inlets. The load classes are based on the EN 124 classes used internationally. The 25 mm limit on parallel openings is set for cyclist and pedestrian safety. APWA 5600 (2006, 5604.2) requires kerb openings of at least 4 ft (about 1.2 m) long and 6 in (about 150 mm) clear height. Clause 9.11.4 sets smaller minimums suited to the narrower roads and lower kerbs common in Chhattisgarh towns.

9.12 Manholes, junction chambers and access

9.12.1 **Location and spacing.** Manholes shall be provided at the locations and spacings of Chapter 6, Clause 6.7.11 and Table 6.8. Where pipes are too large for a standard manhole, a junction chamber with a riser to the surface shall be used (CPHEEO 2019, §11.3.7.3).

9.12.2 **Minimum internal size.** Manholes shall have at least the internal dimensions of Table 9.4 (CPHEEO 2019, §11.3.7.6):

Table 9.4 — Minimum internal size of manholes

Shape and depth	Minimum internal size
Rectangular, depth below 0.9 m	900 mm × 800 mm
Rectangular, depth 0.9–2.5 m	1,200 mm × 900 mm
Circular, depth above 0.9 m and up to 1.65 m	900 mm diameter
Circular, depth above 1.65 m and up to 2.30 m	1,200 mm diameter
Circular, depth above 2.30 m and up to 6.0 m	1,500 mm diameter
Circular, depth above 6.0 m and up to 9.0 m	1,800 mm diameter

The manhole shall also be large enough to fit the largest pipe connected plus the benching on each side.

9.12.3 **Access openings.** The clear access opening of manholes and access chambers on public drains shall be at least 600 mm, so that cleaning equipment can be lowered.

9.12.4 **Drops.** Where the difference between incoming and outgoing inverts exceeds 0.6 m, a drop connection shall be provided. Below 0.6 m, the difference shall be taken up by adjusting the slope of the incoming pipe or the benching (CPHEEO 2019, §11.3.7.5). The invert drop across a manhole where pipe size changes shall be at least the following fraction of the difference in diameters (CPHEEO 2019, Table 11.6):

- conduits below 400 mm: one-half;
- 400 mm to 900 mm: two-thirds;
- above 900 mm: four-fifths.

9.12.5 **Benching.** Manhole floors shall be benched to at least the crown of the outgoing pipe, with channels shaped to guide flow from each incoming pipe.

9.12.6 **Surcharge.** Manholes on mains where the 1% AEP HGL can rise above the cover (Chapter 6, Clause 6.8.4) shall have bolted, locked or hinged covers designed for that pressure.

9.12.7 **Mechanical cleaning.** Manholes, chambers and covered drains shall be designed for cleaning by machine (suction, jetting, grab or robotic equipment) from the surface. Fixed ladders shall be of corrosion-resistant, encapsulated material where provided.

> **Commentary.** The size and drop requirements are from CPHEEO (2019) Chapter 11. The 600 mm access opening, the benching rule and the bolted covers are set by this Manual. Designing for mechanical cleaning supports the prohibition on hazardous manual cleaning under *The Prohibition of Employment as Manual Scavengers and their Rehabilitation Act, 2013*.

9.13 Materials and durability

9.13.1 **Exposure classes.** Concrete structures shall be designed for the exposure class of IS 456: 2000, Table 3. In Chhattisgarh the minimum classes of Table 9.5 apply unless a site investigation shows a more severe condition:

Table 9.5 — Minimum IS 456 exposure classes for Chhattisgarh locations

Location	Minimum IS 456 exposure class
Inland drains and structures	Moderate
Drains and structures in the coastal environment, and any drain that carries or may carry sewage-mixed flow	Severe
Structures exposed to sea-water spray, or in contact with saline or aggressive groundwater	Very severe
Structures in the tidal zone	Extreme

9.13.2 **Concrete.** Minimum concrete grade, maximum water–cement ratio, minimum cement content and nominal cover to reinforcement shall be those of IS 456 for the exposure class. For the reader's convenience, IS 456 sets the minimum reinforced-concrete grades as M25 (moderate), M30 (severe), M35 (very severe) and M40 (extreme), and the nominal covers as 30, 45, 50 and 75 mm respectively.

9.13.3 **Aggressive ground.** Groundwater and soil at every structure in coastal and delta areas shall be tested for chlorides and sulphates. Cement type and concrete mix shall be chosen to suit the results under IS 456.

9.13.4 **Sewage-mixed flow.** Pipes and covered drains that carry, or may carry, sewage-mixed flow shall be protected against sulphide (crown) corrosion, by a sacrificial concrete thickness, a corrosion-resistant lining or a non-concrete pipe material.

9.13.5 **Pipes.** Concrete pipes shall comply with IS 458, with the class selected by structural design. Other pipe materials (for example ductile iron, HDPE or structured-wall plastic pipes) shall comply with the applicable Indian Standard and be approved by the approving authority.

9.13.6 **Metalwork.** Within 5 km of the coast, and in all tidal and saline locations:

- (a) fixings, hinges, ladders, handrails and screen frames shall be stainless steel grade 316, or hot-dip galvanised steel with a protective coating, or a non-metallic material; and
- (b) dissimilar metals shall be isolated from each other.

9.13.7 **Masonry.** New public drains in Tier A bodies shall be in reinforced or plain concrete. Brick or stone masonry may be used for drains in Tier B bodies only if it is plastered on the wetted face and meets the n value of Table 6.6.

> **Commentary.** The IS 456 exposure classes and the grade and cover values are quoted from IS 456: 2000 (Tables 3, 5 and 16) for convenience. The standard governs, and designers shall use its current amendment. Assigning Chhattisgarh locations to exposure classes, the sulphide-corrosion requirement and the ban on new masonry drains in Tier A bodies have no direct precedent and are set by this Manual. Drains that receive sewage cross-connections (a common condition, Chapter 8) generate hydrogen sulphide, which attacks unprotected concrete above the water line.

9.14 Submittal requirements

Every structure design shall be submitted with a **Hydraulic Structures Report** (Form 9-1) containing, as applicable:

1. culvert calculations (HDS-5 or HY-8 output) with HWi, HWo, outlet velocity, tailwater, overtopping and the blockage check;
2. channel water-surface profiles, velocities, Froude numbers and freeboard;
3. outlet protection and dissipator design, with riprap sizing;
4. detention outlet stage–discharge tables and the emergency spillway check;

5. screen and rack design with the 50% blocked check;
6. a pump station design report: inflow hydrograph, storage, pump curves and system curves, wet-well volume, controls, power supply and the Chapter 5 outfall-closed check;
7. gate and valve selection with head-loss data;
8. inlet, grate and cover load classes; and
9. an exposure class and materials statement (Clause 9.13).

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Annexes and forms

Annex / Form	Content
9A	HDS-5 inlet-control coefficients and worked culvert example in SI units
9B	Standard drawings: headwalls, riprap aprons, drop structures, trash racks, detention risers
9C	Pump station design checklist and standard control philosophy

Annex / Form	Content
Form 9-1	Hydraulic Structures Report checklist

CHAPTER 10

Submittals, Review, Operation & Maintenance and Enforcement

10.1 Purpose and scope

10.1.1 This chapter sets out how compliance with this Manual is shown, checked, built, kept working and enforced. It covers:

- (a) the submittal stages and their contents (Clauses 10.3–10.4);
- (b) the compliance calculator (Clause 10.5);
- (c) professional certification and review (Clauses 10.6–10.7);
- (d) construction inspection and as-built certification (Clauses 10.8–10.9);
- (e) maintenance declarations, O&M plans, inspection and access (Clauses 10.10–10.13);
- (f) handover of public assets (Clause 10.14);
- (g) enforcement (Clause 10.15); and
- (h) monitoring, post-event review and data publication (Clauses 10.16–10.17).

10.1.2 This chapter applies to every development and public work to which Chapters 4 and 5 apply under Clause 1.5.2. Clauses 10.3.1 (Stage 0), 10.15 and 10.17 also apply to development that is subject only to Clause 1.5.4.

> Commentary. A criterion that is not checked at approval, built as approved, and maintained afterwards has no effect. In Chhattisgarh the failure is one of missing criteria and of drains that have not kept up. No document of UADD, T&CP, a Municipal Corporation or a development authority stating a design return period, intensity or runoff coefficient for the storm drains of a city was found, except NRDA's 5-year criterion for Nava Raipur (Chapter 11). Raipur's own plan of 2015 found that the "Capacity of drain is not sufficient" and that "In rainy season most of the drains are overflowed", with drains covering 49% of the city against a benchmark of 100%, and proposed ₹1,191 crore of drainage works to reach full coverage by 2020, without stating a design basis (RMC, 2015); no source shows that target met, and the CAG's audit of AMRUT in the State (year ended March 2022) does not mention drains at all. The approval route shapes this chapter. Chhattisgarh did not replace the rules it inherited from Madhya Pradesh in 2000: it still amends the Chhattisgarh Bhumi Vikas Niyam 1984, made under s.85(1) read with s.24(3) of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973 and amended by the Housing and Environment Department, among others by notifications No. 318/1193/32/2006 of 17 February 2009 (rainwater harvesting), No. F 7-27/2017/32 of 6 October 2018 and No. F 07-24/2019/32 of 29 April 2020, none of which adds a drainage, site-level or detention rule; drafts of September and December 2025 and a reclassification of land uses in October 2026 have been reported but not read. The base and consolidated texts of the Niyam as they stand in Chhattisgarh were not found. Rule 50 is still the rule on land unsuitable for development: T&CP's report format asks whether the land is unsuitable under rule 50, whether it is waterlogged and how far it lies from a canal, nala or river, and its checklist asks, for land next to a nala, canal, river or watercourse, for the no-objection of WRD or the Revenue Department and WRD's high flood point (T&CP Directorate letters of 2011); but the 9 m and 15 m lines of the 1984 text as made in Madhya Pradesh (9 m from the highest water mark, or, by a major watercourse, 9 m from the average high flood mark or 15 m from its defined boundary, whichever is more) are not confirmed for Chhattisgarh [TBC]. Construction on the bed or edge of a reservoir or on a natural drain may not be regularised (Chhattisgarh Act of 2002 on the regularisation of unauthorised development, s.7(1), as enacted;

whether later Acts kept the bar was not checked). Development permission is given by T&CP under the Adhiniyam, and building permission by the Municipal Corporation (in Raipur, RMC), Nagar Palika Parishad or Nagar Panchayat under the Municipal Corporation Act 1956 and the Municipalities Act 1961, as adapted in Chhattisgarh; in Nava Raipur, NRDA (now the Nava Raipur Atal Nagar Vikas Pradhikaran) is a Special Area Development Authority under the same Adhiniyam, and delegated most of its powers under ss.69 to 69-ग of that Adhiniyam to its Chief Executive Officer by notification No. 4066 of 23 May 2022; and the works of State departments and local authorities need T&CP's concurrence under ss.27 and 28 (Housing and Environment Department letter of 23 November 2009). The 2018 amendment added a plinth-level check and a plinth certificate (rule 96), whose form checks location, setbacks and deviations but no level or drainage item, a site-inspection form that asks whether a major nala ("Nalah (Major Drain)") is adjacent to the plot and, if so, its nearest distance, the proposal to comply with the Niyam, and a completion form that asks whether rainwater harvesting is installed as per rule and whether the drainage line is connected to final disposal. Rainwater harvesting is required under rule 78(4), by the systems of Appendix ढ (N), and the Authority may levy a penalty of up to ₹1,000 a year per 100 m² of built-up area where a system is not built and kept up (amendment of 17 February 2009); the plot threshold is reported as 500 m² (Rajya Sabha answer of 2019), but the 2009 amendment substitutes the figures "500" and "300" in the rule without saying which is the threshold, and the rule text was not read [TBC]. Integrated townships must provide rainwater harvesting and a sewage treatment plant (Appendix ण, 2008). A draft green-building rule of 2017 (rule 79-C) would have required rainwater harvesting on plots above 100 m² and kept at least 20% of recreational open space unpaved, but its final text was not found. State buildings fall under the General Administration Department's order No. F 9-22/2016/1/5 of 31 May 2016, which makes rainwater harvesting compulsory for buildings under construction of every department and undertaking with roofs above 1,500 sq ft, and requires it on existing buildings with pucca roofs above 5,000 sq ft. The Chhattisgarh Ground Water (Management and Regulation) Act 2022 lets the State enforce rainwater harvesting and recharge in notified critical blocks and stressed urban areas, but it comes into force only on a date the State notifies, and no such notification has been found. No rule asks for a drainage report, limits peak runoff, requires on-site detention or sets a "zero runoff" condition. Every stormwater item in this chapter is therefore written so that the architect or engineer on record can certify it on Forms 1-1, 2-1, 10-2 and 10-3, and the Authority can check it with the layout at development permission, with the building plan, at the plinth-level inspection, where the distance to the nearest major nala is already recorded, and with the rainwater harvesting at completion. This chapter is the process.

10.2 Definitions

Term	Meaning in this chapter
Submittal	A set of documents lodged with the Authority at one of the stages of Clause 10.3.
Drainage report	The preliminary (Stage 1) or final (Stage 2) report required by Clause 10.4.
Stormwater facility	Any retention, detention, water quality, conveyance, pumping, gate or outfall element required by this Manual, public or private.
Hold point	A construction stage at which work shall not proceed until the Authority or the certifying professional has inspected it (Clause 10.8).
As-built certificate	The certificate of Clause 10.9 that the facility was built as approved and performs as designed.
Owner	The person or body responsible for a facility under its maintenance declaration, or the ULB for a handed-over asset.
O&M plan	The operation and maintenance plan of Clause 10.11.
Facility register	The public register of facilities of Clause 10.17.

Other terms are defined in Clause 1.4.

10.3 Submittal stages

10.3.1 Submittals shall be made in the following stages, linked to the approval steps of the Chhattisgarh Bhumi Vikas Niyam 1984 (Table 10.1):

Table 10.1 — Submittal stages and linked approvals

Stage	Submittal	When	Linked approval
0	**Pre-application** meeting and **Form 1-1** (Applicability Determination)	Before the planning permission or layout application	None (advisory record)
1	**Preliminary drainage report**	With the planning permission or layout application	Planning permission / layout approval
2	**Final drainage design**	Before building permit, or before commencement where the permit is issued with the planning permission	Building permit / commencement
3	**Construction inspections** at hold points	During construction	None (records filed)
4	**As-built certificate** and verification tests	Before the completion certificate	Completion / occupancy certificate
5	**Maintenance declaration** registered, or **handover** of public assets	Before or with the completion certificate	Completion certificate; asset acceptance
6	**Annual declarations** and inspection reports	Every year after completion	Continuing compliance

10.3.2 **Stage 0 is mandatory** for:

- (a) sites of 2 ha or more;
- (b) sites in a flood-sensitive catchment;
- (c) sites proposing a variance, waiver, regional facility or fee-in-lieu; and
- (d) sites draining to a tidal outfall or adjoining a waterbody.

For other sites, Stage 0 is optional, and Form 1-1 is lodged with Stage 1.

10.3.3 **Combined stages.** For sites of less than 1 ha outside flood-sensitive catchments, the Authority may accept Stages 1 and 2 as one submittal.

10.3.4 **Public works.** For a public work, Stage 1 is the drainage section of the DPR and Stage 2 is the drainage section of the tender drawings. The sanctioning authority shall not sanction or tender the work until the relevant stage has been reviewed under Clause 10.7.

10.3.5 **Changes after approval.** Any change that alters impervious area, levels, storage, outlet or outfall after Stage 2 shall be resubmitted, with the compliance calculator re-run, before the change is built.

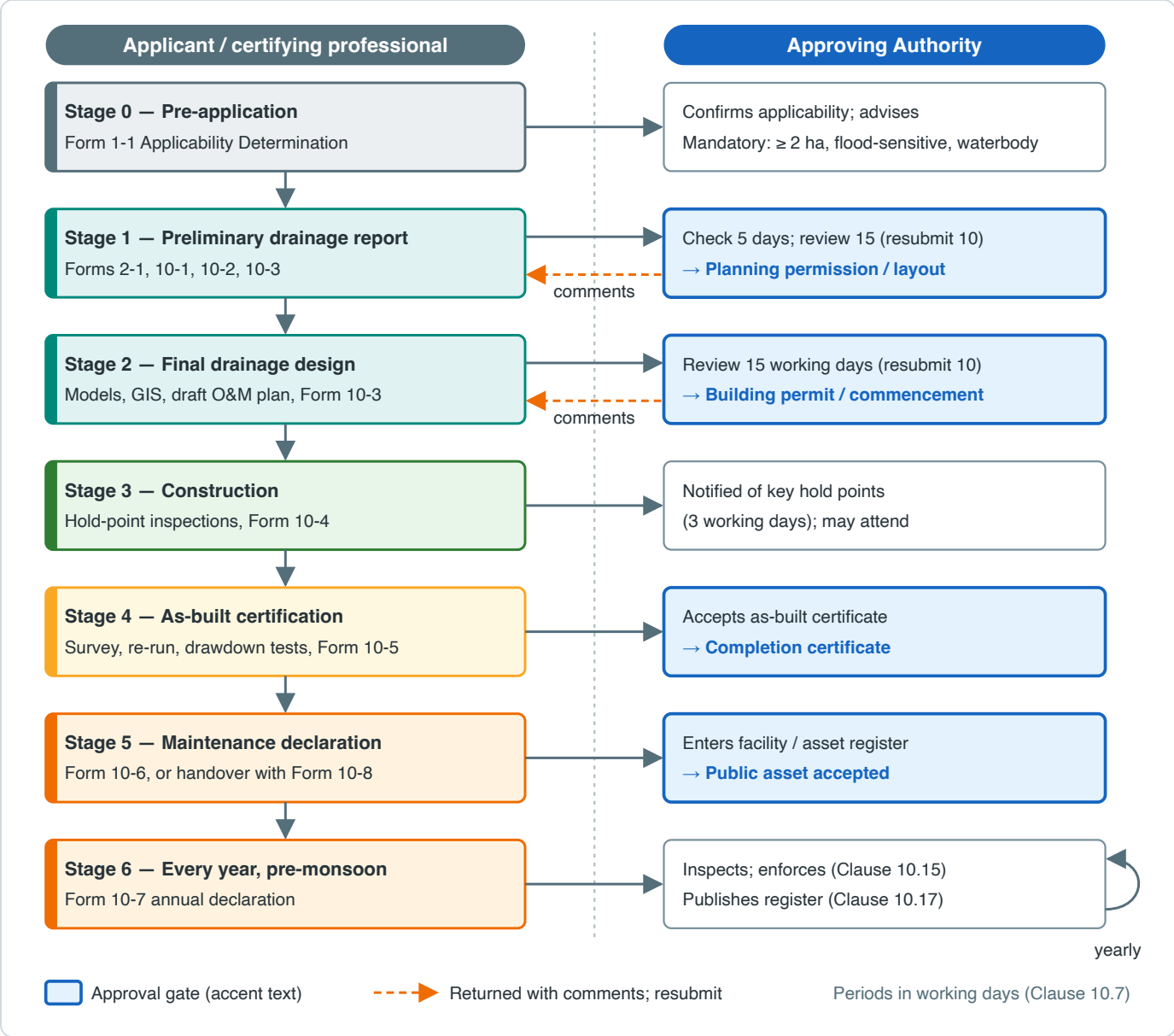

Figure 10.1 — Submittal, review and approval workflow (Clauses 10.3–10.10). Original drawing for this Manual.

> **Commentary.** The stages follow the APWA 5600 sequence of preliminary and final stormwater studies, construction inspection and maintenance agreements (APWA 5600, 2006, §5608 and §5609), mapped onto the CGBVN 1984 steps a Chhattisgarh applicant already follows. Linking Stage 4 to the completion certificate copies the MBBL 2016 RWH model, which requires inspection before the completion certificate (MBBL 2016, §9.6). The 2 ha and 1 ha limits in Clauses 10.3.2 and 10.3.3 have no direct precedent and are set by this Manual.

10.4 Required contents and checklists

10.4.1 **Preliminary drainage report (Stage 1).** The report shall contain at least:

- (a) Form 1-1 and, where applicable, Form 1-2 (Variance / Waiver Request);
- (b) a site plan with existing and proposed levels, impervious areas, and the catchment boundaries draining to and through the site;
- (c) soils (HSG), the seasonal high groundwater table, and infiltration test results or a test plan (Chapter 3);

- (d) the **Design Rainfall Sheet (Form 2-1)**, completed as Clause 2.13 requires;
- (e) the pre-development condition and hydrologic method (Chapter 3);
- (f) how the hierarchy of Clause 1.2 has been applied: areas preserved, retention method, detention concept and conveyance;
- (g) preliminary retention, water quality and detention volumes, and release rates, from the compliance calculator (Clause 10.5);
- (h) the downstream extent and tailwater case (Chapter 5);
- (i) waterbodies, setbacks and the 1% AEP level (Chapters 6 and 7); and
- (j) the land reserved for facilities and maintenance access (Clause 10.13).

10.4.2 **Final drainage design (Stage 2).** The final design shall contain, in addition to updated Stage 1 contents:

- (a) the final calculations for every storage duration of Clause 2.5.2 and every AEP required by Chapter 4, including the outfall-closed check of Chapter 5;
- (b) stage–storage–discharge tables for each facility, and the emergency spillway design (Chapter 4);
- (c) hydraulic design of conveyance elements, inlets and structures (Chapters 6 and 9);
- (d) the water quality design and the first-flush or gross pollutant trap where required (Chapter 8);
- (e) construction drawings, details and specifications of every facility;
- (f) the construction sequence, including protection of retention and infiltration areas from sediment and compaction (Clause 10.8.3);
- (g) the hold-point schedule (Clause 10.8);
- (h) the draft O&M plan (Clause 10.11) and draft maintenance declaration (Clause 10.10);
- (i) the model input files and the GIS data of Clause 10.4.4; and
- (j) the certificates of Clause 10.6.

10.4.3 **Checklist.** Each submittal shall include the **Drainage Report Checklist (Form 10-1)**, showing for each item of Clauses 10.4.1–10.4.2 the page or sheet where it is addressed, or the reason it does not apply. The Authority shall not start technical review of a submittal without a completed Form 10-1.

10.4.4 **Digital submittal.**

- (a) Reports and drawings shall be submitted as searchable PDF.
- (b) Hydrologic and hydraulic models shall be submitted in their native, runnable format (for example SWMM .inp, HEC-HMS or HEC-RAS project files), with the version of the software used.
- (c) Facilities, outfalls, flow paths and catchment boundaries shall be submitted as GIS data (GeoPackage or shapefile) in WGS 84 (EPSG:4326) or the applicable UTM zone (44N statewide, including Jashpur Nagar and the eastern part of Jashpur district, which lie in zone 45N), with attributes as listed in Annex 10B.
- (d) Levels shall be stated relative to a named vertical datum. The relationship to the river datums of the CWC gauges on the receiving rivers, of WRD's dam, barrage, reservoir and tank levels and of the levels of the lakes and reservoirs of Nava Raipur, and, for an outfall to a mine void, of the lessee's mine survey used in Chapter 5 shall be stated.

10.4.5 **Summary of forms used in submittals** (Table 10.2):

Table 10.2 — Forms used in submittals

Form	Title	Stage
1-1	Applicability Determination	0 or 1
1-2	Variance / Waiver Request	1
2-1	Design Rainfall Sheet	1, 2
10-1	Drainage Report Checklist	1, 2
10-2	Compliance Calculator Summary	1, 2, 4

Form	Title	Stage
10-3	Professional Design Certificate	1, 2
10-4	Hold-Point Inspection Record	3
10-5	As-built Certificate	4
10-6	Maintenance Declaration	5
10-7	Annual Maintenance Declaration and Inspection Report	6
10-8	Asset Handover Certificate	5

Commentary. Runnable models and GIS data cost the applicant little and let the Authority check the work, and later build a city-wide picture of private storage. Chhattisgarh has no notified design return period or design storm for its urban drains. The only explicit urban design storm in the State is in the Naya Raipur Development Plan-2031 (NRDA, 2008), which says that "5 years Maximum rainfall intensity should be taken for design of system" (§10.3.1.1), asks that any river embankment be safe against a 100-year flood, keeps roads 0.5–1.0 m above the observed high flood level, follows IRC SP-50 for road drainage, and gives a chart of rainfall intensity against time of concentration for 2, 5, 10 and 50 years prepared from IMD data for the Raipur region (Figure 10.1 of the Plan), with no equation, record length or method; it is a plan for one city, not a State standard. Raipur's storm drainage plan under AMRUT (RMC, 2015) records coverage of 49%, about 480 km of pucca drains and 11 road stretches that flood two or three times a year, and proposes ₹1,191 crore of works, but states no return period, intensity or runoff coefficient; and AMRUT in the State funded water supply, sewerage and septage and green spaces, so the CAG's audit of it (year ended March 2022) does not mention drains at all. The design basis of the Raipur Smart City drainage works and of the drains of Bilaspur, Durg, Bhilai, Korba and the Bhilai Steel Plant township was not found. These figures are history, not a design basis. IMD's national IDF study (Das et al. 2022) has no Chhattisgarh station, the constants for Raipur in an older IMD study (Ram Babu et al., 1979) have been seen only in a search snippet and are unverified, and no station IDF with usable constants has been published for any Chhattisgarh city; the NRDA chart can be read only by eye. The short bursts on record, 79 mm in an hour at Kanker (26 June 1974) and 80 mm at Jagdalpur (CWC), lie below the 1-hour values that the CWC ratios give, while the 24-hour records, 370.3 mm at Raipur on 4 August 1910 and 392.0 mm at Rajnandgaon on 21 September 1965 among them, exceed the 100-year depths of the CWC plates (IMD; this Manual's reading). The State's design values are therefore those of the CWC Flood Estimation Reports for subzones 3(d), 3(g), 3(f), 1(d) and 3(c), with the interim provisions of Chapter 2, and each submission must state its own design basis. Form 2-1 and Form 10-2 ensure that every design states its inputs.

10.5 Compliance calculator

10.5.1 The State Stormwater Technical Committee shall issue and maintain a **compliance calculator**: a version-controlled spreadsheet or web tool that, for a given site:

- (a) applies the applicability tests of Clause 1.5;
- (b) computes the retention volume from the Annex 2E depth, and the water quality volume (Chapter 8);
- (c) applies the credits for preserved areas and retained volume against detention (Chapter 4);
- (d) on the design-aid route (Clause 1.7, S-1), gives retention and detention volumes from the unit-storage tables;
- (e) on alternative S-2, gives the permissible site discharge for each AEP; and
- (f) produces the **Compliance Calculator Summary (Form 10-2)**.

10.5.2 **Use.**

- (a) Every Stage 1, 2 and 4 submittal shall include Form 10-2 from the current version of the calculator.

- (b) On the design-aid route, the calculator may be the only storage calculation.
- (c) Outside the design-aid route, the calculator is a summary and a screening check. It does not replace the hydrograph analysis of Chapter 3. Where a model result differs from the calculator screen by more than the tolerance stated in the calculator guide, the report shall explain why.

10.5.3 **Version control.** The calculator shall record its version in every output. A new version applies to new submittals 6 months after release, in line with Clause 2.14.2. A submittal shall use one version throughout Stages 1 to 4, unless the Applicant elects a later one.

10.5.4 **Openness.** The calculator, its user guide, and the source of every table in it shall be published free of charge.

> Commentary. The Mid-America Regional Council issued a Compliance Calculator (.xlsm) and a Calculator Start-Up Guide with APWA 5600 Version 2025.01, together with a site activity plan template and the Policy Supplement (MARC stormwater management page, 2025). The calculator turns a layered standard with credits into one consistent set of numbers that a reviewer can check quickly. Singapore achieves a similar effect with a much simpler rule: a single allowable C value of 0.55 for 10-yr storms up to 4 h, applied by formula (PUB Code of Practice, cl. 7.1.5). A Chhattisgarh calculator serves Tier B authorities in particular, where it may be the whole calculation.

10.6 Professional certification

10.6.1 **Who may sign.** Drainage reports and designs shall be prepared and signed by a registered professional (Clause 1.4) competent in hydrology and hydraulics. For any of the following, the signatory shall be a civil engineer **empanelled as a Stormwater Design Professional** by the Authority or T&CP:

- (a) Tier A sites of 1 ha or more;
- (b) any site in a flood-sensitive catchment;
- (c) any facility with an embankment, or a high-hazard basin checked for the 0.2% AEP (Chapter 4); and
- (d) any pumped facility, tide gate or regional facility.

10.6.2 **Empanelment criteria** shall be issued by T&CP and shall include a civil engineering degree, relevant experience in drainage design, and completion of a training course on this Manual.

10.6.3 **Certificate.** The signatory shall complete the **Professional Design Certificate (Form 10-3)**, certifying that:

- (a) the design complies with this Manual, except for variances listed and approved;
- (b) the rainfall, soils and tailwater inputs are as recorded in Forms 2-1 and 10-2; and
- (c) the models submitted reproduce the results reported.

10.6.4 **Independent check.** Facilities under Clause 10.6.1(c) and (d) shall be checked by a second empanelled professional who is independent of the designer. That professional shall sign Form 10-3 as checker.

10.6.5 **False certification.** A false or negligent certificate is an offence under Clause 10.15.

> Commentary. Singapore requires that its ABC Waters design features used for peak control be endorsed by an ABC Waters Professional who is also a Professional Engineer (Civil) (PUB Detention Tank Guide, 2024). India has no statutory register of professional engineers comparable to Singapore's, so this clause uses the CGBVN 1984 registration and adds empanelment for the higher-risk cases. The 1 ha limit is set by this Manual.

10.7 Review procedure and timelines

10.7.1 **Completeness check.** The Authority shall check each submittal against Form 10-1 within **5 working days** and either accept it for review or return it with a list of missing items.

10.7.2 **Technical review.** The Authority shall complete technical review and issue approval, conditional approval or a single consolidated list of comments within:

- (a) **15 working days** of acceptance for a first submittal; and
- (b) **10 working days** of acceptance for a resubmittal.

10.7.3 **No new comments.** On resubmittal, the Authority shall not raise new comments on matters that were unchanged and not commented on in the earlier review, unless the matter affects safety.

10.7.4 **Review capacity.** A Tier B authority, or a Tier A authority for overflow work, may use an empanelled reviewer from the T&CP list. The empanelled reviewer shall not have worked on the same site. The Authority remains the decision-maker.

10.7.5 **Concurrent clearances.** Where the permission of WRD for a work on, or a discharge to, one of its rivers, dams, barrages, reservoirs, tanks or canals, among them Gangrel, Minimata Bango, the Hasdeo barrage at Darri, the Mongra and Arpa Bhaisajhar barrages, Kharang, the Kelo dam and Kopra Jalashay, the permission of the Commissioner or Chief Municipal Officer to connect to a municipal drain or to build over one (Municipal Corporation Act 1956 and Municipalities Act 1961, as adapted in Chhattisgarh), the concurrence of T&CP that a work of a State department or local authority needs (Nagar Tatha Gram Nivesh Adhiniyam 1973, ss.27 and 28), the views of the Chhattisgarh State Wetland Authority on a work within 50 m of the highest flood level of a talab or wetland that it treats as protected, and its approval for a regulated activity in a notified wetland or at Kopra Jalashay, the consent of the Collector for a work on a tank vested in the State (Land Revenue Code 1959, s.251), the consent of the lessee and of DGMS that this Manual requires for any discharge to a mine void or pit lake or for a drain that crosses a mining lease, the consent of the body that owns or runs the receiving drain where that body is not the Authority (for example NRDA in Nava Raipur, or Bhilai Steel Plant in its township), or another agency's concurrence is required (Chapter 7), the Authority shall forward the submittal on the day it is accepted for review, and the review periods shall run concurrently.

10.7.6 **Fit with permit timelines.** The periods in Clauses 10.7.1–10.7.2 run within, and do not extend, the overall time limits for planning permission and building permits set by the Government.

10.7.7 **Reasons.** Every refusal or condition shall state the clause of this Manual on which it is based.

> Commentary. The review periods have no direct precedent and are set by this Manual. A single consolidated comment list and the "no new comments" rule follow common good practice for engineering review.

10.8 Construction inspection

10.8.1 **Hold points.** The approved hold-point schedule (Form 10-4) shall include at least:

- (a) the excavated base of each retention, infiltration and detention facility, before any media, liner or structure is placed;
- (b) the outlet structure, orifice and trash screen, before backfill or before the facility is first allowed to fill;
- (c) the infiltration tests of Clause 10.9.3;
- (d) the outfall, flap or tide gate and non-return valve, before the downstream connection is made; and
- (e) pumps and controls, at commissioning.

10.8.2 Each hold point shall be inspected by the certifying professional and recorded on Form 10-4 with dated, geotagged photographs. For sites under Clause 10.6.1, the Authority shall be given at least 3 working days' notice of hold points (a) and (b), and may attend.

10.8.3 **Protection during construction.**

- (a) Retention and infiltration areas shall be fenced off from construction traffic and stockpiles.
- (b) Infiltration facilities shall not be brought into service until the area draining to them is stabilised.
- (c) Sediment shall be removed from any facility used for temporary sediment control before the as-built survey.

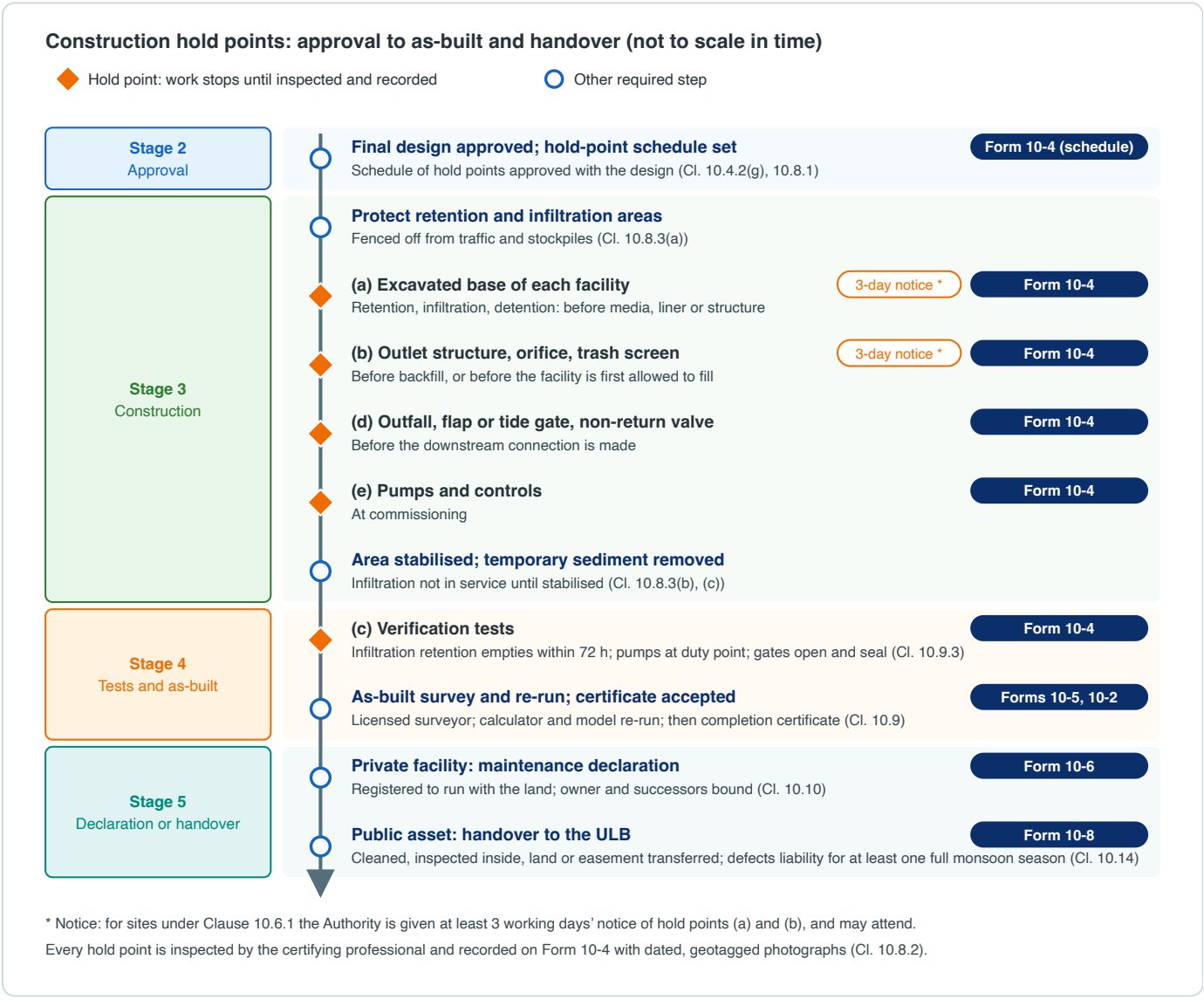

Figure 10.2 — Construction hold points from approval to as-built certification and handover, with the form used at each step (Clauses 10.8–10.10 and 10.14). Hold points are lettered as in Clause 10.8.1. Original drawing for this Manual, adapted from the construction-inspection and maintenance-agreement sequence of APWA Section 5600 (2006), §5608.

> **Commentary.** Infiltration facilities fail most often in construction, when they are compacted or filled with silt before they ever operate. Hold points (a) and (c) target that failure. The 3-day notice is set by this Manual.

10.9 As-built certification

10.9.1 Before the completion certificate is issued, the certifying professional shall submit the **As-built Certificate (Form 10-5)** with:

- (a) an as-built survey by a licensed surveyor of each facility's levels, storage volume, outlet dimensions and inverts, and the outfall;
- (b) a re-run of the compliance calculator, and of the model where one was used, with as-built values;
- (c) the completed hold-point records (Form 10-4); and
- (d) the final O&M plan and the executed maintenance declaration or handover documents (Clauses 10.10 and 10.14).

10.9.2 **Acceptance.** The as-built facility is accepted where:

- (a) the as-built storage volume at each design level is not less than the volume required by Chapter 4;
- (b) the outlet dimensions and inverts match the approved design; and
- (c) the re-run shows compliance with every criterion of Chapters 4 and 5.

Any deviation shall be corrected, or re-certified as compliant under Clause 10.3.5.

10.9.3 **Verification tests.** Every retention facility that relies on infiltration shall be tested after construction to show that it empties within **72 hours**, as Chapter 4 requires. Pumped facilities shall be tested at their design duty point. Tide gates and non-return valves shall be shown to open and seal.

10.9.4 The Authority shall not issue the completion certificate until Form 10-5 is accepted.

> Commentary. The 72-hour retention drawdown is the criterion of Chapter 4. This Manual requires it to be tested after construction, rather than relying on pre-construction tests alone, because compaction during construction can cut infiltration rates far below the design values.

10.10 Maintenance declarations and covenants

10.10.1 **Declaration.** The owner of every private stormwater facility shall execute a **Maintenance Declaration (Form 10-6)** before the completion certificate is issued. The declaration shall:

- (a) identify each facility, its location and the approved O&M plan;
- (b) bind the owner and successors in title to operate, inspect and maintain the facility under the O&M plan;
- (c) grant the Authority a right of entry to inspect, and to carry out works in default under Clause 10.15;
- (d) prohibit removal, filling, covering, reduction or change of use of the facility without the Authority's approval; and
- (e) be registered so that it runs with the land and is disclosed to purchasers.

10.10.2 **Multi-owner developments.** For apartments, layouts and gated communities, the declaration shall name the association or body responsible after handover by the developer. The developer remains responsible until that body is legally constituted and has accepted the declaration in writing.

10.10.3 **Annual declaration.** Every owner of a facility in a Tier A area, and of any pumped facility, tide gate or regional facility anywhere, shall submit an **Annual Maintenance Declaration and Inspection Report (Form 10-7)** by **31 May** each year. The report shall be signed by the owner and, for facilities under Clause 10.6.1(c) and (d), by an empanelled professional. It shall state that the pre-monsoon inspection of Clause 10.12 has been done and any defects corrected.

10.10.4 **Transfer.** On sale or transfer, the seller shall disclose the declaration and the latest Form 10-7. The purchaser takes on the obligations from the date of transfer.

> Commentary. Singapore requires owners of pumped detention systems to file yearly declarations and documents with PUB (PUB Detention Tank Guide, citing Code of Practice §13.2). Addendum No. 3 to the Code of Practice (16 Apr 2025) introduced annual maintenance declarations for flood barriers and pumped detention tanks, following amendments to the Sewerage and Drainage Act. APWA 5600 (2006, §5608) required easements and maintenance provisions for detention facilities. A registered declaration matters in Chhattisgarh because detention tanks under podiums and in basements are easy to convert to parking or storage once the building is occupied. The 31 May date is set by this Manual to fall before the south-west monsoon (June–September), when most Chhattisgarh extreme rainfall occurs.

10.11 O&M plans

10.11.1 Every stormwater facility shall have an O&M plan, approved at Stage 2 and finalised at Stage 4. The plan shall include:

- (a) an inventory and a plan drawing of each facility, with design levels, volumes and outlet details;
- (b) routine tasks and their frequency: inspection, debris and sediment removal, vegetation care, screen cleaning;
- (c) the trigger for sediment removal, stated as a depth or a percentage of design volume;
- (d) inspection checklists matching Form 10-7;
- (e) for pumped systems, gates and valves: the operating rules, test schedule, standby power, and actions on a cyclone or heavy-rainfall warning;
- (f) actions to prevent standing water and mosquito breeding, consistent with the drawdown times of Chapter 4;
- (g) safety measures for confined spaces, deep water and public access;
- (h) access routes and equipment needed (Clause 10.13);
- (i) an annual cost estimate and who pays it; and
- (j) the records to be kept, and for how long.

10.11.2 **Public facilities.** For facilities owned by a ULB or department, the O&M plan shall be included in the asset register (Clause 10.14.3) and funded in the annual maintenance budget.

> Commentary. Chhattisgarh has no State order fixing a date for pre-monsoon desilting: no order of UADD, RMC or CGSDMA with a date was found, and no State disaster management plan or urban flood plan of CGSDMA was found. After the Raipur floods of July 2026 residents blamed inadequate desilting and drains narrowed by encroachment, and RMC's Commissioner was reported angered by garbage-choked drains (as reported). The drains are run by several hands: the Municipal Corporations, Nagar Palika Parishads and Nagar Panchayats for the municipal drains; NRDA in Nava Raipur; Bhilai Steel Plant's Town Services in its township, which the Commissioner of Bhilai Municipal Corporation warned in July 2025 over uncleaned township drains after a 15 June deadline was missed (as reported); WRD for the rivers, dams and barrages; and the coal companies on their leases, whose clearance conditions require garland drains and sumps to be desilted regularly (MoEFCC, clearance for Gare Palma Sector II, 2024). The rivers are run by dam operation: Gangrel's release of October 2025 was preceded by alerts to 72 riverside villages (as reported); the gates of Hirakud in Odisha were opened at Chhattisgarh's request in August 2020 and September 2024, and in August 2022 the State was reported to have opened its barrages on the Mahanadi before informing Odisha (as reported). No operating rule for any sluice, flap gate, interception weir or pumping station in a Chhattisgarh town, linking it to a river stage, a dam or barrage release or a rainfall warning, was found in the public domain. Every gate and pump in this Manual needs a written rule for what happens when a warning is issued.

10.12 Inspection frequencies

10.12.1 **Owner inspections.** Owners shall inspect facilities at least as set out in Table 10.3:

Table 10.3 — Minimum owner inspection frequencies

Facility type	Routine	Pre-monsoon (by 31 May; 25 May in Bastar)	After a heavy-rainfall event
Retention / infiltration (soakaways, trenches, bioretention, permeable paving)	Every 6 months	Yes	Yes
Surface detention basin or multi-use open space area	Every 6 months	Yes	Yes
Underground or podium detention tank, gravity outlet	Every 6 months	Yes	Yes
Pumped detention, pumps and controls	Monthly test run	Yes, with full duty test	Yes
Flap / tide gate, non-return valve	Every 3 months	Yes, with seal test	Yes
Gross pollutant trap / first-flush device	Every 3 months	Yes	Yes
Private drains and inlets	Every 6 months	Yes	—

A heavy-rainfall event, for this Clause, is a day on which the nearest approved gauge records rainfall at or above the IMD "heavy rainfall" category, or a day for which a red alert is issued for the district. In Chhattisgarh, the pre-monsoon inspection of sluices, flap gates, non-return valves and pumped facilities, including the sluices and gates on outfalls to reservoirs, barrage ponds and talabs and on outfalls that the Kharun, the Seonath, the Arpa, the Hasdeo, the Kelo, the Indravati or the Hirakud backwater can close, with the pump duty and gate seal tests, shall be completed by **31 May**, and in the Bastar division by **25 May**; these facilities, with their standby generators, shall be certified ready by **10 June**, the normal onset date used by IMD Raipur and before the new normal onsets of 11–17 June, with every gate on an outfall to a river or reservoir operated through its full travel. Pumps, their standby power and the gates shall then be kept on monsoon standby, with the monthly test run, until **20 October**, and in the Bastar division until **31 October**, after the normal withdrawal of 7–15 October, and at an outfall to a river regulated by a dam or barrage upstream until the releases from the dam have ended and the river at the governing gauge has fallen below its danger level where one is published, if that is later.

10.12.2 **Authority inspections.** The Authority shall inspect:

- (a) every facility on a site of 1,000 m² or more, or under Clause 10.6.1(c) or (d), at least once every 2 years, and every year in a flood-sensitive catchment; and
- (b) a random sample of at least 10% of other facilities each year.

10.12.3 The results of Authority inspections shall be entered in the facility register (Clause 10.17).

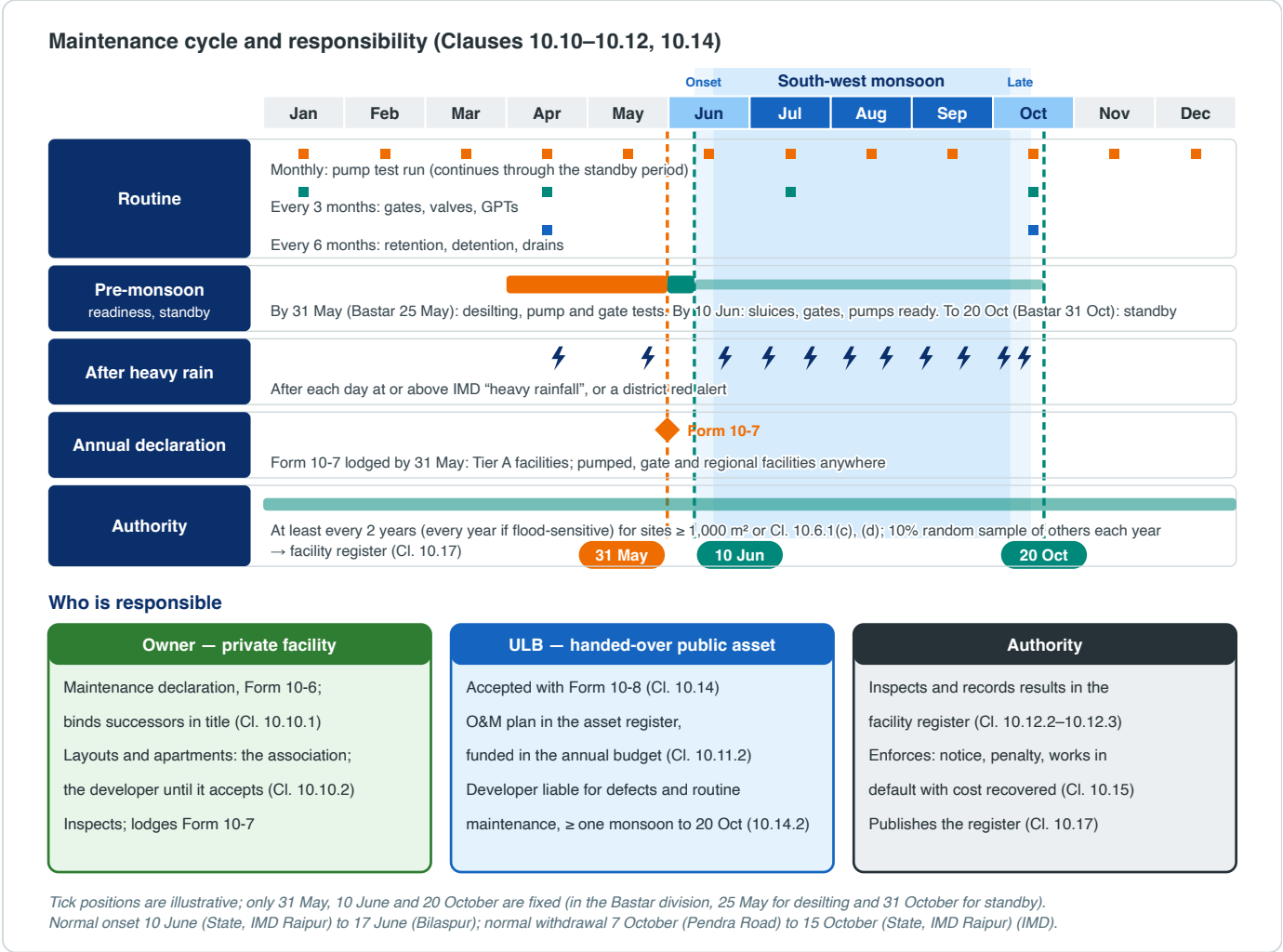

Figure 10.3 — Annual maintenance cycle and who is responsible: routine, pre-monsoon (by 31 May) and post-event inspections, the annual declaration (Form 10-7) and Authority inspections (Clauses 10.10–10.12 and 10.14). Original drawing for this Manual. The timing follows the south-west monsoon (June–September); tick positions within the year are illustrative.

Commentary. The split in Clause 10.12.2 follows the MBBL 2016 inspection regime for RWH, which requires 100% monitoring of buildings over 1,000 m² and a 10% random check below that (MBBL 2016, §9.6). The inspection intervals in Table 10.3 are set by this Manual. The 31 May deadline is set by this Manual for the onset of the south-west monsoon in the second and third weeks of June: IMD Raipur uses 10 June as the normal onset for the State, and the new normals are 11–13 June at Jagdalpur, 14 June at Kondagaon, 16 June at Raipur and Pendra Road and 17 June at Bilaspur (IMD, 1961–2019 normals); the monsoon set in on 12 June in 2020 and entered the State through Sukma on 8 June in 2024 (IMD Raipur). Large falls come in June: 365.0 mm at Jashpurnagar on 30 June 2001, 337.0 mm at Durg on 15 June 2004 and 297.0 mm at Raipur on 30 June 2007 (IMD). The south of the State is wetter earlier, with the onset at Jagdalpur from 11 June and 207.0 mm at Bailadila on 11 May 1990 (IMD), so this Manual sets 25 May in the Bastar division. No State or municipal deadline for desilting was found; this Manual takes 31 May for desilting, the pre-monsoon inspection and the annual declaration. NDMA (2010) sets 31 March for pre-monsoon desilting nationally; this Manual allows until 31 May so that dry-season work can be finished. Sluices, flap gates, pumps and their standby generators must in addition be certified ready by 10 June, the earliest normal onset. The normal withdrawal runs from 7 October at Pendra Road and 9 October at Raipur to 12–14 October at Jagdalpur (IMD, 1971–2019 normals), and IMD Raipur uses 15 October; 21 of the 100 storms and depressions that affected the State from 1891 to 2014 came in October, on more southerly tracks across Bastar, and October storms are on record, 228.1 mm at Pendra Road on 8 October 1994, 206.0 mm at Ambikapur on 1 October 1961 and 148.6 mm at Raipur on 8 October 1900 (IMD); pumps and gates are therefore kept on

standby to 20 October, and to 31 October in the Bastar division (Figure 10.3), and at an outfall to a river regulated by a dam or barrage until the releases have ended and the river has fallen below its danger level, if that is later. The post-event inspections of Table 10.3 cover storms at any time of year.

10.13 Maintenance access

10.13.1 Every facility shall have permanent access for inspection and maintenance, shown on the approved drawings and protected by the maintenance declaration or, for public facilities, by an easement or land transfer.

10.13.2 **Surface facilities.** Basins, ponds and open channels shall have a vehicle access route to the inlet, the outlet and the base, and a maintenance strip along at least one side. Ramps into basins shall be suitable for the plant needed to remove sediment.

10.13.3 **Underground facilities.** Tanks shall have:

- (a) an access opening over the inlet, the outlet and each chamber;
- (b) fixed ladders or steps, and ventilation for confined-space entry;
- (c) a sump or low point for sediment removal; and
- (d) headroom sufficient for manual cleaning.

10.13.4 **Outlets.** Orifices and screens shall be reachable for cleaning without entering a full facility.

10.13.5 Access routes shall not be built over, fenced off or used for parking in a way that prevents access.

> Commentary. APWA 5600 (2006, §5608) required a 15-ft (about 4.5 m) maintenance access strip for detention facilities. Plot sizes in Chhattisgarh cities make a fixed width hard to apply to every site, so this Manual sets functional requirements. Annex 10C covers maintenance access dimensions.

10.14 Handover of public assets to the ULB

10.14.1 Drains, roads, open space facilities, regional facilities and other stormwater assets that a developer builds for transfer to a ULB shall be handed over only when:

- (a) the as-built certificate (Clause 10.9) has been accepted;
- (b) the facility has been cleaned, and pipes and culverts have been inspected internally (for example by CCTV or manual entry), with the record handed over;
- (c) the land, or an easement over it, has been transferred to the ULB;
- (d) the O&M plan, drawings, GIS data and warranties have been handed over; and
- (e) the **Asset Handover Certificate (Form 10-8)** has been signed by both parties.

10.14.2 **Defects liability.** The developer shall remain responsible for defects and for routine maintenance for a defects liability period that includes at least **one full monsoon season, from 10 June to 20 October (31 October in the Bastar division),** after handover. The ULB may hold a security deposit for this period.

10.14.3 **Asset register.** On acceptance, the ULB shall enter the asset in its GIS drainage asset register and in the facility register of Clause 10.17.

10.14.4 **Private facilities.** A private facility remains the owner's responsibility under its maintenance declaration unless the ULB formally accepts it under this Clause.

> **Commentary.** NDMA (2010) calls for a GIS inventory of drains. Requiring the developer's GIS data at handover builds that inventory as the city grows, at no cost to the ULB. The defects period spans one monsoon because many defects in Chhattisgarh drainage show up only in the first SW monsoon.

10.15 Enforcement

10.15.1 **Offences.** The following are contraventions of this Manual:

- (a) building without an approved drainage design where one is required;
- (b) building a facility other than as approved;
- (c) removing, filling, covering, reducing or changing the use of a facility without approval;
- (d) failing to maintain a facility, or to submit Form 10-7;
- (e) a sewage connection to a storm drain;
- (f) building or filling in a waterbody setback or no-fill zone (Chapter 7);
- (g) obstructing a natural drainage path or major-system flow path; and
- (h) false or negligent certification.

10.15.2 **Graduated response.** The Authority shall respond in the following order, moving to a later step where an earlier one fails or where the contravention is serious (Table 10.4):

Table 10.4 — Graduated enforcement response

Step	Action
1	Written notice stating the contravention, the clause breached, the remedy and a compliance period.
2	Stop-work order, or withholding of the completion certificate, for development not yet complete.
3	Penalty under the applicable Act or rules.
4	Works in default by the Authority, with the cost recovered from the owner.
5	Disconnection of the private drain connection to the public system, where this causes no hazard to others.
6	Report of the certifying professional to the registering authority, and suspension from the empanelled list.

10.15.3 **Immediate action.** Where a contravention puts life or property at risk during a monsoon, the Authority may carry out works in default at once and recover the cost later.

10.15.4 **Security deposits.** For sites under Clause 10.6.1, the Authority may require a security deposit or bank guarantee, released after the as-built certificate is accepted, to cover works in default.

10.15.5 **Record.** Every notice and action under this Clause shall be entered in the facility register.

> **Commentary.** Penalties are referred to generically. This Manual cannot itself create offences or fines, and the powers come from the Acts under which the Chhattisgarh Bhumi Vikas Niyam 1984 are made and from the municipal Acts. The graduated steps are set by this Manual. In Chhattisgarh the courts, the National Green Tribunal and the State Wetland Authority have become the working regulators of the rivers, tanks and nalas. The Tribunal's Central Zone bench directed the Collector of Janjgir-Champa to remove encroachments from ponds recorded in the land records at Baloda (order of 14 November 2024 in O.A. 78/2024), and the Tahsildar proceeded against the encroachers under s.248 of the Chhattisgarh Land Revenue Code 1959 (report of 27 December 2024); in January 2026 it ordered that the treatment plants for Durg's sewage, carried to the Seonath by the Shankar and Pulgaon nalas, be finished within six months (as reported). The High Court, hearing the pollution of the Arpa,

recorded that about 40% of Bilaspur's sewage would remain untreated after the new plants (January 2025, as reported), and in May 2026 called the pollution of the Kharun, the Seonath and the Arpa a "systematic failure" (as reported); after its orders of 22 January 2019 in WPC 25/2016 and 2696/2016, the State decided that the permission of the Arpa Special Area Development Authority would no longer be needed to sell, build on or develop land in Bilaspur (Housing and Environment Department letter of 14 February 2019). The State Wetland Authority halted works at Karbala Talab in Raipur within 50 m of its highest flood level (September 2025), ordered a probe into a toy-train plan in the buffer of Narhaiya Talab (2025) and directed NRDA to withdraw its tenders for plots beside the Jhanjh wetland in Nava Raipur (March 2025) (as reported). Rainwater harvesting has been enforced case by case: 18 builders and 7 industrial units in Raipur district faced action for not building it (2016), and RMC directed builders to provide at least 50 recharge pits in each township (as reported). The statutes carry real powers: the Authority may levy up to ₹1,000 a year per 100 m² of built-up area on an owner who does not build and keep up rainwater harvesting (Bhumi Vikas Niyam, rule 78(4), as amended in 2009); construction on the bed or edge of a reservoir or on a natural drain may not be regularised (regularisation Act 2002, s.7(1)); tanks recorded for nistar vest in the State (Land Revenue Code, s.251), and unauthorised occupation of them is met by summary eviction (s.248); and the Municipal Corporation and Municipalities Acts, as adapted in Chhattisgarh, carry the drainage and tank powers of the Madhya Pradesh Acts from which they come (their Chhattisgarh texts were not read). This shows that a rule without routine enforcement does little, and that where the State has not drawn a line the courts, the Tribunal and the Wetland Authority have. Professional accountability (step 6) is often more effective than fines on owners, because it acts on the person who signs the design.

10.16 Monitoring and post-event review

10.16.1 **Monitoring network.** Tier A authorities shall maintain, or have access to, recording rain gauges and water-level gauges at their main outfalls and regional facilities. Data shall be archived at intervals of 15 minutes or less.

10.16.2 **Event reports.** After any event that floods buildings within a ULB area, the ULB shall prepare a short event report within **3 months**, recording:

- (a) rainfall at the nearest approved gauges, by duration, compared with the design depths of Chapter 2;
- (b) the level of the receiving river (the Kharun at Pathardihi, the Seonath, the Arpa, the Hasdeo at Bangodam or Katghora, the Kelo, the Mahanadi at Rajim, Kalma or Surajgarh, or the Indravati at the CWC site at Jagdalpur) or of the receiving reservoir, barrage pond or talab, and, where it is gauged, at the outfall, each on a named datum, with any reading of a local gauge kept apart from the CWC levels; the state of every sluice, flap gate, interception weir and pumping station on the outfall and on the drain upstream of it, with the hours each pump ran and any loss of power; and the gate openings and releases of any dam or barrage that sets the receiving level, such as Gangrel, Minimata Bango, the Hasdeo barrage at Darri, the Mongra or Arpa Bhaisajhar barrage, Kharang or the Kelo dam, with the level of the Hirakud reservoir where its backwater reaches the outfall;
- (c) flooded locations, depths and durations, with flood marks surveyed where possible;
- (d) the performance of public and inspected private facilities, including any blocked outlets, failed pumps or closed gates; and
- (e) recommended actions.

10.16.3 **Post-event review.** Where an event exceeds the 1% AEP 24-hour or 72-hour design depth at two or more approved gauges in the State (Clause 2.14.1), CGSDMA, with the State Stormwater Technical Committee, shall lead a post-event review within **12 months**. It shall assess whether the criteria of this Manual were adequate, and shall feed into the review of Clause 1.12.

10.16.4 **Facility performance sampling.** The Authority shall, at least every 5 years, monitor a sample of private retention and detention facilities through a monsoon, to test whether they perform as designed.

Commentary. The post-event trigger is the review trigger already set in Clause 2.14.1. Chhattisgarh's floods have been recorded by IMD, CWC and the press rather than examined by technical reviews. Raipur's record day, 370.3 mm on 4 August 1910, is the highest in the observatory's record since 1901 (IMD); on 30 June 2007 Raipur recorded 297.0 mm and more than 6,000 people were moved from its low-lying areas (IMD; as reported). On 28–29 August 2020 a Bay depression brought 282.7 mm to Akaltara (IMD Raipur), the Kharun at Pathardihi, the Mahanadi at Kalma and Surajgarh and the Mand at Kurubhata set new high flood levels (CWC levels, as reported), 11,942 houses were damaged and 219 relief camps were opened (as reported). On 25–28 August 2025 floods in the Bastar division killed eight people and damaged about 495 houses and 16 culverts and bridges (as reported). In July 2026 Raipur's underpasses and low colonies were flooded and five ponds overflowed into homes, with no depth of rain reported (as reported), and Bilaspur was reported to have received 28 cm in a day in one source and 415.6 mm in another, a conflict that IMD has not resolved. On 24–27 September 2026 floods across the State killed eight people and affected 22,377 people in 571 villages, all 14 gates of Gangrel were opened, the Kharun overflowed at Mahadev Ghat and the Indravati at Jagdalpur set a new high flood level (as reported). The records of the past are larger still: 392.0 mm at Rajnandgaon on 21 September 1965, 358.0 mm at Korba on 12 August 2004 and 337.0 mm at Durg on 15 June 2004 (IMD). No review of any flood event, and no record of flood extents, depths or durations kept by any Municipal Corporation, NRDA or CGSDMA, was found. NDMA (2010) called for automatic rain gauges at "1 in every 4 sq km" in Class I, II and III cities. Clause 10.16.1 asks only for gauges where they serve design and review. The report and sampling intervals are set by this Manual.

10.17 Data publication

10.17.1 Each Authority shall publish, on a public website, and update at least quarterly:

- (a) a **facility register**: location, type, design volume, release rate, owner, date of as-built acceptance, and the latest inspection result for every facility approved under this Manual;
- (b) a summary of each approved drainage report, with Forms 2-1 and 10-2;
- (c) the register of variances and waivers (Clause 1.10);
- (d) the alternatives adopted under the Policy Supplement (Clause 1.7);
- (e) enforcement actions (Clause 10.15.5); and
- (f) event reports and post-event reviews (Clause 10.16).

10.17.2 The State Stormwater Technical Committee shall publish the compliance calculator, all Annexes, the flood-sensitive catchment maps and the State-level summary of facility registers as open data.

10.17.3 Personal data other than the name of a corporate owner or association shall not be published. Security-sensitive details of critical infrastructure may be withheld, with the reason recorded.

10.17.4 Published data shall be machine-readable (for example CSV and GeoPackage) and released under the Government Open Data Licence – India or an equivalent open licence.

Commentary. Transparency is a design criterion in its own right here. Some of Chhattisgarh's drainage records are public, though most of the State's websites could be read only in archived copies: the amendments of 2018 and 2020 to the Chhattisgarh Bhumi Vikas Niyam 1984, the General Administration Department's order on rainwater harvesting of 31 May 2016 and the approval notifications of the Raipur and Arpa development plans (on the T&CP website); chapters of the Naya Raipur Development Plan-2031 and Raipur's AMRUT storm drainage plan of 2015; the Ground Water (Management and Regulation) Act 2022 and WRD's daily tank gauge reports; the CAG's audit of AMRUT; the reports of CECB and the district administration to the National Green Tribunal; IMD's *Climate of Chhattisgarh* and the monsoon reports of the Meteorological Centre Raipur; CGWB's groundwater assessments; and the Ramsar information sheet for Kopra Jalashay. Much else could not be found in the public domain: the

consolidated text of the Bhumi Vikas Niyam; the Chhattisgarh texts of the Municipal Corporation and Municipalities Acts; NRDA's drainage report, lake levels and development regulations; the design basis of any storm drain in Raipur, Bilaspur, Durg, Bhilai or Korba; the approval of a revised Raipur development plan; any State disaster management or urban flood plan; warning and danger levels for any CWC site in the State except Jagdalpur; the operating rules of WRD's dams and barrages; an inventory of the sluices, interception weirs and pumping stations of any town; any notification bringing the Ground Water Act into force; the list of wetlands under the State Wetland Authority; and the texts of the High Court's orders on the Arpa and the Kharun. Publishing inputs and facility records lets the public, researchers and other agencies check that the city's growing stock of private storage actually exists. The open-format requirement is set by this Manual.

10.18 Review of this chapter

10.18.1 This chapter shall be reviewed with the Manual under Clause 1.12. The forms and the compliance calculator may be revised by the State Stormwater Technical Committee between reviews, under Clause 10.5.3.

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Annexes and forms

Annex / Form	Content
10A	Compliance calculator and user guide
10B	GIS data specification and attribute schema
10C	Maintenance access dimensions
10D	Model O&M plans by facility type
10E	Empanelment criteria for Stormwater Design Professionals and reviewers
Form 10-1	Drainage Report Checklist
Form 10-2	Compliance Calculator Summary
Form 10-3	Professional Design Certificate
Form 10-4	Hold-Point Inspection Record
Form 10-5	As-built Certificate
Form 10-6	Maintenance Declaration
Form 10-7	Annual Maintenance Declaration and Inspection Report
Form 10-8	Asset Handover Certificate
Form 2-1	Design Rainfall Sheet (Chapter 2)

CHAPTER 11

Basin and City Stormwater Master Plans

11.1 Purpose and scope

11.1.1 Chapters 2 to 10 control one project at a time. This chapter sets the requirements for the plan that ties those projects together: the **Basin Stormwater Master Plan (BSMP)**. A BSMP is prepared for each drainage basin of a city or urban area. A city's set of BSMPs makes up its **City Stormwater Master Plan**.

11.1.2 A BSMP shall:

- (a) map flood hazard for the basin under existing and future conditions;
- (b) set the level of service each part of the drainage system must achieve (Clause 11.6);
- (c) identify the works, storage and land needed to reach that level of service, and rank them (Clauses 11.7 and 11.8);
- (d) reserve land for regional storage, talab and tank restoration, and major flow paths in the statutory land-use plan (Clause 11.9);
- (e) set the basin-specific inputs that site designs under Chapter 4 depend on: regional facilities, the fee-in-lieu unit rate, and any basin release rate stricter than Chapter 4 (Clause 11.8.3); and
- (f) recommend catchments for notification as flood-sensitive (Clause 4.5.4).

11.1.3 **Relationship to site design.** Site criteria (Chapters 4 to 8) apply whether or not a BSMP exists. A BSMP may make the site criteria **stricter** for its basin. It shall not relax them, except through the regional detention and fee-in-lieu routes of Clauses 4.13 and 4.5.6.

> Commentary. Chhattisgarh's cities have drainage proposals but, with one exception, no published drainage plan with a design standard. The exception is Nava Raipur: the Naya Raipur Development Plan-2031 (NRDA, 2008) divides the new capital into four catchments along its four main streams, which it keeps, asks that the existing channels be rehabilitated and that "5 years Maximum rainfall intensity should be taken for design of system" (§10.3.1.1), that any river embankment be safe against a 100-year flood, that roads stand at least 0.5–1.0 m above the observed high flood level and follow IRC SP-50, and that rainwater from the storm drains be stored in dug or natural lakes and depressions; it gives a chart of intensity against time of concentration for 2, 5, 10 and 50 years from IMD data for the Raipur region, read only by eye, and the new city's 55 reservoirs, Sendh, Jhanjh and Khandawa among them, are reported as its storage (as reported). This is the only explicit urban design storm in the State, and it is a plan for one city, not a State standard. Raipur's storm drainage plan under AMRUT (RMC, 2015) records drains covering 49% of the city, about 480 km of pucca drains, 11 road stretches flooding two or three times a year and drains "not sufficient" in capacity, and proposes ₹1,191 crore of works, but states no return period or intensity; AMRUT in the State then funded no drainage, and a ₹222.22 crore urban flood risk project for Raipur under the NDMA's programme, approved on 1 October 2025, has been reported without its criteria (as reported). Raipur's development plan, the Raipur Development Plan (Revised) 2021, was approved on 5 April 2008, and the revised plan to 2031, presented in 2022 with an emphasis on water bodies, has not been found approved. In Bilaspur the Arpa Special Area Development Authority's development plan to 2033, approved on 10 February 2014, was largely sidelined after the High Court's orders of January 2019, when the State dropped the Authority's permission for land sale, building and development. Bhilai's township is planned and drained by Bhilai Steel Plant (SAIL) through its Town Services, beside the drains of the Bhilai and Risali Municipal Corporations, with no

published division of the drains between them. No published document sets a design return period for the storm drain network of any other Chhattisgarh city. This chapter supplies the design storm, levels of service, storage and buffer criteria that such a plan needs, with the design storms of Chapter 2 and the levels of service of Clause 11.6; it has not been adopted by UADD, the Housing and Environment Department, T&CP, NRDA, RDA, any Municipal Corporation, Bhilai Steel Plant, WRD or any other body. A site-level manual cannot fix basin-level problems: lost tank storage, city nalas that reach the Kharun, the Seonath, the Arpa and the Hasdeo while barrage and dam releases hold them high, nalas that carry sewage to their outfalls and into the talabs, talabs filled or encroached, as in Raipur, where about 40 remain of the 130 or more once reported, catchments changed by mine voids, overburden dumps and ash ponds in the coalfields, and single drains split between a Municipal Corporation, NRDA, Bhilai Steel Plant, WRD and the coal and power companies, and drains that are adequate on their own but overload the next reach. Comparable manuals pair site criteria with basin planning:

- Denver's Mile High Flood District prepares major drainageway master plans that set regional facilities and release rates.
- The Kansas City APWA criteria rest on regional watershed studies (MARC).
- England's Surface Water Management Plans (Defra, Surface Water Management Plan Technical Guidance, 2010) set a phased method of preparation, risk assessment, options and implementation.
- Malaysia's MSMA requires drainage master plans for urban areas.

This chapter adapts that method to Chhattisgarh: to a State of plains, plateau and hills whose city nalas run to rivers held by dams and barrages, as in Raipur, where the nalas and talabs drain to the Kharun, at Durg and Bhilai on the Seonath below the Mongra barrage, at Bilaspur on the Arpa at the Bhaisajhar barrage, at Korba on the Hasdeo below Minimata Bango and the Darri barrage, and at Raigarh on the Kelo, with the Hirakud backwater reaching the low outfalls of Raigarh and Sarangarh districts and the Indravati at Jagdalpur setting a new high flood level in 2026; to a planned capital, Nava Raipur, drained to a network of lakes and reservoirs on the only explicit urban design storm in the State; to talabs filled or encroached, of which Raipur keeps about 40 of the 130 or more once reported, and to a Wetland Authority that now enforces a 50 m line around them; to the coalfields and power stations of Korba, Raigarh and Surguja, where mine voids, overburden dumps and ash dykes change the catchments and their hazards; to nalas that carry sewage, intercepted by weirs at Raipur's new treatment plants and under orders of the Tribunal at Durg; to drains vested in the Municipal Corporations, the drains and lakes of NRDA, the Bhilai Steel Plant township and the rivers, dams and barrages of WRD, without an order that divides the natural nalas from the municipal drains; to a State with no floodplain zoning law and no notified flood zone; to a State with no published design return period for the drains of any city other than Nava Raipur; to hard-rock and karst aquifers critical under Raipur's Dharsiwa block and the Seonath plain, with a water table near the surface after the monsoon; and to a south-west monsoon and Bay of Bengal depressions whose recorded days exceed the 100-year depths of the CWC plates, 370.3 mm at Raipur on 4 August 1910 and 392.0 mm at Rajnandgaon on 21 September 1965 (IMD).

11.2 Definitions

Term	Meaning in this Manual
Basin	The land draining to one outfall to the sea, a river, or a major waterbody, as delineated under Clause 11.3.
Basin Stormwater Master Plan (BSMP)	The plan for one basin prepared under this chapter.
Lead Authority	The body responsible for preparing a BSMP (Clause 11.4).
Level of service (LoS)	The flood performance a part of the drainage system must achieve, expressed as an AEP and an outcome (Clause 11.6).

Term	Meaning in this Manual
Flood hazard map	A map of flood extent, depth, velocity and hazard class (H1–H6, Chapter 6) for a stated AEP and scenario.
Regional facility	A storage, conveyance or outfall work serving more than one development, identified in a BSMP.
Capital Improvement Programme (CIP)	The ranked, costed list of works in a BSMP (Clause 11.8.4).

11.3 Planning unit: the basin, not the ward

11.3.1 BSMPs shall be prepared **by hydrological basin**, not by ward, zone or local-body boundary. Where a basin crosses local-body boundaries, one BSMP shall cover the whole basin.

11.3.2 Basin boundaries shall be delineated from the digital elevation model of Clause 11.5.2, checked against the drain network, road embankments, canals and tank supply channels. Diversions between basins, whether by canal, drain or surplus course, shall be mapped and represented in the model.

11.3.3 No basin delineation has been published for any Chhattisgarh city. For the urban areas where the evidence identifies them, the basins are at least the following:

- **Raipur, Kharun:** the catchments of Raipur and Birgaon that drain to the **Kharun** through the city's nalas, with the city's **talabs** (Budha Talab, Telibandha, Karbala, Narhaiya, Maharajbandh and the others) as storage, the diversion weirs of the **Chandanidih, Kara and Nimora** treatment plants and the anicut at **Mahadev Ghat** in the 1D network, their sewage interceptions mapped (Clause 8.9.5), and the level of the Kharun at **Pathardihi** as the boundary condition;
- **Nava Raipur:** the four catchments of the four main streams of the Naya Raipur Development Plan-2031, with the city's lakes and reservoirs, **Sendh**, **Jhanjh** and **Khandawa** among them, as storage and outfall, the 50 m line of the State Wetland Authority shown, and the river that receives each stream as the boundary condition;
- **Durg and Bhilai:** the catchments of Durg, Bhilai, Risali and Bhilai-Charoda that drain to the **Seonath**, with the **Shankar** and **Pulgaon** nalas and the drains of the Bhilai Steel Plant township in the 1D network, their sewage interceptions mapped, and the Seonath level below the **Mongra** barrage as the boundary condition;
- **Bilaspur, Arpa:** the catchments of Bilaspur that drain to the **Arpa**, with the pond level and releases of the **Arpa Bhaisajhar** barrage as boundary conditions and **Kopra Jalashay**, west of the city, shown with its catchment;
- **Korba, Hasdeo:** the catchments of Korba that drain to the **Hasdeo** below the **Darri** barrage, with the releases of **Minimata Bango** and Darri as inflows and the levels at Bangodam and Katghora as boundary conditions, the outfall-closed case a design case, mine voids shown as storage only where the lessee and DGMS consent and their levels and spills are surveyed, overburden dumps as sediment sources, and the breach flow paths of the ash dykes of Dhanras, the Hasdeo Thermal Power Station and BALCO shown as hazards;
- **Raigarh, Kelo:** the catchments of Raigarh that drain to the **Kelo** below the **Kelo dam**, with the levels of the **Mahanadi** and the **Mand** and of the **Hirakud** backwater as boundary conditions for the low outfalls of Raigarh and Sarangarh districts; and
- **Jagdalpur, Indravati:** the catchments of Jagdalpur that drain to the **Indravati**, with the level at CWC's Jagdalpur site (warning 539.49 m, danger 540.80 m, high flood level 544.891 m) as the boundary condition and the outfall-closed case a design case.

Other Tier A bodies, among them Rajnandgaon, Dhamtari, Ambikapur and Chirmiri, shall define their basins in the first BSMP that covers them. Where WRD, CWC, DGMS, the State Wetland Authority or a court has a high flood level, a full reservoir level, a line around a talab or wetland, or an operating rule for a dam, barrage, sluice or pumping station, the BSMP shall take its boundary conditions from it and show those levels against its own 1% AEP map.

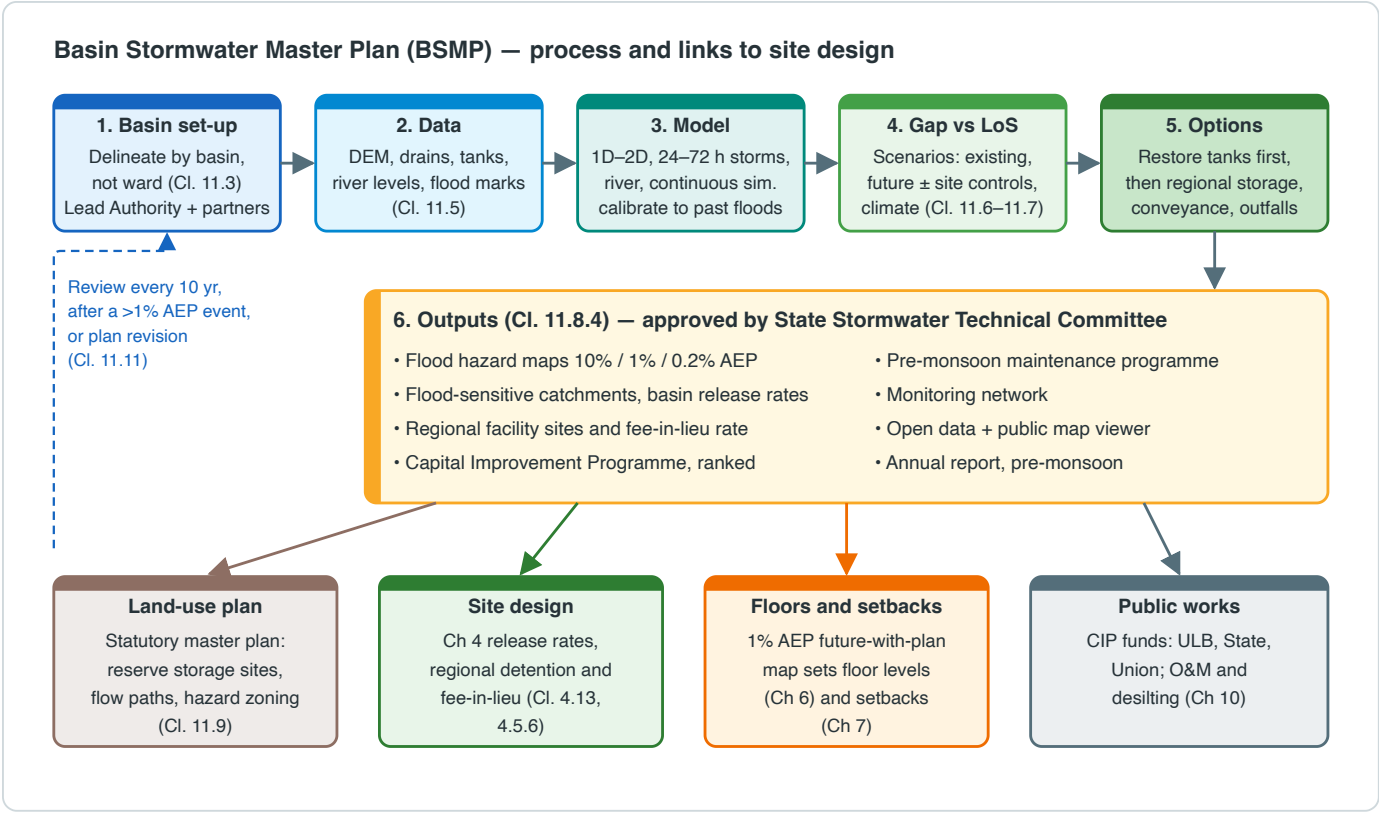

Figure 11.1 — Basin Stormwater Master Plan process and its links to site design. Original drawing for this Manual. The phase structure is adapted from Defra, Surface Water Management Plan Technical Guidance (2010), and Mile High Flood District master planning practice.

Commentary. Water does not follow ward boundaries. A ward-by-ward drain plan can pass the problem to the next ward, and to the next local body. In Chhattisgarh a single nala can pass through several hands on its way to a river. The Municipal Corporation Act 1956 and the Municipalities Act 1961, as adapted from the Madhya Pradesh Acts, vest the public drains and tanks of a city in its local body (their Chhattisgarh texts were not read); NRDA plans and builds the drains and lakes of Nava Raipur; Bhilai Steel Plant drains its township, whose drains Bhilai Municipal Corporation has had to warn it to clean (2025, as reported); tanks recorded for irrigation or nistar vest in the State, under the Collector (Land Revenue Code 1959, s.251); WRD holds the rivers, dams, barrages and reservoirs; and the coal companies and power stations hold the drains, voids, dumps and ash ponds on their land. No rule divides the natural nalas from the municipal drains (as this Manual reads the record). The nalas carry sewage across these boundaries: in January 2021 Bilaspur was the only city with a sewerage system, and the new plants in Raipur take the flow of the nalas through diversion weirs (CECB); Durg's sewage reaches the Seonath through the Shankar and Pulgaon nalas (National Green Tribunal, 2026, as reported); and about 40% of Bilaspur's sewage was expected to remain untreated even after its new plants (High Court, 2025, as reported). This clause makes the basin the planning unit, so that one model carries each nala from its head to the river whoever owns each reach, and so that the interception works are tested against the capacity of the nalas they tap (Chapter 8, Clause 8.9.5).

11.4 Who prepares, and when

11.4.1 **Lead Authority.**

- For the basins of a city with a Municipal Corporation: the **Municipal Corporation** (in Raipur, **RMC**; elsewhere the Corporations of Bilaspur, Durg, Bhilai, Bhilai-Charoda, Risali, Birgaon, Rajnandgaon, Korba, Raigarh, Jagdalpur, Ambikapur, Dhamtari and Chirmiri), in which the public drains and tanks vest under the Municipal Corporation Act 1956 as adapted in Chhattisgarh, jointly with **T&CP**, which prepares the development plan into which the BSMP is carried (Clause 11.9); either may prepare the plan as UADD directs. In Nava Raipur the Lead Authority is **NRDA**, the Special Area Development Authority under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973, for the whole of its area and its lakes and reservoirs. Where a basin extends beyond the Corporation, the other local bodies, RDA and NRDA in the basin are partners, and one BSMP covers the whole basin (Clause 11.3.1); a basin that drains from Raipur into Nava Raipur, or the reverse, has one BSMP.
- No order dividing the natural nalas from the municipal drains, or the drains of the local bodies from the rivers, dams and barrages of WRD, was found; until one is made, this Manual assigns the reaches within each BSMP as follows, each body being Lead Authority for the reaches and works it owns or runs and for the parts of the CIP on them: **WRD** for the rivers, dams, barrages and reservoirs and their gates and releases, among them Gangrel, Minimata Bango, the Hasdeo barrage at Darri, the Mongra and Arpa Bhaisajhar barrages, Kharang and the Kelo dam; the **coal company or other lessee**, with **DGMS**, for the mine voids, pit lakes, overburden dumps, garland drains and sediment basins within its lease, and the **power company** (NTPC, CSPGCL, BALCO or another) for its ash ponds, ash dykes and their breach flow paths; the **Chhattisgarh State Wetland Authority** for notified wetlands, for the 50 m lines of the talabs and wetlands it treats as protected and, with WRD as its managing authority, for **Kopra Jalashay**, the State's Ramsar site; the **Municipal Corporation**, **Nagar Palika Parishad** or **Nagar Panchayat** for the drains, nalas and public tanks vested in it; **NRDA** in Nava Raipur; **Bhilai Steel Plant** for the drains of its township, until the responsibility for them is settled with the Municipal Corporations; **RDA** and the **Chhattisgarh Housing Board** for the drains of their schemes until they are handed over; the **Collector** for the tanks vested in the State under s.251 of the Land Revenue Code 1959 in the parts of a basin outside municipal limits; and PWD and NHAI for the drains of their roads.
- In Tier B bodies: the Lead Authority is named by UADD. Basins draining to a common river reach, reservoir or talab may be grouped into one district-level plan.

The Chhattisgarh State Wetland Authority shall be consulted on every BSMP whose basin drains to a talab or wetland it treats as protected or to Kopra Jalashay; CGSDMA and the district administration on every BSMP; the Chhattisgarh Environment Conservation Board on every BSMP that proposes works on a nala that carries sewage to a river or talab, or that receives runoff from a mine, a dump or an ash pond; and, for a basin whose outfall lies on a river shared with Odisha, such as the Mahanadi below the State's barrages or within the Hirakud backwater, or the Indravati, the water resources department of that State, through WRD.

WRD shall be a partner in every BSMP that includes a river, canal or tank under its control.

11.4.2 **When a BSMP is required.**

- (a) Every Tier A body shall approve BSMPs covering its whole area within **3 years** of this Manual taking effect.
- (b) A BSMP is required before:
  - any regional detention or fee-in-lieu is accepted under Clause 4.13;
  - a fee held under Clause 4.5.6.3 is spent; and
  - a basin-specific release rate is applied under Clause 11.8.3.
- (c) A Tier B body shall prepare a BSMP where its Lead Authority, or the State Stormwater Technical Committee, finds recurrent flooding of buildings or arterial roads.

11.4.3 **Preparation.** BSMPs shall be prepared by, or under the direction of, an engineer empanelled as a Stormwater Design Professional (Chapter 10) with experience in basin-scale modelling.

11.5 Data and surveys

11.5.1 Each BSMP shall assemble and publish (Clause 11.10) the data in Table 11.1:

Table 11.1 — Minimum data and surveys for a BSMP

Data	Minimum requirement
Terrain	Digital elevation model from LiDAR or photogrammetry, with vertical accuracy of 0.15 m RMSE or better in urban areas, tied to the Survey of India datum
Drain network	Inventory of all public drains, culverts and outfalls with invert levels, sizes, condition and silt depth; private drains above 600 mm
Waterbodies	Survey of every tank, talab, pond and marsh under Chapter 7: FTL, MWL, bund level, stage–storage, sluices, surplus weirs, supply channels, encroachment
Rivers and canals	Cross-sections at no more than 200 m spacing and at every structure
Outfalls, rivers, reservoirs, barrages and mine voids	The datum of every level named; outfall invert levels against the warning, danger and high flood levels of the receiving river at the nearest CWC gauge and its recorded peaks (on the Indravati at Jagdalpur a warning level of 539.49 m, a danger level of 540.80 m and a high flood level of 544.891 m, 26 September 2026; on the Kharun at Pathardihi a high flood level of 281.68 m, 29 August 2020, with no warning or danger level found; CWC levels, as reported), with any local gauge kept as a separate series; for outfalls to reservoirs, barrage ponds and talabs, the full reservoir level, maximum water level and spillway and sluice data from WRD or the custodian (for example the full reservoir levels of 302.000 m for the Arpa Bhaisajhar barrage, 337.150 m for the Mongra barrage and 233.000 m for the Kelo dam in WRD's tank gauge report of 10 October 2024), naming which is used; the release records of Gangrel, Minimata Bango, the Hasdeo barrage at Darri, Kharang and the barrages on the Seonath, the Arpa and the Mahanadi; the extent of the Hirakud backwater inside the State; for any mine void proposed as storage, the lessee's survey of its level, spill and highwall stability, with the consent of DGMS; the breach flow paths of ash dykes mapped by their owners; WRD's high flood points given for land next to a nala, canal or river, the lines of rule 50 of the Chhattisgarh Bhumi Vikas Niyam 1984 once confirmed, and the 50 m line of the State Wetland Authority where they bind; and the sluices, flap gates, interception weirs, anicuts, check dams, lake outlets and pumping stations on the outfalls, with their capacities and operating rules
Rainfall and levels	All available rain gauges (IMD, including its observatories at Raipur (since 1901) and Raipur (Mana) airport, its stations at Jagdalpur, Pendra Road, Ambikapur, Raigarh, Durg, Champa, Jashpurnagar, Kanker and Rajnandgaon, and the automatic weather stations and rain gauges reported by the Meteorological Centre Raipur; the district and tehsil rain gauges of the State, whose records IMD tabulates in the *Climate of Chhattisgarh*; the rain and river gauges of CWC and of WRD, including those at its dams and barrages; the agrometeorological observatory of Indira Gandhi Krishi Vishwavidyalaya at Raipur; and any gauges kept by the Municipal Corporations, NRDA and the Smart City companies) and water-level records, including those for the 4 August 1910 (Raipur's record day of 370.3 mm, rainfall only), August 2020 (the Kharun at Raipur and the upper Mahanadi), August 2025 (the Bastar division) and September 2026 (statewide, with the Indravati at Jagdalpur and the Kharun at Raipur) events
Flood evidence	Surveyed flood marks and mapped flood extents from past events, including public reports
Land use	Existing land use and imperviousness; future land use from the statutory master plan
Soils and groundwater	HSG map (Annex 3A); seasonal high groundwater; aquifer and recharge zones
Sewer interaction	Known sewage cross-connections, dry-weather flow in drains, and solid-waste hotspots

Commentary. The 0.15 m accuracy has no direct precedent; this Manual sets it because on the Chhattisgarh plain, where slopes are gentle (0–5% across Nava Raipur, NRDA) and the kanhar and dorsa lowlands, the talabs and the floodplains of the Kharun, the Seonath and the Arpa decide where water stands, and around the margins of Raipur's talabs and Nava Raipur's lakes and reservoirs, where the line 50 m from the highest flood level that the State Wetland Authority enforces decides what may be built, a small error in ground level moves the flood edge a

long way, and the 1% extent from the BSMP sets floor levels (Chapter 6) and setbacks (Chapter 7). Past flood marks matter as much as models. The 4 August 1910 (Raipur's record day of 370.3 mm, rainfall only), August 2020 (the Kharun at Raipur and the upper Mahanadi), August 2025 (the Bastar division) and September 2026 (statewide, with the Indravati at Jagdalpur and the Kharun at Raipur) events are the best calibration data Chhattisgarh has, and much of the evidence is held in photographs, news reports and residents' records that should be collected before it is lost.

11.6 Levels of service

11.6.1 Each BSMP shall adopt the following minimum levels of service. A BSMP may adopt stricter levels.

Table 11.2 — Minimum levels of service

Element	Minimum level of service
Minor system	The design AEP of Chapter 6 (Table 6.1)
Habitable buildings	No flooding above floor level in the 1% AEP (100-yr) event
Critical facilities (hospitals, emergency services, substations, water and sewage plants)	No flooding of the facility in the 0.2% AEP (500-yr) event; access road passable in the 1% AEP event
Evacuation routes and arterial roads	Passable (hazard class H2 or better, Chapter 6) in the 1% AEP event
Ponding drain time	Surface water cleared from roads within **6 hours** after rain ends in the 10% AEP (10-yr) event
Tank and talab storage	No net loss of flood storage below the MWL in the basin, measured against the earliest available survey
Outfalls	Functioning under the joint and outfall-closed cases of Chapter 5

Commentary. The building and road levels repeat Chapter 6, so that the basin plan and the site designs aim at the same outcome. The 0.2% AEP for critical facilities, the 6-hour ponding drain time and the no-net-loss rule for tank storage have no direct precedent and are set by this Manual. The drain time reflects what residents notice most after a storm. The no-net-loss rule reflects a loss of storage that has been reported but not measured. In law the tanks of Chhattisgarh are largely public: village tanks on land with rights of irrigation or nistar vested in the State on 6 April 1959 (Land Revenue Code 1959, s.251), the Collector may set land apart for tanks for nistar (s.237), and encroachment on a pond recorded in the land records is removed as unauthorised occupation (s.248), as at Baloda in 2024–25 under the National Green Tribunal's order (O.A. 78/2024); within a city the public tanks vest in the local body under the Municipal Corporation and Municipalities Acts as adapted (their Chhattisgarh texts were not read); and the State Wetland Authority bars works within 50 m of the highest flood level of the talabs and wetlands it treats as protected (as reported). Kopra Jalashay near Bilaspur, a 210 ha reservoir built in 1993 and managed by WRD, is the State's first Ramsar site (designation date 8 August 2025 in the Ramsar information sheet; announced in December 2025, as reported). The losses are visible all the same: Raipur, a city of talabs, is reported to have had about 130 ponds or more, of which about 40 remain over about 69 ha, all polluted and silted (as reported; the counts conflict); a directive of 8 May 2023 to survey every water body in Raipur district was reported to have been carried out only at Karbala Talab (as reported); and the State Wetland Authority found works at Karbala Talab within 50 m of its highest flood level and stopped them (2025, as reported). Beyond the cities lie the talabs, bandhs and dabris of the villages, and the farm ponds dug under the State's schemes. No survey of the storage of any talab or reservoir inside a Municipal Corporation was found, and no level or capacity has been published for any of Raipur's talabs or Nava Raipur's lakes.

11.7 Modelling and scenarios

11.7.1 **Model.** Each BSMP shall use a coupled 1D–2D hydraulic model of:

- the minor system;
- the major (overland) system;
- rivers, canals and tanks; and
- the outfalls.

Methods shall follow Chapter 3. Tidal and river boundaries shall follow Chapter 5.

11.7.2 **Design storms.** The model shall be run for the design storms of Chapter 2, including the 24-, 48- and 72-hour storms, for at least the 50%, 10%, 1% and 0.2% AEPs. Areal reduction shall be applied under Clause 2.7.3.

11.7.3 **Continuous simulation.** Basins with tank cascades shall also be run by continuous simulation (Clause 3.13), using the full IMD gridded daily record where available and at least 30 years, disaggregated by the temporal patterns of Annex 2D. This shows how full the tanks are when the design storms arrive, and how often heavy rain coincides with a closed outfall. The two-step approach of Clause 3.13.5 may be used: a simplified storage model is run for the whole record, and the coupled 1D–2D model of Clause 11.7.1 is run only for the design storms, starting from the tank levels the continuous run gives. A continuous run of the full 1D–2D model is not required.

11.7.4 **Scenarios.** Each BSMP shall model at least:

- (a) **Existing:** existing land use, network, tanks and outfalls;
- (b) **Future, uncontrolled:** future land use under the statutory master plan, with no site controls;
- (c) **Future, controlled:** future land use with the site controls of Chapter 4;
- (d) **Future, with plan:** scenario (c) plus the works of the CIP; and
- (e) **Climate:** scenarios (a) and (d), with the climate factors of Clause 2.8, used unchanged: no projection of heavy rainfall for Chhattisgarh with a design factor was found; the State Action Plan on Climate Change (2014) is summarised as finding falling monsoon totals, while the maximum 1- and 5-day rainfalls at stations of the Mahanadi plain are reported to have risen over 1971–2021 (as reported); and station records, 370.3 mm at Raipur on 4 August 1910, 392.0 mm at Rajnandgaon on 21 September 1965, 358.0 mm at Korba on 12 August 2004 and 337.0 mm at Durg on 15 June 2004 (IMD), exceed the 100-year 24-hour depths of the CWC plates there (this Manual's reading), which is a reason to keep the factors, not to reduce them.

11.7.5 **Calibration.** The model shall be calibrated and validated under Clause 3.15, using at least two of the 4 August 1910 (Raipur's record day of 370.3 mm, rainfall only), August 2020 (the Kharun at Raipur and the upper Mahanadi), August 2025 (the Bastar division) and September 2026 (statewide, with the Indravati at Jagdalpur and the Kharun at Raipur) events where data exist.

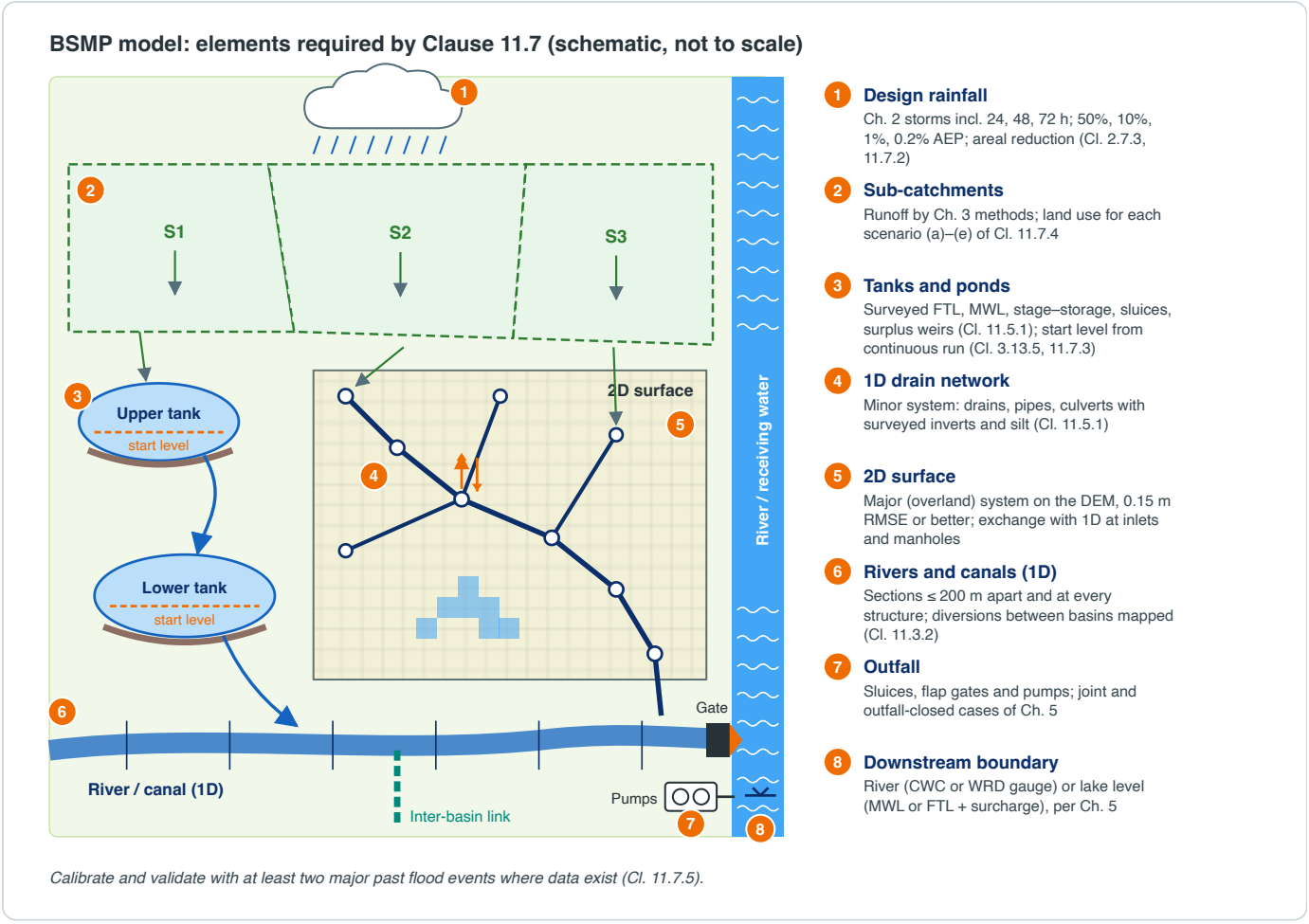

Figure 11.2 — Elements of the BSMP model required by Clause 11.7: sub-catchments, the 1D drain and channel network, the 2D surface, tanks and talabs with starting levels from continuous simulation, and the tidal or river boundary at the outfall (schematic, not to scale). Original drawing for this Manual, adapted from common coupled 1D–2D urban flood modelling practice.

> **Commentary.** Scenario (b) against scenario (c) shows what the site rules achieve on their own. Scenario (d) shows what the public works add. This comparison is also the evidence base for notifying flood-sensitive catchments (Clause 4.5.4) and for setting a stricter basin release rate (Clause 11.8.3).

11.8 Options, outputs and the Capital Improvement Programme

11.8.1 **Options hierarchy.** Options shall be considered in the following order, the basin-scale form of the hierarchy in Clause 1.2:

1. **Protect and restore storage:** talabs, tanks, marshes and floodplains, including desilting to the original bed and recovering encroached area (Chapter 7);
2. **Regional retention and detention**, sited by modelling so that releases do not coincide with the main-stem peak (Clause 4.13);
3. **Conveyance** upgrades to drains, canals and culverts;
4. **Outfall works:** sluices and flap gates on the outfalls to the Kharun, the Seonath, the Arpa, the Hasdeo, the Kelo and the Indravati, pumping stations and the storage behind them where an outfall can be closed by a river in flood, a barrage pond or the Hirakud backwater, interception weirs that take the dry-weather flow of the nalas of Raipur and Durg to the sewage treatment plants without taking their storm capacity, sediment basins and toe

drains below overburden dumps and mine sites, and the operating rules of the gates of Gangrel, Minimata Bango, the Hasdeo barrage at Darri, Kharang and the Mongra and Arpa Bhaisajhar barrages that set the receiving levels, agreed with WRD;

5. **Property-level protection** where works cannot reach the level of service; and
6. **Non-structural measures:** flood warning (for example IMD district rainfall warnings and nowcasts issued by the Meteorological Centre Raipur, CWC flood forecasts and river stages at its sites, among them the Indravati at Jagdalpur against its warning and danger levels, notice of releases from Gangrel, Minimata Bango, the Hasdeo barrage at Darri, the Mongra and Arpa Bhaisajhar barrages and Kharang to the towns and villages downstream, and the district flood control rooms), flood-hazard zoning, emergency planning and public information.

11.8.2 **Option assessment.** Each option shall be assessed for:

- reduction in flood risk (buildings and people flooded; expected annual damage);
- capital and 30-year maintenance cost;
- land required;
- effect on groundwater recharge in the critical and semi-critical blocks of the Seonath plain, Dharsiwa (Raipur), Durg and Dhamdha among them, where the Chandi and Charmuria limestone that supplies Raipur is karstified and cavernous, the shallow post-monsoon water table, within 5 m in about three wells in four, kanhar clays that take no recharge once wet, mine dewatering in the Korba coalfield, and the quality of the groundwater, with fluoride and nitrate in parts of the State and arsenic at Ambagarh Chowki, and of the rivers, reservoirs and talabs that the runoff reaches (Chapter 8);
- effect on downstream basins and the receiving river, reservoir or talab; and
- who benefits.

11.8.3 **Basin release rates.** Where scenario (c) does not meet the level of service, the BSMP may set a basin-specific **permissible site discharge** ($m^3/s/ha$) stricter than Chapter 4, by AEP and sub-catchment. The rate takes effect under the notification procedure of Clause 4.5.4.

11.8.4 **Outputs.** Each BSMP shall produce:

- (a) flood hazard maps (extent, depth, velocity, hazard class) for the 10%, 1% and 0.2% AEPs, existing and future-with-plan. The **1% AEP future-with-plan map** becomes the reference map for floor levels (Chapter 6) and setbacks (Chapter 7);
- (b) recommended flood-sensitive catchments and any basin release rates (Clauses 4.5.4 and 11.8.3);
- (c) regional facility sites, with storage volumes, land needs and the sites they serve;
- (d) the fee-in-lieu unit rate for the basin (Clause 4.13.3);
- (e) the **Capital Improvement Programme**: works ranked by risk reduction per rupee, with cost, phasing and agency;
- (f) a pre-monsoon **maintenance programme**: desilting, gate and pump testing, and tank operating rules (Chapter 10); and
- (g) monitoring needs: new rain gauges, level sensors and flood-mark survey points.

> Commentary. Restoring storage comes first because many of Chhattisgarh's city outfalls are closed or limited in the wettest weeks, and because conveyance on its own has not kept up. The Kharun at Pathardihi stood above its 1994 mark for about 46 hours in August 2020 and overtopped the road at Mahadev Ghat in 2024 and 2026, Gangrel opened all 14 of its gates in September 2026, Minimata Bango and the Darri barrage released together onto Korba in August 2024 (as reported), and the Hirakud backwater reaches into the State in the east. Raipur once held its runoff in its talabs, and of the 130 or more reported in the city about 40 remain, over about 69 ha (as reported; the counts conflict), while Nava Raipur was planned on the opposite principle, with its storm drains and parks leading to its lakes and reservoirs (NRDA, 2008). Conveyance alone has not served: Raipur's plan of 2015 found its drains insufficient in capacity and choked, and proposed to reach full coverage by 2020 (RMC, 2015), but its low colonies and underpasses flooded again in July 2026 (as reported). The storms are large for drains with no stated design basis: IMD has recorded 370.3 mm in a day at Raipur (4 August 1910), 392.0 mm at Rajnandgaon

(21 September 1965) and 358.0 mm at Korba (12 August 2004), above the 100-year 24-hour depths of the CWC plates there (IMD; this Manual's reading). Conveyance upgrades on their own move water faster to an outfall that the Kharun, the Seonath, the Arpa, the Hasdeo or the Indravati in flood, a barrage pond or reservoir held at its full level, or the Hirakud backwater may close, or that is limited by the capacity of its sluice, interception weir or pumping station. Groundwater in Chhattisgarh calls for recharge on the Seonath plain and for care everywhere. The State's stage of extraction was 47.32% in 2024, with 120 of its 146 blocks safe, 21 semi-critical and 5 critical, among them Dharsiwa in Raipur district at 93.51% and Gurur, Nawagarh, Bemetara and Berla nearby; Durg, Dhamdha and Rajnandgaon were semi-critical (CGWB, 2024); a press report that no block was critical in 2026 has not been verified. About 87% of the State is hard rock, and the Chandi limestone that supplies Raipur, with the Charmuria limestone of the same belt, is described as "Karstified and Cavernous" (CGWB), so recharge there reaches the aquifer fast, with whatever the runoff carries, and needs a cavity check. But the post-monsoon water table stands within 5 m of the ground in about 76% of wells (CGWB, 2023), so recharge on the kanhar and dorsa lowlands adds to waterlogging. Quality limits recharge too: fluoride exceeds 1.5 mg/l in samples from eight districts and nitrate exceeds 45 mg/l in about 11% of samples, arsenic is found at Kaudikasa in Ambagarh Chowki block and in samples from Korba, Koriya and Raigarh (CGWB), and in the Korba coalfield mine dewatering draws the water table down around the pits (as reported). The Ground Water (Management and Regulation) Act 2022 would let the State enforce recharge in notified critical blocks and stressed urban areas, but no notification bringing it into force has been found. The rainwater harvesting rule of the Bhumi Vikas Niyam asks for recharge pits or storage above a plot threshold reported as 500 m² (rule 78(4) and Appendix ढ, as amended in 2009; threshold [TBC]) without regard to the depth of the water table or the ground. This is why recharge, waterlogging, karst, mine-disturbed ground and groundwater quality are all assessment criteria. This Manual ranks projects by risk reduction per rupee so that limited budgets go first where most people are protected.

11.9 Integration with land-use planning

11.9.1 An approved BSMP shall be incorporated into the statutory land-use plan: the development plans approved by the State under s.19 of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam 1973, among them the Raipur Development Plan (Revised) 2021 and the plan that replaces it and the development plan of Durg–Bhilai; the plans of the Special Area Development Authorities under the same Adhiniyam, among them the Naya Raipur Development Plan-2031 and the Arpa special-area development plan 2033 where it still applies; and the layouts given development permission under them and under the Chhattisgarh Bhumi Vikas Niyam 1984. In particular:

- (a) regional facility sites, restored tank flood storage, and major flow paths shall be **reserved** in the plan;
- (b) the 1% AEP hazard map shall be shown, and Chapter 7 setbacks applied to it;
- (c) land in hazard classes H5–H6 in the 1% AEP event shall not be zoned for new habitable development; and
- (d) the open space of layouts within a basin should be located, where possible, to form part of the major flow paths or regional storage shown in the BSMP.

11.9.2 Where a land-use plan is revised before a BSMP is approved, the planning authority shall use the best available flood mapping and record the BSMP as pending.

Commentary. Chhattisgarh draws few water lines by rule, and the widest it has come from the Wetland Authority. Rule 50 of the Chhattisgarh Bhumi Vikas Niyam 1984 is still the rule on land unsuitable for development, but its Chhattisgarh text was not read: as made in 1984 it bars building within 9 m of the highest water mark, or, by a major watercourse, within 9 m of the average high flood mark or 15 m of its defined boundary, whichever is more, without defining either mark, and these lines are not confirmed for Chhattisgarh [TBC]. T&CP's checklist asks, for land next to a nala, canal or river, for the no-objection of WRD or the Revenue Department and for WRD's high flood point (2011); integrated townships may not be sited on the bank of a perennial river and may not count water sources in their area (Appendix ण, 2008 and 2018); and construction on the bed or edge of a reservoir or on a natural drain may not be regularised (regularisation Act 2002, s.7(1)). Since 2018 the site-inspection form

records whether a major nala is near the plot and its distance, for compliance with the Niyam, but no form or rule asks how a flood edge is drawn. Tanks recorded for nistar vest in the State (Land Revenue Code 1959, s.251), and encroachment on them is removed as unauthorised occupation (s.248), as at Baloda under the National Green Tribunal's order of 2024. The State Wetland Authority treats wetlands larger than 2.25 ha as deemed wetlands and bars works within 50 m of their highest flood level, the line it applied at Karbala and Narhaiya talabs in Raipur and at the Jhanjh wetland in Nava Raipur in 2025 (as reported), though no list of notified wetlands has been found. In Nava Raipur the Development Plan asks for river embankments safe against a 100-year flood and roads 0.5–1.0 m above the observed high flood level (NRDA, 2008). There is no floodplain zoning law, no notified flood zone and no WRD demarcation of the floodplain of any river in the State. These are watercourse, tank and wetland lines, not drainage-flood lines; the hydraulic corridor and setback zones of Chapter 7 are this Manual's proposal, and where the lines of the Niyam or the 50 m wetland line bind and are wider, they apply. A BSMP gives the development plans the 1% AEP drainage-flood map, and the buffer widths of Chapter 7, that they need. Using open space land as storage or flow path has no direct precedent; this Manual adopts it because the Chhattisgarh Bhumi Vikas Niyam 1984 already require community open space in layouts (rule 47; the Chhattisgarh percentage was not seen), at least 10% of an integrated township as gardens and playgrounds (Appendix ण, as amended) and rainwater harvesting under rule 78(4), and the Naya Raipur Development Plan-2031 asks that rainwater from the storm drainage system be stored in dug or natural lakes and depressions and makes rainwater harvesting mandatory for every development (§10.3.2), but no rule sets a detention or peak-runoff limit; storage placed in a layout's open space builds on these duties, and because that land still serves as open space, the storage must stay usable as one. Placing it on the flow path costs the developer nothing extra and gives the basin storage it would otherwise have to buy.

11.10 Consultation and publication

11.10.1 A draft BSMP shall be published for public comment for at least **60 days**, with public meetings in each affected local body.

11.10.2 After approval, the following shall be published as open data:

- the model files;
- the flood hazard maps;
- the data of Clause 11.5.1, except security-sensitive asset details; and
- the CIP.

They shall also be shown on a public map viewer.

Commentary. Publishing the maps lets buyers, lenders, insurers and residents see flood hazard before they build or buy. Publishing the model lets designers use the same boundary conditions as the Authority, so site designs and the basin plan stay consistent. This matches the publication approach of Chapter 10.

11.11 Approval, review and reporting

11.11.1 **Approval.** BSMPs shall be approved by the State Stormwater Technical Committee (Chapter 1), after review of the model and of the public comments.

11.11.2 **Review.** Each BSMP shall be reviewed:

- every **10 years**;

- within **2 years** of an event exceeding the 1% AEP design depth in the basin (Clause 2.14); and
- when the statutory land-use plan is revised.

11.11.3 **Annual report.** The Lead Authority shall publish an annual implementation report before each SW monsoon (by 31 May). The report shall show:

- CIP progress;
- the fee-in-lieu ledger (Clause 4.13.4);
- maintenance completed; and
- any flooding against the levels of service.

11.12 Interim arrangements

11.12.1 Until a BSMP is approved for a basin:

- (a) all site criteria of Chapters 4 to 8 apply in full;
- (b) regional detention and fee-in-lieu under Clause 4.13 are not available, except the holding of fees under Clause 4.5.6.3;
- (c) floor levels and setbacks use the best available mapping, and the methods of Chapters 6 and 7 where no mapping exists; and
- (d) public drain projects shall include a basin-level check that their outfall and downstream reaches can take the design flow under the Chapter 5 tailwater cases.

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Annexes and forms

Annex / Form	Content
11A	BSMP terms of reference template
11B	Basin lists and boundaries for Tier A bodies
11C	Depth–damage curves for Chhattisgarh building types
11D	BSMP data and model publication standard
Form 11-1	Annual implementation report

ANNEXES TO CHAPTER 1

Policy, Applicability and Adoption

These annexes support Chapter 1. They hold the lists and maps that define where the Manual applies, the model text of the Policy Supplement and of the legal instruments that would give the Manual effect, and the two forms an Applicant files under Chapter 1. The Manual is a draft; it is not adopted by any government or agency: model legal text is offered for consideration by a competent authority and is not in force, and the forms are draft forms. An annex marked "To be prepared" states the data it needs and the rule to use until it is published (Clause 1.11.5).

Annex 1A — List of Tier A authorities, with the boundaries of the Municipal Corporations, of the planning area of Raipur and of the special area of Nava Raipur Atal Nagar

Supports Clauses 1.6.1 to 1.6.5 of Chapter 1.

1A.1 Clause 1.6.1 defines Tier A by class of authority. Table 1A.1 sets out the classes, and the authorities named in the clause, as Chapter 1 defines them. Every authority not in Table 1A.1 is Tier B (Clause 1.6.2).

Table 1A.1 — Tier A authorities as defined in Clause 1.6.1

Item	Tier A authority	Clause
(a)	Raipur Municipal Corporation (RMC)	1.6.1(a)
(b)	Every Approving Authority within the planning area of Raipur under the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973, including the Directorate of Town and Country Planning where it grants development permission, RDA in its scheme areas, and the Municipal Corporations, Nagar Palika Parishads and Nagar Panchayats of the area	1.6.1(b)
(b)	NRDA, the Special Area Development Authority of Nava Raipur Atal Nagar under the 1973 Adhiniyam, in its special area	1.6.1(b)
(c)	Every other Municipal Corporation (Nagar Palik Nigam) constituted under the Municipal Corporation Act, 1956 as adapted in Chhattisgarh: Bilaspur, Durg, Bhilai, Ambikapur, Jagdalpur, Korba, Rajnandgaon and Raigarh (named by the CAG among the State's 14 Municipal Corporations), and Bhilai-Charoda, Risali, Birgaon, Dhamtari and Chirmiri (to be confirmed [TBC])	1.6.1(c)
(c)	The Directorate of Town and Country Planning and every development authority constituted under the 1973 Adhiniyam, where it grants development permission within the planning area of a Municipal Corporation	1.6.1(c)
—	A Tier B authority that opts into Tier A by council resolution, or that the Government notifies as Tier A	1.6.4

1A.2 Whatever the tier, the design-aid route and the alternatives of Clause 1.7 are not available in a flood-sensitive catchment (Clause 1.6.5; Annex 1B).

To be prepared. This annex will contain the full list, by name, of every Municipal Corporation and every development authority in the State, and of every Approving Authority within the planning area of Raipur and the special area of Nava Raipur Atal Nagar, with the date from which each is Tier A, and the boundaries of those areas as a map and as a GIS layer. It needs the notified planning areas of the development plans under the 1973 Adhiniyam, among them the Raipur Development Plan (Revised) 2021, approved on 5 April 2008, and the Durg–Bhilai plan, and the special area of NRDA (T&CP, the Housing and Environment Department, NRDA); the scheme areas of RDA, whose constitution was not verified for this Manual, and of the other development authorities [TBC]; the current list and boundaries of the Municipal Corporations, Nagar Palika Parishads and Nagar Panchayats (the Urban Administration and Development Department; the CAG gives 14 Municipal Corporations and names nine of them, and the other five were not verified [TBC]); and the district and tehsil boundaries (Revenue and Disaster Management Department, Survey of India), many of which have changed since 2012 with the creation of new districts. Until it is published, the tier is decided by the classes in Clause 1.6.1 and recorded on Form 1-1, and the Authority confirms it at the pre-application stage (Clause 1.5.9).

Table 1A.2 — Register of Tier A authorities (layout)

No.	Authority	Class (Municipal Corporation / Nagar Palika Parishad / Nagar Panchayat / Special Area Development Authority / development authority / Directorate of Town and Country Planning / other)	District	Basis (Clause 1.6.1(a), (b), (c) or 1.6.4)	Tier A from (date)	Notification or resolution reference
1						
2						
3						

1A.3 The Municipal Corporation, Raipur planning-area and Nava Raipur special-area boundary map shall be published as open data in a machine-readable format (Clause 10.17.4), with the date of the notification it shows.

Annex 1B — Maps of flood-sensitive catchments

Supports Clauses 1.4, 1.6.5 and 1.10.3(d) of Chapter 1, and Clauses 4.5.3 and 4.5.4 of Chapter 4.

1B.1 The flood-sensitive catchments are listed in Clause 4.5.3.2. Table 1B.1 repeats that list. The maps of this annex and the boundaries of Annex 4A are the same data set and shall be issued together.

Table 1B.1 — Flood-sensitive catchments listed in Clause 4.5.3.2

Item	Catchment	Extent as described in Chapter 4
(a)	Raipur's low colonies on the Kharun nalas	The catchments of the nalas and drains of Raipur and Birgaon that discharge to the Kharun, with the talabs on them and the low-lying colonies that drain to them. The city's sewage and storm water reach the Kharun through some eight large nalas (as reported), now intercepted by diversion weirs for the treatment plants at Chandanidih, Kara and Nimora (CECB, 2021); RMC recorded 11 road stretches that flood two or three times a year and drain coverage of 49% (RMC, 2015); in July 2026 Mowa, Shankar Nagar, Tatibandh, Chaubey Colony, where five ponds overflowed into homes, and the Fafadih and Gudhiyari underpasses were flooded (as reported). The Kharun at Pathardihi has a highest flood level of 281.68 m (29 August 2020) and no published warning or danger level (CWC data as transcribed by SANDRP)

Item	Catchment	Extent as described in Chapter 4
(b)	Bilaspur on the Arpa	The land of Bilaspur that drains to the Arpa below the Arpa Bhaisajhar barrage (full reservoir level 302.000 m, WRD) and the Kharang reservoir at Khutaghat, including Sarkanda and the other low colonies of the river's banks, which flooded in September 2025 (as reported); a fall reported as 28 cm in 24 hours flooded the city in July 2026, and another report of 415.6 mm for the same storm is unresolved (as reported); no gauge level for the Arpa at Bilaspur was found [TBC]
(c)	Durg–Bhilai on the Seonath nalas	The land of Durg, Bhilai, Bhilai-Charoda and Risali that drains to the Seonath (Shivnath) through the Shankar and Pulgaon nalas and the other nalas of the twin cities, including the township drains of the Bhilai Steel Plant, whose blockage at the Junwani and Kosanala culverts was the subject of a warning by the Bhilai Municipal Corporation in July 2025 (as reported); the NGT ordered Durg's treatment plants on the Shankar and Pulgaon nalas finished within six months in January 2026 (as reported); the Mongra barrage upstream on the Seonath has a full reservoir level of 337.150 m (WRD); no gauge level for the Seonath at Durg was found [TBC]
(d)	Korba below Bango and Darri	The land of Korba that drains to the Hasdeo below the Minimata Bango dam (spillway capacity 24,000 m³/s) and the Hasdeo barrage at Darri (19,820 m³/s) (CWC, NRLD 2009); releases of about 30,000 cusecs through six gates of Bango and 20,000 cusecs at Darri flooded houses at Motisagarpada, Seetamani and Imliduggu in August 2024, and the Hasdeo set new highest levels at Katghora, 313.58 m, and Bangodam, 303.5 m, in July and August 2026 (as reported); the ash dykes of the power plants and the mine voids beside the river are flood hazards to map under Chapter 5
(e)	Raigarh–Sarangarh in the Hirakud backwater	The land of Raigarh and Sarangarh that drains to the Kelo, the Mand or the Mahanadi within reach of the backwater of the Hirakud reservoir in Odisha, in which 33 villages of Chhattisgarh are reported to be partly submerged; the Kelo overflowed in Raigarh in August 2019, and the Mand at Kurubhata reached 220.4 m and the Mahanadi at Surajgarh 201.06 m in August 2020 (as reported); the Kelo dam above Raigarh has a full reservoir level of 233.000 m (WRD); the extent of the backwater within the State is not mapped [TBC]
(f)	Jagdalpur on the Indravati	The land of Jagdalpur that drains to the Indravati, whose CWC site has a warning level of 539.49 m and a danger level of 540.80 m (CWC advisories of 2019, seen as search extracts) and reached a new highest level of 544.891 m on 26 September 2026, above the 544.68 m of 9 July 1973 (as reported); the river rises in Odisha, where dams on its upper reaches regulate the flow reaching the city, and their releases were not researched for this Manual [TBC]
(g)	Notified catchments	Any catchment notified under Clause 4.5.4

To be prepared. This annex will contain a map of each catchment in Table 1B.1 at a scale fit for deciding whether a plot lies inside it, and the same boundaries as a GIS layer. It needs catchment boundaries drawn from a digital elevation model and the drain network (the Municipal Corporations of Raipur, Birgaon, Bilaspur, Durg, Bhilai, Bhilai-Charoda, Risali, Korba, Raigarh and Jagdalpur, the Bhilai Steel Plant for its township, Survey of India); the city drains and nalas with their outfalls, the interception weirs of the new treatment plants and their levels on a stated datum (the Municipal Corporations); the full reservoir levels, maximum water levels and gate-operation rules of Gangrel, Minimata Bango, the Hasdeo barrage at Darri, Kharang, the Arpa Bhaisajhar and Mongra barrages and the Kelo dam, and the levels of the Hirakud reservoir and its backwater within the State (WRD, and through it the Government of Odisha); the CWC levels, with warning and danger levels where they exist, on the Kharun at Pathardihi, the Hasdeo at Bangodam and Katghora, the Mand at Kurubhata, the Mahanadi at Surajgarh and the Indravati at Jagdalpur, and any gauge on the Arpa at Bilaspur and the Seonath at Durg; the talabs recorded in the revenue records (the Collectors); and the land flooded in 2007, 2020, 2024, 2025 and 2026 (the Collectors, CGSDMA, the Municipal Corporations). Nava Raipur Atal Nagar, whose streams and lakes NRDA planned as a storage network; Rajnandgaon on the Shivnath; Dhamtari and Rajim below Gangrel; Ambikapur; and the towns of the Bastar division flooded in August 2025 (as reported) are candidates for notification under Clause 4.5.4 once their catchment boundaries and flood record are documented [TBC]. The floodplains of the rivers, the 50 m line that the State Wetland Authority applies around wetlands and the village tanks vested in the State are governed by Chapter 7 and by the instruments that bind there rather than by this list. Until it is published, the Authority decides at the pre-application stage whether a site lies in a listed catchment, from the best boundary available, and the decision and its basis are recorded on Form 1-1 and in the drainage report (Clauses 1.5.9 and 1.11.5).

Table 1B.2 — Attributes of the flood-sensitive catchment layer (layout)

Field	Content	Unit or format
catchment_id	Unique identifier	text
name	Catchment name	text
basis	Clause 4.5.3.2 item (a) to (f), or the notification under Clause 4.5.4	text
notification_ref	Gazette notification number, where notified	text
notified_on	Date of notification	date
applies_from	Date from which the rule applies to applications (Clause 4.5.4.2, step 5)	date
boundary_source	Source and date of the boundary	text
area_km2	Catchment area	km²

Annex 1C — Policy Supplement: text of alternatives S-1 to S-5 and model council resolution

Supports Clauses 1.6.3, 1.6.5 and 1.7.1 to 1.7.3 of Chapter 1.

Model text for consideration by a competent authority. Not in force. The text below shows how the Policy Supplement of Clause 1.7 could be written. It has no effect unless and until it is issued by the Government and adopted by an Approving Authority under Clause 1.7.3.

Part 1 — General provisions

P1.1 **Scope.** This Supplement sets out alternatives S-1 to S-5 to the requirements of the Chhattisgarh Stormwater Design Manual (the Manual) named in each alternative. An alternative applies within the area of an Approving Authority (the Authority) only from the date on which the Authority adopts it under P1.3.

P1.2 **No lower criterion.** No alternative lowers any criterion of the Manual. S-1 and S-2 are simplified methods for meeting the same criterion (Clause 1.7.3(a)).

P1.3 **Adoption.**

- (a) A Tier B authority adopts an alternative by council resolution in the form of Part 3, and sends a copy to the State Stormwater Technical Committee (the Committee) within 30 days of the resolution.
- (b) A Tier A authority adopts an alternative only after the Committee has concurred in writing.
- (c) The Authority publishes the alternatives it has adopted on its website and in the register of Clause 10.17.

P1.4 **Retention.** No alternative replaces the retention requirement (Annex 2E depth), except where fee-in-lieu under S-5 funds equivalent retention in the same basin (Clause 1.7.3(d)).

P1.5 **Tables.** The tables used by S-1, S-2 and S-3 are those issued by the Committee, and are reviewed with Chapter 2 of the Manual (Clauses 1.7.3(e) and 2.14). An alternative that depends on a table cannot be used until that table is issued.

P1.6 **Record.** An Applicant who uses an alternative states it on Form 1-1 and in the drainage report, with the resolution or concurrence under which it was adopted.

Part 2 — The alternatives

S-1 — Design-aid route for storage

S-1.1 *Replaces:* storage sized by a hydrograph method under Chapter 3.

S-1.2 *Alternative:* retention and detention volumes may be taken from the unit-storage tables issued for Annex 4B (m³ per ha, by rainfall zone, hydrologic soil group and impervious fraction) and the compliance calculator, by the method of Clause 4.14.2.

S-1.3 *Eligibility:* sites of 2 ha or less outside flood-sensitive catchments, and of 0.5 ha or less within them (Table 1.2; Clause 4.14.1).

S-1.4 *Conditions:* the post-development imperviousness used shall be that shown on the approved plans; the outlet, spillway, RWH accounting and safety provisions of Clause 4.14.2 apply; the compliance calculator may be the sole calculation (Table 1.1).

S-2 — Permissible site discharge (PSD)

S-2.1 *Replaces:* the release rate $Q_{post}(T) \le 0.9 \times Q_{pre}(T)$ computed for the site (Clause 2.11.2).

S-2.2 *Alternative:* the release rate for each AEP may be taken as the tabulated unit-area release rate for the site's zone and AEP, multiplied by the site area. The tabulated rates are derived from the $0.9 \times Q_{pre}$ rule (Table 1.2).

S-2.3 *Eligibility:* any site outside a flood-sensitive catchment.

S-2.4 *Conditions:* storage is then sized to hold every duration of Clause 2.5.2 at the PSD; drawdown, minimum orifice and spillway rules of Chapter 4 apply unchanged.

S-3 — Fixed-width waterbody setback

S-3.1 *Replaces:* the waterbody setback extended to the mapped 1% AEP flood extent plus margin (Chapter 7).

S-3.2 *Alternative:* where no 1% AEP extent is mapped, the setback shall be the fixed width for the class of waterbody in the table of Clause 7.4.6 (and Annex 7B), measured as in Clause 7.4.3.

S-3.3 *Eligibility:* waterbodies with no mapped 1% AEP extent.

S-3.4 *Conditions:* no setback under this alternative is less than the Zone 1 minimum of Clause 7.4.7 (Clause 1.10.3(c)); the no-fill zone of Chapter 7 applies; where a 1% AEP extent is later mapped and adopted, the mapped setback applies to new applications.

S-4 — Downstream screening checklist

S-4.1 *Replaces:* downstream analysis by hydraulic model under Chapter 5.

S-4.2 *Alternative:* the Applicant completes a screening checklist of downstream capacity and known flooding, covering at least the conditions of Clause 5.3.4: that the receiving public drain has documented capacity for its minor-system design flow (Chapter 6), and that no flood-prone location is recorded within 500 m downstream. A hydraulic model under Chapter 5 is required only where the screen fails.

S-4.3 *Eligibility:* sites of less than 1 ha, outside flood-sensitive catchments, not discharging to a tidal outfall.

S-4.4 *Conditions:* the checklist is signed by the registered professional who signs the drainage report (Clause 10.6); flood-prone locations are those recorded by the local body, the Collector and the District Disaster Management Authority, CGSDMA, the district flood control rooms, or WRD (Clause 5.3.3(d)).

S-5 — Fee-in-lieu toward regional storage

S-5.1 *Replaces:* on-site detention under Chapter 4.

S-5.2 *Alternative:* all or part of the detention requirement may be met by a fee-in-lieu toward a regional detention facility or restored talab or tank storage identified in the basin plan (Clause 4.13.1).

S-5.3 *Eligibility:* sites in a basin for which a basin plan has been notified (Table 1.2).

S-5.4 *Conditions:*

- (a) the conditions of Clause 4.13.2 apply, including interim on-site detention until the regional facility or restored tank is operational, and retention and WQv treatment on site;
- (b) the fee is computed under Clause 4.13.3 and held in the ring-fenced basin account;
- (c) the volume credited and the fee are entered in the public ledger of Clause 4.13.4; and
- (d) in a flood-sensitive catchment, fee-in-lieu is available only as allowed by Clause 1.10.3(d).

Part 3 — Model council resolution

> **Model text for consideration by a competent authority. Not in force.**

Resolution No. _____ of _____ (name of Municipal Corporation (Nagar Palik Nigam) / Nagar Palika Parishad / Nagar Panchayat), dated _____

Subject: Adoption of alternatives under the Policy Supplement to the Chhattisgarh Stormwater Design Manual.

The Council, having considered the report of the Municipal Commissioner / Chief Municipal Officer dated _____, and noting that:

- (i) the Chhattisgarh Stormwater Design Manual and its Policy Supplement have been given effect by _____ (instrument and date);

- (ii) this local body is a Tier B / Tier A authority under Clause 1.6 of the Manual; and
- (iii) for a Tier A authority, the State Stormwater Technical Committee concurred on _____ (reference _____),

resolves:

1. to adopt the following alternatives of the Policy Supplement within its area, under the conditions of Clause 1.7.3 of the Manual:

Table 1C.1 — Alternatives adopted by this resolution

Alternative	Adopted (Yes / No)	Area to which it applies (whole area, or wards / zones)	Applies from (date)
S-1 Design-aid route			
S-2 Permissible site discharge			
S-3 Fixed-width setback			
S-4 Downstream screening checklist			
S-5 Fee-in-lieu			

2. that no adopted alternative shall apply within a flood-sensitive catchment except as the Manual allows (Clause 1.6.5);
3. that a copy of this resolution be sent to the State Stormwater Technical Committee within 30 days;
4. that the adopted alternatives be published on the local body's website and entered in the register of Clause 10.17; and
5. that this resolution be reviewed when the tables of Clause 1.7.3(e) are revised.

Mayor / President: _____ Municipal Commissioner / Chief Municipal Officer: _____ Date: _____

Annex 1D — Model CGBVN 1984 amendment and model ULB bye-law

Supports Clauses 1.3.1, 1.3.4, 1.11 and 1.12 of Chapter 1.

> **Model text for consideration by a competent authority. Not in force.** The text shows one way the instruments of Clause 1.3.1 could be drafted. Rule and section numbers are left blank.

> **Drafting note D1D.1.** The rule numbers of the Chhattisgarh Bhumi Vikas Niyam 1984 at which the new rule and the conditions would be inserted, and the enabling sections of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973 and the municipal and panchayat Acts, are to be confirmed by the Government's legal advisers. None is stated here.

Part A — Model amendment to the Chhattisgarh Bhumi Vikas Niyam 1984

A.1 **Short title and commencement.**

- (1) These rules may be called the Chhattisgarh Bhumi Vikas (Stormwater Management) Amendment Rules, _____.

- (2) They shall come into force on the date six months after their publication in the Chhattisgarh Gazette (Rajpatra) (Clause 1.11.1).

A.2 **Insertion of a new rule.** After rule [_____] of the land development rules made by the State Government under section 85(1) read with section 24(3) of the Chhattisgarh Nagar Tatha Gram Nivesh Adhiniyam, 1973, the following rule shall be inserted:

> **Rule [_____]. Stormwater management.**
>
> (1) *Definitions.* In this rule, "the Manual" means the Chhattisgarh Stormwater Design Manual as notified by the Government, with its annexes and amendments; other words have the meanings given in Clause 1.4 of the Manual.
>
> (2) *Application.* This rule applies to every development for which planning permission, building permit or layout approval is required under these rules.
>
> (3) *Runoff control.* Where a development, over its common plan of development, involves land disturbance of 0.1 ha or more, or new or replaced impervious area of 500 m² or more, the development shall comply with Chapters 4 and 5 of the Manual (retention, detention, downstream and tailwater analysis).
>
> (4) *Provisions for every development.* Every development, whatever its size, shall comply with Clause 1.5.4 of the Manual: no sewage connection to a storm drain; floor levels; waterbody setbacks and the no-fill zone; and no obstruction of a drainage path.
>
> (5) *Exemptions.* A single dwelling on its own plot below the thresholds of sub-rule (3), and linear road resurfacing that adds no impervious area, are exempt from sub-rule (3) but not from sub-rule (4) or the rainwater harvesting provisions of these rules.
>
> (6) *Conditions of approval.* The competent authority shall not grant:
>
> - (a) planning permission or layout approval, unless a preliminary drainage report under the Manual has been accepted;
> - (b) a building permit, or permission to commence, unless the final drainage design has been accepted; or
> - (c) a completion certificate, unless the as-built certificate has been accepted and the maintenance declaration has been registered, or the public assets have been handed over,
>
> for a development to which sub-rule (3) applies (Manual, Clause 10.3).
>
> (7) *Certification.* Drainage reports and designs shall be signed by a registered professional, and, where Clause 10.6.1 of the Manual so requires, by an empanelled Stormwater Design Professional.
>
> (8) *Rainwater harvesting.* Nothing in this rule alters the rainwater harvesting provisions of these rules.
>
> (9) *Variances and waivers.* The competent authority may grant a variance or waiver only on the grounds and by the procedure of Clause 1.10 of the Manual, and shall publish each decision.
>
> (10) *Transition.* Applications filed complete before the commencement of this rule, and Tier B authorities, are governed by Clause 1.11 of the Manual.

Part B — Model ULB bye-law

B.1 **Title and commencement.** These bye-laws may be called the _____ (name of local body) Stormwater Management Bye-laws, _____. They come into force on _____.

B.2 **Adoption of the Manual.** The Chhattisgarh Stormwater Design Manual, as notified by the Government, applies to:

- (a) every development approved by the local body; and
- (b) every drainage, road, culvert, waterbody and flood-mitigation work executed by the local body, including its own drains and roads (Clause 1.3.1(b)).

B.3 **Public works.** Every detailed project report and contract for a work under B.2(b) shall require compliance with the Manual, from the dates in Clause 1.11 (Clause 1.3.4).

B.4 **Tier and alternatives.** The local body is a Tier _____ authority under Clause 1.6 of the Manual. The alternatives of the Policy Supplement adopted by the local body are listed in the Schedule, with the resolution under which each was adopted.

B.5 **Submittals and review.** Submittals shall be made in the stages of Clause 10.3 of the Manual and reviewed within the periods of Clause 10.7.

B.6 **Maintenance, inspection and enforcement.** Maintenance declarations, inspections, handover of public assets and enforcement shall follow Clauses 10.10 to 10.15 of the Manual.

B.7 **Publication.** The local body shall publish the registers and reports of Clause 10.17 of the Manual.

B.8 **Review.** These bye-laws shall be reviewed whenever the Manual is amended under Clause 1.12.

Table 1D.1 — Schedule: alternatives adopted (layout)

Alternative	Resolution number and date	Committee concurrence (Tier A only)	Applies from	Area

---

Form 1-1 — Applicability Determination

Supports Clauses 1.5.1 to 1.5.9 and 1.6 of Chapter 1, and Clause 10.3 of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Lodged at the pre-application meeting (Stage 0) or with the preliminary drainage report (Stage 1), as Clause 10.3.2 requires.

Table 1F.1 — Form 1-1: Applicability Determination

No.	Item (clause)	Entry
	A. Applicant and site	
A1	Applicant (owner, developer or department)	_____
A2	Site address and survey numbers	_____
A3	Site coordinates (latitude, longitude)	_____
A4	Approving Authority (Clause 1.4)	_____
A5	Tier of the Authority: A / B (Clause 1.6; Annex 1A)	_____
A6	Approval sought: planning permission / building permit / layout approval / public work (DPR)	_____
A7	Type of development: building / layout / sub-division / road / paving / land-use change / other (Clause 1.4)	_____
A8	Stage at which this form is lodged: Stage 0 / Stage 1 (Clause 10.3)	_____
	B. Coverage (Clause 1.5.1)	
B1	Development requiring approval under the Chhattisgarh Bhumi Vikas Niyam 1984 — 1.5.1(a): Yes / No	_____
B2	Public work by a ULB, State department, board or State undertaking — 1.5.1(b): Yes / No	_____
B3	Work by a Central agency needing the Authority's approval or no-objection — 1.5.1(c): Yes / No	_____
	C. Areas over the common plan of development (Clauses 1.4, 1.5.5)	
C1	Phases, plots and sub-divisions included (one layout approval or one ownership)	_____
C2	Total site area (ha)	_____
C3	Land disturbance (ha)	_____
C4	Existing impervious area (m²)	_____
C5	New impervious area (m²)	_____
C6	Replaced impervious area, removed to subgrade and rebuilt (m²)	_____
C7	New plus replaced impervious area, C5 + C6 (m²)	_____
C8	Plot area for the RWH mandate (m²) (Clause 1.5.3)	_____
	D. Thresholds and exemptions	
D1	Land disturbance 0.1 ha or more — 1.5.2(a): Yes / No	_____
D2	New or replaced impervious area 500 m² or more — 1.5.2(b): Yes / No	_____
D3	Redevelopment (existing impervious cover on the site) — 1.5.6: Yes / No	_____
D4	Constrained redevelopment route of Clause 4.5.6 proposed: Yes / No	_____

No.	Item (clause)	Entry
D5	Linear public work: added impervious area per drainage catchment crossed given in Table 1F.2 — 1.5.7: Yes / No / not applicable	_____
D6	Exemption claimed — 1.5.8(a) single dwelling below the thresholds / 1.5.8(b) resurfacing with no added impervious area / none	_____
D7	RWH mandate applies (CGBVN 1984, Rule 78(4): building on a plot above the threshold, reported as 500 m² [TBC]; government building with a roof of more than 1,500 sq ft (new) or 5,000 sq ft (existing) (General Administration Department order of 31 May 2016)) — 1.5.3: Yes / No	_____
	E. Site conditions	
E1	Site in a flood-sensitive catchment (Annex 1B; Clause 4.5.3): No / Yes — name	_____
E2	Source of the catchment boundary used	_____
E3	1% AEP level mapped or determinable for the site (Clause 1.5.4(b)): Yes / No; source	_____
E4	Waterbody on or adjoining the site (Clause 1.5.4(c); Chapter 7): No / Yes — name	_____
E5	Natural drainage path or major-system flow path crossing the site (Clause 1.5.4(d)): No / Yes	_____
E6	Site drains to a tidal outfall (Clause 10.3.2(d)): Yes / No	_____
E7	Variance, waiver, regional facility or fee-in-lieu proposed (Clause 10.3.2(c)): No / Yes — Form 1-2 attached	_____
	F. Determination	
F1	Result: (i) Chapters 4 and 5 apply, with Clause 1.5.4 and the RWH mandate; (ii) Clause 1.5.4 and the RWH mandate only (below thresholds); (iii) exempt under 1.5.8, Clause 1.5.4 and the RWH mandate still apply	_____
F2	Stage 0 mandatory under Clause 10.3.2: Yes (state item (a) to (d)) / No	_____
F3	Storage method: hydrograph method (Chapter 3) / design-aid route S-1 (Clause 1.7, where adopted and eligible)	_____
F4	Other Policy Supplement alternatives relied on (S-2 to S-5), with the adopting resolution	_____
F5	Tier B phased start applies (Clause 1.11.3): Yes / No	_____
	G. Declaration	
G1	I declare that the information above is correct and covers the whole common plan of development. Applicant: name, signature, date	_____
G2	Prepared by (registered professional, Clause 1.4): name, registration number, signature, date	_____
	H. For the Authority (Clause 1.5.9)	
H1	Determination confirmed / corrected	_____
H2	Corrections and reasons	_____
H3	Officer: name, designation, signature, date	_____

Table 1F.2 — Form 1-1: added impervious area by drainage catchment, linear works (Clause 1.5.7)

Drainage catchment crossed	Chainage from – to	Added impervious area (m²)	500 m² or more (Yes / No)	Storage location: right of way / regional facility

Form 1-2 — Variance / Waiver Request

Supports Clause 1.10 of Chapter 1.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Lodged with the preliminary drainage report (Clause 1.10.4(a)), with one form for each requirement concerned.

Table 1F.3 — Form 1-2: Variance / Waiver Request

No.	Item (clause)	Entry
	A. Applicant and site	
A1	Applicant	_____
A2	Site address and coordinates	_____
A3	Form 1-1 reference and date	_____
A4	Tier of the Authority: A / B	_____
A5	Site area (ha)	_____
A6	Site in a flood-sensitive catchment: Yes / No	_____
	B. Request	
B1	Type: variance from a numeric requirement (1.10.1) / waiver of Chapter 4 detention (1.10.2)	_____
B2	Clause of the Manual concerned	_____
B3	Value required by the Manual	_____
B4	Value or arrangement proposed	_____
	C. Matters that cannot be varied or waived (Clause 1.10.3) — confirm that the request does not concern any of them	
C1	(a) a sewage connection to a storm drain	Not concerned: _____
C2	(b) the lowest habitable floor level of Chapter 6	Not concerned: _____
C3	(c) a waterbody setback below the Zone 1 minimum of Clause 7.4.7, or fill or construction in the no-fill zone of Chapter 7	Not concerned: _____
C4	(d) retention in a flood-sensitive catchment, except by fee-in-lieu under Clause 1.7	Not concerned: _____
	D. Variance: grounds (Clause 1.10.1(a))	
D1	Site condition: high groundwater table / contaminated soil / rock / site too small for the required facility / other	_____
D2	Evidence (tests, surveys, plans), with references	_____
D3	Options assessed and why each is impracticable	_____
	E. Variance: equal performance or make-up (Clause 1.10.1(b))	
E1	Downstream point used (Chapter 5)	_____
E2	Performance at that point with full compliance and with the variance (peak flows, water levels)	_____

No.	Item (clause)	Entry
E3	Or: shortfall made up by fee-in-lieu or off-site works in the same catchment — volume (m³), works or fee	_____
	F. Waiver of detention: grounds (Clause 1.10.2)	
F1	Site discharges directly to the sea / to a tidal reach with no flood-prone property downstream	_____
F2	Analysis showing that undetained flow does not worsen the outfall-closed condition of Chapter 5 — reference	_____
F3	Retention still provided in full: Yes	_____
	G. Attachments (Clause 1.10.4(a))	
G1	Report certified under Clause 10.6	_____
G2	Preliminary drainage report	_____
	H. Certification and declaration	
H1	Certified by (registered professional; empanelled professional where Clause 10.6.1 applies): name, registration or empanelment number, signature, date	_____
H2	Applicant: name, signature, date	_____
	I. For the Authority (Clause 1.10.4)	
I1	Referred to the State Stormwater Technical Committee (Tier B, site 1 ha or more): Yes / No; date	_____
I2	Decision: granted / granted with conditions / refused	_____
I3	Reasons and conditions	_____
I4	Entry number in the public register (Clause 10.17)	_____
I5	Note: a variance runs with the approval and lapses if the approval lapses (Clause 1.10.4(d))	—
I6	Officer: name, designation, signature, date	_____

ANNEXES TO CHAPTER 2

Design Rainfall and Design Storms

These annexes support Chapter 2. Annexes 2A to 2E will hold the statewide rainfall tables that Chapter 2 refers to: the benchmark checks and zone map, sub-daily ratios, grid-to-point factors, temporal patterns and retention depths. Each needs analysis of NIH-DHAARA and IMD records that has not yet been done for this Manual, so each is marked "To be prepared" and gives its intended layout and the rule to use until it is published (Clause 1.11.5). Form 2-1 is a draft form for the Design Rainfall Sheet that every hydrologic submittal must include (Clause 2.13). The Manual is a draft; it is not adopted by any government or agency.

Annex 2A — NIH-DHAARA vs IMD station and IMD gridded checks at benchmark points; zone map

Supports Clauses 2.3.1 to 2.3.5 of Chapter 2.

To be prepared. This annex will compare, at benchmark points across the State, the NIH-DHAARA design depths with frequency analyses of long IMD daily station records and of IMD gridded daily data (converted to point values with Annex 2C), for 24 hours and longer and for the AEPs of Table 2.1. It will also contain the map of rainfall zones used by Annexes 2B, 2C and 2E and by the Tier B tables of Chapter 4. It needs NIH-DHAARA values at the benchmark coordinates (NIH), IMD daily and recording-gauge station records and IMD gridded rainfall (IMD), State rain-gauge records (the district and tehsil rain gauges whose readings IMD tabulates in the *Climate of Chhattisgarh*, the gauges of WRD at its dams and barrages, and the agrometeorological observatory of Indira Gandhi Krishi Vishwavidyalaya at Raipur [TBC]), and district and basin boundaries (Survey of India, WRD). Until it is published, design depths are taken from NIH-DHAARA under Clause 2.3.1, the cross-checks of Clause 2.3.3 are reported on Form 2-1, and the zone of a site is the one agreed with the Authority and recorded on Form 2-1.

Table 2A.1 — Benchmark comparison of design depths (layout)

Benchmark point	IMD station ID	Latitude, longitude	Years of record	Duration (h)	AEP	NIH-DHAARA (mm)	IMD station analysis (mm)	IMD gridded, point value (mm)	NIH-DHAARA / IMD station	Remarks
				24						
				48						
				72						

2A.1 The zone names below are the working names used in the Tier B table template of Chapter 4 (Clause 4.14.2). Their boundaries, and whether further zones are needed, are to be set by the analysis of this annex.

Table 2A.2 — Rainfall zones (layout)

Zone (working name)	Extent (districts, basins)	Representative stations	Notes
CG-Z1 Mahanadi plain	The Chhattisgarh plain and its northern and southern fringes in CWC subzone 3(d), the Mahanadi and Brahmani basins (Clause 2.3.6(b)); the Wainganga side of Rajnandgaon and Khairagarh lies in 3(f) and the Narmada fringe of Kawardha in 3(c); catchments that cross a zone line take the higher zone value	Raipur (Obsy) 42875 (daily record 1901–2012), Raipur (Mana) (A) 11203 (1979–2012), Durg 42876 (1969–2012), Rajnandgaon 11204 (1981–2008), Champa 42783 (1945–2012), Raigarh 42884 (1950–2012), Kanker 42953 (1927–2008), Pendra Road 42779 (1903–2012)	District normals about 1,010–1,390 mm (Kabirdham 1,011.2, Durg 1,061.5, Bilaspur and Balod 1,110.5, Raipur 1,129.3, Dhamtari 1,187.6, Mahasamund 1,205.7, Raigarh 1,321.2, Korba 1,354.1 mm; IMD, 1961–2010). FER 3(d) 50-yr 24-h about 200–360 mm, from a closed cell of less than 200 mm over Mungeli and Kawardha to about 360 mm at Gariaband, with a closed cell of more than 440 mm on the Odisha border; 1-h ratio 0.39, the highest in the State. Records that qualify under Clause 2.3.6(h) and raise the 100-yr depth above the plates: Raipur (Obsy) 42875, 370.3 mm (4 Aug 1910); Korba (23 years), 358.0 mm (12 Aug 2004); Champa 42783, 347.2 mm (26 Jul 1947) (IMD). Higher district-gauge records whose length of record is not confirmed, among them Rajnandgaon 392.0 mm (21 Sep 1965) and Durg 337.0 mm (15 Jun 2004), are excluded pending confirmation: each is admitted to the check of Clause 2.3.6(h) once IMD confirms at least 20 years of daily record at its gauge [TBC]. NIH-DHAARA 50-yr 24-h close to the plates at Raipur, Rajnandgaon, Bilaspur and Raigarh, high at Durg–Bhilai, Korba, Nava Raipur and Mahasamund
CG-Z1 Mahanadi plain: CG-A Raipur–Durg–Bhilai–Rajnandgaon plain (Seonath–Kharun, limestone)	Raipur, Birgaon, Nava Raipur Atal Nagar, Abhanpur, Durg, Bhilai, Bhilai-Charoda, Risali, Kumhari, Rajnandgaon, Dongargarh, Bemetara, Balod, Dhamdha, Simga and Baloda Bazar; boundaries [TBC]	Raipur (Obsy), Raipur (Mana), Durg, Rajnandgaon; the NRDA chart for Nava Raipur Atal Nagar (cross-check only, Clause 2.3.6(g))	FER 50-yr 24-h about 240–280 mm at Durg and Bhilai and 280–320 mm at Raipur, Nava Raipur and Rajnandgaon. Records: Raipur (Obsy) 370.3 mm (4 Aug 1910; the same storm gave Kurud 381.0 mm and Arang 318.0 mm), qualifying under Clause 2.3.6(h); Raipur (Mana) 273.0 mm (30 Jun 2007) and Durg (Obsy) 260.4 mm (13 Aug 2006), below the plates; Rajnandgaon 392.0 mm (21 Sep 1965), Dongargarh 359.4 mm (1 Aug 1959), Durg 337.0 mm (15 Jun 2004) and Bhilai 288.3 mm (19 Aug 1939), district gauges excluded pending confirmation: each is admitted to the check of Clause 2.3.6(h) once IMD confirms at least 20 years of daily record at its gauge [TBC] (IMD). The karstified, cavernous Chandi and Charmuria limestone of the Raipur–Durg–Rajnandgaon belt; kanhar and dorsa on the plain, matasi and bhata on the uplands; critical groundwater blocks Dharsiwa (Raipur), Gurur, Nawagarh, Bemetara and Berla (CGWB, 2024). Receiving waters: the Kharun (Pathardihi highest level 281.68 m, 2020; no warning or danger level), the Seonath (Mongra barrage full reservoir level 337.150 m), Raipur's talabs and the lakes of Nava Raipur
CG-Z1 Mahanadi plain: CG-B Bilaspur–Mungeli–Kawardha (Arpa–Maniari–Hamp)	Bilaspur, Takhatpur, Bilha, Masturi, Mungeli, Lormi, Kawardha and Pandariya; the Narmada fringe of Kawardha lies in subzone 3(c); boundaries [TBC]	Pendra Road (the nearest long record, about 90 km north); the Bilaspur district gauge, whose length of record is to be obtained from IMD [TBC]	The driest part of the plain (Kabirdham 1,011.2 mm) and the lowest band of the FER plates: 50-yr 24-h about 200–240 mm at Bilaspur, at the edge of a closed cell of less than 200 mm. Records: Bilaspur 277.0 mm (29 Jun 2005), Pendra Road 262.1 mm (18 Aug 1953) (IMD); a fall reported as 28 cm, and as 415.6 mm, in 24 hours at Bilaspur in July 2026 is unresolved (as reported) [TBC]. Limestone and shale, with alluvium up to about 30 m at Bilaspur; Takhatpur and Belha semi-critical (CGWB, 2024). Receiving waters: the Arpa (Bhaisajhar barrage, full reservoir level 302.000 m; Kharang, spillway 994 m^3/s), the Maniari (Maniyari, highest level 236.23 m, 2022) and the Hamp; Kopra Jalashay (Ramsar)

Zone (working name)	Extent (districts, basins)	Representative stations	Notes
CG-Z1 Mahanadi plain: CG-C Korba–Janjgir–Champa–Raigarh (Hasdeo–Mand–Kelo coalfields)	Korba, Katghora, Dipka, Gevra, Kusmunda, Champa, Janjgir, Akaltara, Sakti, Raigarh, Kharsiya, Gharghoda, Tamnar and Sarangarh; boundaries [TBC]	Champa, Raigarh; the Korba district gauge (23 years)	FER 50-yr 24-h about 240–280 mm at Korba and Champa and about 320 mm at Raigarh (inferred [TBC]). Records: Korba 358.0 mm (12 Aug 2004) and Champa 347.2 mm (26 Jul 1947), qualifying under Clause 2.3.6(h); Raigarh (IMD station 42884) 315.2 mm (25 Aug 1970), below the plate; Janjgir 385.4 mm (16 Jun 1994), Raigarh 360.9 mm (10 Jul 1958) and Kharsiya 340.2 mm (7 Sep 2003), district gauges excluded pending confirmation: each is admitted to the check of Clause 2.3.6(h) once IMD confirms at least 20 years of daily record at its gauge [TBC]; Akaltara 282.7 mm on 28 Aug 2020 (IMD); Kathgora's 433.6 mm of 24 Nov 1958 is excluded under Clause 2.3.6(h) [TBC]. The Gondwana coalfields, with flowing wells, mine inflows and drawdown cones of about 1.2 km around Gevra and Kusmunda (as reported); the ash dykes of the power plants, with breaches reported in 2024–2026. Receiving waters: the Hasdeo below Minimata Bango and Darri (highest levels Bangodam 303.5 m and Katghora 313.58 m, 2026), the Mand (Kurubhata 220.4 m, 2020), the Kelo below the Kelo dam (full reservoir level 233.000 m) and the Mahanadi (Kalma 202.43 m, Surajgarh 201.06 m, 2020), with the Hirakud backwater on the low outfalls of Raigarh and Sarangarh
CG-Z1 Mahanadi plain: CG-D South-east plain (Dhamtari–Mahasamund–Gariaband–Rajim)	Dhamtari, Kurud, Mahasamund, Saraipali, Basna, Pithora, Rajim, Gariaband and the east of Arang; boundaries [TBC]	Kanker (nearest long record); Raipur (Obsy); the district gauges of Dhamtari and Mahasamund [TBC]	The wettest band of the plain on the FER plates: 50-yr 24-h about 320 mm at Dhamtari, 320–360 mm at Mahasamund and Rajim and about 360 mm at Gariaband, with a closed cell of more than 440 mm on the Odisha border; NIH-DHAARA's highest plain cell, Mahasamund, 380 mm. Records: Saraipali 508.3 mm and Basna 453.0 mm (29 Aug 2003), Kurud 381.0 mm (4 Aug 1910) and Dhamtari 313.2 mm (IMD), all district gauges excluded pending confirmation: each is admitted to the check of Clause 2.3.6(h) once IMD confirms at least 20 years of daily record at its gauge [TBC]; no record in this sub-area yet qualifies. Alluvium up to about 70 m at Dhamtari; Dhamtari, Kurud, Basna and Pithora semi-critical. Receiving waters: the Mahanadi below Gangrel (Ravishankar Sagar, full reservoir level 348.700 m, 14 gates, spillway 17,230 m³/s), the Pairi (Malgaon highest level 329.33 m, 2026) and the Kodar
CG-Z2 Bastar plateau	The Bastar plateau and the Godavari frontage of the south, in CWC subzones 3(g) (the Indravati) and 3(f) (the Sabari and the Godavari) (Clause 2.3.6(b)); catchments that cross a zone line take the higher zone value	Jagdalpur 43041 (daily record 1910–2012), Kanker 42953 (1927–2008), Bailadila 43093 (1977–2002)	The wettest districts of the State (Bastar 1,575.7 mm, Dantewada 1,839.3 mm; IMD). FER 3(g) 50-yr 24-h about 240–280 mm at Jagdalpur, with a closed cell of less than 200 mm over the Indravati valley west of it; 1-h ratio 0.32 (3(g), 3(f)). NIH-DHAARA high here (Jagdalpur 340, Dantewada 342, Sukma 355 mm). Earliest onset (Jagdalpur 11–13 June) and latest withdrawal (12–14 October) in the State; pre-monsoon storms (Bailadila 207.0 mm, 11 May 1990) (IMD)

Zone (working name)	Extent (districts, basins)	Representative stations	Notes
CG-Z2 Bastar plateau: CG-E Bastar plateau and Godavari frontage	Jagdalpur, Kondagaon, Keskal, Narayanpur, Dantewada, Kirandul and Bacheli (Bailadila), Bijapur, Bhopalpatnam, Sukma, Konta, Pakhanjur and the south of Kanker district; Jagdalpur on the line between 3(g) and 3(f) (Clause 2.3.6(b)); boundaries [TBC]	Jagdalpur, Bailadila; the FER recording gauges at Jagdalpur, Paralkote, Bhopalpatnam and Sukma (1969–1986)	Records: Jagdalpur 43041, 259.7 mm (17 Jun 1974), and Bailadila 43093, 292.4 mm (18 Jun 1999), below the plates; Bijapur 362.0 mm (26 Jul 1988), Dantewada 355.0 mm (31 Aug 1976), Konta 340.0 mm and Sukma 325.0 mm (3–4 Aug 2006), district gauges excluded pending confirmation: each is admitted to the check of Clause 2.3.6(h) once IMD confirms at least 20 years of daily record at its gauge [TBC] (IMD); recording-gauge maxima Jagdalpur 80.0 mm in 1 hour and Bhopalpatnam 145.5 mm in 3 hours (8 Aug 1974) (CWC FER 3(g)). Granite-gneiss of the Bastar craton, the Bailadila iron formation and laterite; crystalline aquifers of low yield. Receiving waters: the Indravati at Jagdalpur (CWC warning 539.49 m, danger 540.80 m, highest level 544.891 m on 26 Sep 2026), rising in Odisha; the Indravati at Chindnar, the Narangi at Sonarpal, the Sabari at Talnar and Konta (as reported)
CG-Z3 Northern hills	The northern hill ranges, in CWC subzones 1(d) (the Rihand, Kanhar and Gopad, to the Sone) and 3(d) (the headwaters of the Hasdeo and the Mand, the Ib and the Brahmani side of Jashpur) (Clause 2.3.6(b)); catchments that cross a zone line take the higher zone value	Ambikapur 42693 (daily record 1951–2012), Jashpurnagar 42790 (1965–2012), Pendra Road 42779 (1903–2012)	District normals about 1,190–1,490 mm (Balrampur 1,191.5, Koriya 1,212.0, Surajpur 1,301.0, Surguja 1,386.9, Jashpur 1,486.9 mm; IMD). FER 1(d) 50-yr 24-h a closed contour of more than 280 mm around Ambikapur; 3(d) about 200–280 mm at Manendragarh and Jashpur Nagar, unresolved [TBC]; 1-h ratio 0.37 (1(d)). NIH-DHAARA low here (Ambikapur 249, Manendragarh 217, Jashpur 208 mm). October storms (Ambikapur 206.0 mm, 1 Oct 1961; Pendra Road 228.1 mm, 8 Oct 1994) (IMD)
CG-Z3 Northern hills: CG-F Northern hills (Surguja–Koriya–Jashpur)	Ambikapur, Bishrampur, Surajpur, Balrampur, Ramanujganj, Wadrafnagar, Manendragarh, Chirmiri, Baikunthpur, Jashpur Nagar, Kunkuri, Pathalgaon and Mainpat; Jashpur Nagar and the east of Jashpur district lie in UTM zone 45N; boundaries [TBC]	Ambikapur, Jashpurnagar	Records: Jashpurnagar 42790, 365.0 mm (30 Jun 2001), qualifying under Clause 2.3.6(h); Ambikapur 42693, 206.0 mm (1 Oct 1961), the station's highest day; Samaria (Surajpur) 540.0 mm (13 Aug 2003), Ambikapur (district gauge) 341.6 mm (27 Jun 1945), Baikunthpur 326.4 mm (4 Jul 1915) and Ramanujganj 320.0 mm (11 Sep 1987), district gauges excluded pending confirmation: each is admitted to the check of Clause 2.3.6(h) once IMD confirms at least 20 years of daily record at its gauge [TBC] (IMD). Excluded under Clause 2.3.6(h) until IMD confirms them [TBC]: Wadrafnagar 917.2 mm (8 Sep 2003; a gauge of about 10 years), Ambikapur 456.7 mm (24 Jul 1991; contradicted by IMD's station extremes, 174.0 mm on 23 Jul 1991) and Bharatanagar 301.0 mm (9 Jan 1995). Gondwana coalfields (Bishrampur, Chirmiri, Baikunthpur and Hasdeo-Arand); laterite on the trap of Mainpat and Jashpur. Receiving waters: the Rihand and the Kanhar, the headwaters of the Hasdeo, and the Ib and the Sankh; no CWC site with published levels found [TBC]

Annex 2B — Sub-daily ratios P(t)/P(24h) by zone: NIH-DHAARA (hourly-based) vs the one-third rule

Supports Clauses 2.6.1 to 2.6.4 of Chapter 2.

To be prepared. This annex will tabulate, for each zone of Annex 2A, the ratios of the t-hour to the 24-hour design depth from NIH-DHAARA, which are derived from hourly records, against the ratios implied by IMD Das et al. (2022) and by the one-third rule. It needs NIH-DHAARA depths for each duration at representative points in each zone (NIH), IMD recording-gauge data and the IDF values of Das et al. (2022) (IMD). Until it is published, sub-daily depths are read directly from NIH-DHAARA or an approved station IDF (Clause 2.6.1). Where only daily depths are available, the ratios used are those agreed with the Authority, which in Chhattisgarh are those of Clause 2.3.6: the CWC Flood Estimation Report ratios for the site's subzone in Table 2B.1, used as they stand at every AEP, with no absolute floor, applied to the 24-hour depth before the record check of Clause 2.3.6(h) (Clause 2.6.2), and are recorded on Form 2-1. The one-third rule is not used (Clause 2.6.4).

2B.1 The first row of Table 2B.1 gives the ratios of the one-third rule P(t) = P(24)·(t/24)^(1/3), for comparison only. The 1, 3, 6 and 12 h values are those stated in the commentary to Clause 2.6; the 2 h value is computed from the same formula. The rule shall not be used for design unless this annex confirms it for the zone concerned (Clause 2.6.4).

Table 2B.1 — Ratios P(t)/P(24h) by zone (layout)

Zone	Source	1 h	2 h	3 h	6 h	12 h	24 h
All (reference only, not for design)	One-third rule	0.35	0.44	0.50	0.63	0.79	1.00
CG-Z1 (CG-A to CG-D) and CG-Z3 (CG-F, the Hasdeo, Mand, Ib and Brahmani side)	CWC FER 3(d), 10-yr basis (interim design ratios, Clause 2.3.6(f))	0.39		0.61	0.76	0.86	1.00
CG-Z2 (CG-E, the Indravati side, including Jagdalpur)	CWC FER 3(g), 10-yr basis (interim design ratios, Clause 2.3.6(f))	0.32		0.50	0.635	0.80	1.00
CG-Z2 (CG-E, the Sabari and Godavari side: Sukma, Konta; and Jagdalpur, which takes the higher of 3(g) and 3(f) at each duration) and CG-Z1 (the Wainganga side of CG-A)	CWC FER 3(f), 10-yr basis (interim design ratios, Clause 2.3.6(f))	0.32		0.52	0.65	0.79	1.00
CG-Z3 (CG-F, the Sone side, including Ambikapur)	CWC FER 1(d), 50-yr (interim design ratios, Clause 2.3.6(f))	0.37		0.58	0.70	0.89	1.00
CG-Z1 (CG-B, the Narmada fringe of Kawardha; used with the higher of the 3(d) ratios, Clause 2.3.6(b))	CWC FER 3(c), 10-yr basis (interim design ratios, Clause 2.3.6(f))	0.34		0.50	0.63	0.79	1.00
CG-Z1 (CG-A edge): Kanker	CWC FER 3(d), Annex 4.4: the recording-gauge maxima of 1969–1980 as ratios to the 24-h maximum of the same record (separate storms, comparison only; 1-h value 75.3 mm in the 3(g) report)	0.34		0.49	0.69	0.75	1.00
CG-Z1 (CG-A): Nava Raipur Atal Nagar	NRDA chart, Naya Raipur Development Plan-2031, Fig. 10.1 (no 1-h or 24-h point, so no ratio to 24 h can be formed; 50-yr depths read by eye about 120 mm at 3 h and 138 mm at 6 h; cross-check only, Clause 2.3.6(g))	—		—	—	—	—
CG-Z1 (CG-A): Raipur	NIH-DHAARA, 50-yr, Raipur cell (comparison only; 3- and 6-hour values not queried)	0.20	0.34			0.80	1.00
CG-Z1 (CG-A): Nava Raipur	NIH-DHAARA, 50-yr, Nava Raipur cell (comparison only; 3- and 6-hour values not queried)	0.22	0.37			0.79	1.00

Zone	Source	1 h	2 h	3 h	6 h	12 h	24 h
CG-Z1 (CG-A): Durg	NIH-DHAARA, 50-yr, Durg cell (comparison only; 3- and 6-hour values not queried)	0.20	0.35			0.83	1.00
CG-Z1 (CG-B): Bilaspur	NIH-DHAARA, 50-yr, Bilaspur cell (comparison only; 3- and 6-hour values not queried)	0.27	0.41			0.77	1.00
CG-Z1 (CG-C): Korba	NIH-DHAARA, 50-yr, Korba cell (comparison only; 3- and 6-hour values not queried)	0.25	0.42			0.83	1.00
CG-Z1 (CG-C): Raigarh	NIH-DHAARA, 50-yr, Raigarh cell (comparison only; 3- and 6-hour values not queried)	0.25	0.39			0.79	1.00
CG-Z1 (CG-D): Mahasamund	NIH-DHAARA, 50-yr, Mahasamund cell (comparison only; 3- and 6-hour values not queried)	0.20	0.34			0.82	1.00
CG-Z2 (CG-E): Jagdalpur	NIH-DHAARA, 50-yr, Jagdalpur cell (comparison only; 3- and 6-hour values not queried)	0.25	0.43			0.84	1.00
CG-Z3 (CG-F): Ambikapur	NIH-DHAARA, 50-yr, Ambikapur cell (comparison only; 3- and 6-hour values not queried)	0.25	0.39			0.78	1.00

2B.2 Depths from fixed observation windows are converted to true-interval depths with the factors of Clause 2.6.3, repeated in Table 2B.2. They are the standard WMO (Hershfield) values and are not provisional.

Table 2B.2 — Fixed-interval adjustment factors (Clause 2.6.3)

Number of fixed observation intervals in the duration	Factor
1	1.13
2	1.04
3–4	1.02

Annex 2C — Grid-to-point factors

Supports Clauses 2.7.1 and 2.7.4 of Chapter 2.

To be prepared. This annex will give the factors that convert grid-average design depths (for example from 0.25° cells) to point depths, by zone, duration and AEP. It needs paired frequency analyses of IMD gridded rainfall and of the IMD and State gauges inside each cell (IMD, WRD, and the district and tehsil rain gauges of the State whose records IMD tabulates [TBC]), the benchmark points of Annex 2A, and a stated method for deriving the factors. Until it is published, design depths are taken as point values from NIH-DHAARA or an approved station IDF (Clauses 2.3.1 and 2.3.2). Grid-average values are not used as design depths unless a conversion agreed with the Authority is recorded on Form 2-1 (Clause 1.11.5).

2C.1 The commentary to Clause 2.7 shows why the factors matter, and in Chhattisgarh the reanalysis cells err both ways. At Jashpur the IMDAA cell of NIH-DHAARA (about 12 km) gives a 50-year 24-hour depth of about 208 mm and a 100-year depth of about 233 mm, while the Jashpurnagar gauge recorded 365.0 mm on 30 June 2001; at Ambikapur the cell's 50-year value, about 249 mm, is below the closed contour of more than 280 mm of the CWC plate and below the 341.6 mm recorded at Ambikapur on 27 June 1945; and at Kanker the cell's value, about 247 mm, is below the plate

value of 280–320 mm. At Durg the cell's value, about 333 mm, is above the plate value of 240–280 mm, and at Jagdalpur its value of about 340 mm is above both the plate value of 240–280 mm and the station's record of 259.7 mm (17 June 1974). The cells' 50-year 1-hour depths, about 54–85 mm, are 0.20–0.29 of their 24-hour depths, and at Raipur 62.7 mm against about 109–125 mm from the 3(d) plates and ratios (NIH-DHAARA, queried 7 Nov 2026; IMD; CWC). Values of this kind are grid-average values and need these factors before any use as point depths; under Clause 2.3.6 they are not used as design depths in Chhattisgarh until this annex and Annex 2A are published.

Table 2C.1 — Grid-to-point factors (layout)

Zone	Grid product and cell size	Duration (h)	AEP	Factor (point / grid)	Stations used	Years of record
		24				
		48				
		72				

Annex 2D — Chhattisgarh temporal patterns (Huff curves)

Supports Clauses 2.9.3 and 2.9.4 of Chapter 2, and Clause 11.7.3 of Chapter 11.

To be prepared. This annex will give dimensionless time-distribution (mass) curves of Chhattisgarh storms, in the form introduced by Huff (1967), by zone, storm-duration class and quartile, at stated probability levels. It needs storm hyetographs from recording gauges and automatic weather stations (IMD, its automatic weather stations and automatic rain gauges in the State, under the Meteorological Centre Raipur, and any recording gauges of WRD and of the Municipal Corporations, NRDA and the Smart City companies [TBC]), separated into independent storms by a stated rule, and the storms recorded by the recording gauges of the CWC reports at Kanker (26 June 1974 and 18 September 1980), Jagdalpur and Bhopalpatnam (8 August 1974), the Durg storm of 15 June 2004, the Korba storm of 12 August 2004, the Bilaspur storm of 29–30 June 2005, the Raipur storm of 30 June 2007, the upper Mahanadi storm of 28–29 August 2020, the Bastar storm of 26–27 August 2025 and the statewide storm of 24–27 September 2026 among them. Until it is published, design storms use the alternating-block pattern of Clause 2.9.1 and the 48- and 72-hour pattern of Clause 2.9.2; for catchments of 25–5,000 km², the CWC Flood Estimation Report curves may be used with a sensitivity run under Clause 2.9.1 (Clause 2.9.4). SCS Type distributions are not used (Clause 2.9.3). For the disaggregation of daily records in continuous simulation (Clause 11.7.3), the method is agreed with the Authority and recorded (Clause 1.11.5).

Table 2D.1 — Cumulative fraction of storm depth against cumulative fraction of storm time (layout, one table per zone, duration class and quartile)

Fraction of storm time	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
10% probability curve	0										1.0
50% probability curve	0										1.0
90% probability curve	0										1.0

2D.1 Each table shall state the zone, the duration class, the quartile (the quarter of the storm in which most rain fell), the number of storms analysed and the period of record.

Reference: Huff, F.A. (1967). Time distribution of rainfall in heavy storms. *Water Resources Research* 3(4), 1007–1019.

Annex 2E — Retention percentile depths by zone

Supports Clauses 2.8.3 and 2.10.1 to 2.10.2 of Chapter 2, Clause 4.4 of Chapter 4 and Chapter 8.

To be prepared. This annex will give, for each zone of Annex 2A, the retention design depth P_R: the 24-hour rainfall depth not exceeded on 90% of days with rain (≥ 2.5 mm) in the long-term record (Clause 2.10.1). It needs long daily records at stations in each zone (IMD, WRD, and the district and tehsil rain gauges of the State whose records IMD tabulates [TBC]) and IMD gridded data with the factors of Annex 2C where stations are sparse. Until it is published, P_R is computed by the definition of Clause 2.10.1 from the long-term daily record of a station agreed with the Authority, by the steps in 2E.1, and recorded on Form 2-1 (Clause 1.11.5).

2E.1 **Computing P_R from a daily record.**

1. Take the daily (24-hour) rainfall series of the station for its full record.
2. Keep only the days with rain of 2.5 mm or more (wet days).
3. Rank the wet-day depths and take the depth not exceeded on 90% of them (the 90th percentile).
4. Do not apply the climate factor (Clause 2.8.3) or the fixed-interval factor; P_R is a statistic of everyday rainfall, not an AEP design depth.
5. Record the station, the period of record, the number of wet days and P_R on Form 2-1.

Table 2E.1 — Retention depth P_R by zone (layout)

Zone	Stations or grid cells used	Period of record	Number of wet days (≥ 2.5 mm)	P_R, 90th percentile (mm)	Issued on
CG-Z1 Mahanadi plain					
CG-Z2 Bastar plateau					
CG-Z3 Northern hills					

Form 2-1 — Design Rainfall Sheet

Supports Clauses 2.3 to 2.13 of Chapter 2, and Clause 10.4.1(d) of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. One sheet for each hydrologic submittal. The steps follow Figure 2.3 of Chapter 2.

Table 2F.1 — Form 2-1: site, source and storms

No.	Item (clause)	Entry
	A. Site (Clause 2.13, item 1)	
A1	Project and site	_____
A2	Site coordinates (latitude, longitude, decimal degrees)	_____
A3	Rainfall zone (Annex 2A, or as agreed with the Authority)	_____
A4	Contributing catchment area (km²) (Clause 2.7)	_____
A5	Design life: more than 25 years / temporary works, 5 years or less (Clause 2.8.2)	_____
A6	Outfall under tidal or river backwater influence: Yes / No (Clauses 2.5.3, 2.12)	_____
	B. Source (Clauses 2.3, 2.3.5; item 2)	
B1	NIH-DHAARA: version or access date	_____
B2	Coordinates queried	_____
B3	Screenshot or export of the values extracted attached: reference	_____
B4	Station IDF used (Clause 2.3.2): No / Yes — station, ID, distance from site (km)	_____
B5	Station IDF: years of recording-gauge data (30 or more); published or approved by	_____
B6	Station IDF: change of terrain or coastal exposure between gauge and site: None / describe	_____
	C. AEPs and durations (Clauses 2.4, 2.5)	
C1	Elements designed and their AEPs (Table 2.1)	_____
C2	Conveyance: Tc and design duration (not less than 10 min) (Clause 2.5.1)	_____
C3	Storage: durations checked from {1, 2, 3, 6, 12, 24, 48, 72 h}; 24, 48 and 72 h included (Clause 2.5.2)	_____
C4	Critical duration found for each storage element	_____
	D. Conversions	
D1	Sub-daily depths: read directly / from Annex 2B ratios / from ratios agreed with the Authority (Clauses 2.6.1, 2.6.2)	_____
D2	Fixed-interval factor applied: none (true-interval source) / 1.13 / 1.04 / 1.02 (Clause 2.6.3)	_____
D3	Grid-to-point factor: not needed (point source) / Annex 2C / agreed value (Clause 2.7.1)	_____
D4	ARF: 1.0 (10 km² or less; single-site storage) / CWC curve / interpolated (Clauses 2.7.2–2.7.4)	_____
D5	Climate factor: 1.20 (≤ 6 h) and 1.10 (> 6–72 h) / 1.0 temporary works / projected depths under Clause 2.8.4	_____
	E. Cross-checks (Clause 2.3.3; item 4)	
E1	IMD Das et al. (2022) values for 1–24 h: reported in Table 2F.2	_____

No.	Item (clause)	Entry
E2	CWC subzone 3(d) / 3(g) / 3(f) / 1(d) / 3(c) 24-hour point value, catchments of 25–5,000 km²: subzone used	_____
E3	CWC floor governs for any AEP: No / Yes — AEPs	_____
	F. Retention depth (Clause 2.10)	
F1	P_R (mm) and source: Annex 2E zone value / computed under Annex 2E, 2E.1 — station and period	_____
F2	No climate factor applied to P_R (Clause 2.8.3): confirmed	_____
	G. Design storms (Clauses 2.9, 2.11, 2.12; items 5 and 6)	
G1	Storms up to 24 h: alternating block, peak at 50%; time step (≤ Tc/3 and ≤ 15 min)	_____
G2	48 and 72 h storms: built under Clause 2.9.2 (Table 2F.3)	_____
G3	Other pattern used: none / Annex 2D / CWC FER with sensitivity run (Clauses 2.9.3, 2.9.4)	_____
G4	Hyetographs attached as a table and a plot for each design storm: reference	_____
G5	AMC: III for every design storm of 1 July–30 September on kanhar or dorsa paddy lowland, talab-draining and valley-floor sites, whatever its duration, and at any time in catchments draining to a reservoir or barrage pond, the Hirakud backwater, a mine void or a river at or above its CWC warning level; at any other site or time, III for a storm that follows or contains a 5-day total of more than 53 mm; II otherwise (Clause 2.11.4)	_____
G6	Pre-development condition: greenfield, open land in good condition for the mapped HSG (Clause 2.11.5)	_____
G7	Release criterion: standard, Q_post ≤ 0.9 × Q_pre (Clause 2.11.2) / flood-sensitive, mixed-frequency rule (Clause 2.11.3)	_____
G8	Tailwater joint cases run (Clause 2.12): case (a) MHWS + sea-level allowance; case (b) 10% AEP 48 h storm with 1% AEP still-water level + sea-level allowance; governing case	_____
	H. Prepared by	
H1	I confirm that the values above are those extracted and used in the submittal. Name, registration number, signature, date	_____

Design depth = source depth × fixed-interval factor × grid-to-point factor × ARF × climate factor. Where the design depth is lower than the CWC floor for the same AEP, the floor is adopted (Clause 2.3.3).

Table 2F.2 — Form 2-1: depth table (Clause 2.13, item 3)

AEP (T-yr)	Duration	Source depth (mm)	Fixed-interval factor	Grid-to-point factor	ARF	Climate factor	Design depth (mm)	IMD Das et al. (2022) (mm)	CWC floor (mm)	Adopted depth (mm)

Table 2F.3 — Form 2-1: 48- and 72-hour storm construction (Clause 2.9.2)

AEP (T-yr)	Duration (h)	Total design depth (mm)	24 h design depth, final 24 h (mm)	Remainder (mm)	Hours before the final 24 h	Uniform intensity before the final 24 h (mm/h)
	48				24	
	72				48	

ANNEXES TO CHAPTER 3

Hydrologic Methods

These annexes support Chapter 3. Annex 3C sets out benchmark test problems for software acceptance, and Forms 3-1 and 3-2 are the submittal forms required by Clause 3.17. Annexes 3A, 3B, 3D and 3E need survey, gauging or published report data that this draft does not yet contain; each gives the intended layout and the interim rule of Chapter 3 that applies until it is published. The forms are draft forms and have no statutory status.

Annex 3A — Chhattisgarh soil series to HSG lookup; seasonal high water table map

Supports Clauses 3.7 and 3.10 of Chapter 3.

To be prepared. This annex will contain (a) a lookup from each soil series mapped in Chhattisgarh to its hydrologic soil group (HSG), in both the drained and the undrained state, and (b) a map of the depth to the seasonal high groundwater table in the south-west monsoon (June–September). It needs NBSS&LUP and Chhattisgarh Department of Agriculture (soil survey, with Indira Gandhi Krishi Vishwavidyalaya, Raipur) [TBC] series maps with their scales, measured saturated hydraulic conductivity and depth to an impermeable layer for each series, and observation-well records from the Central Ground Water Board and the Water Resources Department, supplemented by piezometers in the kanhar and dorsa paddy lowlands of the Chhattisgarh plain, the floodplains of the Kharun, the Seonath, the Arpa, the Hasdeo, the Kelo, the Mahanadi and the Indravati at the cities, the land round the talabs of Raipur and the lakes and reservoirs of Nava Raipur, and the Hirakud backwater at Raigarh and Sarangarh. Until it is published, use the interim rules of Chapter 3: assign HSG from measured saturated hydraulic conductivity and depth to an impermeable layer or water table by the criteria of NEH Part 630, Ch. 7 (Clause 3.7.2); verify it by site infiltration tests wherever retention relies on infiltration (Clause 3.7.3); take the seasonal high water table from site piezometers, trial pits or the nearest observation well (Clause 3.10.2); and assign HSG D wherever that level is within 0.6 m of the ground surface (Clause 3.10.1).

Table 3A.1 — Soil series to HSG lookup (layout)

Soil series	District(s)	Taxonomic class	Surface texture	Saturated hydraulic conductivity (mm/h)	Depth to impermeable layer (m)	HSG, drained	HSG, undrained	Source map and scale

Table 3A.2 — Seasonal high water table, by mapping unit (layout)

Mapping unit or grid cell	Observation wells used	Years of June–September record	Highest June–September level below ground (m)	Within 0.6 m of ground (Clause 3.10.1)?	Data source

Mapping unit or grid cell	Observation wells used	Years of June–September record	Highest June–September level below ground (m)	Within 0.6 m of ground (Clause 3.10.1)?	Data source

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Annex 3B — Chhattisgarh CN calibration and supplementary land covers (cracked kanhar fallow, bunded paddy, talab and reservoir beds, bhata and laterite crust, overburden dumps and ash ponds, Bastar and northern forest and scrub)

Supports Clauses 3.8 and 3.15 of Chapter 3.

To be prepared. This annex will contain (a) curve numbers calibrated against observed rainfall and runoff in Chhattisgarh catchments, compared with the TR-55 values of Table 3.5, and (b) curve numbers for land covers that Table 3.5 does not list: the kanhar of the lowlands of the Chhattisgarh plain while it is still dry and cracked early in the monsoon; the bunded paddy fields of the kanhar and dorsa lowlands, whose bunds hold water on the field until they overtop; the beds of talabs, dabris and reservoirs, such as the talabs of Raipur and the reservoirs of Nava Raipur, that are dry or cultivated at the start of a storm; the bare bhata uplands, with their thin soil over murram or ferricrete, and the laterite crust and rock outcrop of the Bastar plateau and the northern hills; the overburden dumps, backfilled mine land, compacted mine benches and haul roads of the Korba, Raigarh and northern coalfields, and the ash ponds and ash dykes of the power stations, which shed water fast and carry heavy sediment; and the sal forest, scrub and grassland of the Bastar plateau and of the hills of Surguja, Koriya and Jashpur. It needs paired rainfall–runoff records for small gauged catchments of known cover and soil, from WRD gauging sites, agricultural research stations and tank gauges, analysed by event and by antecedent moisture. Until it is published: the pre-development reference stays open space in good condition for every site (Clause 3.3.2), so these covers do not enter the pre-development run; for off-site existing conditions, use the nearest Table 3.5 or TR-55 cover row, including the cultivated rows (Clause 3.8.3(c)), and state the row chosen and the reason on Form 3-1; land under standing water at the start of the design storm is modelled as water surface with no losses (Table 3.5; Clause 3.10.4(b)). Any departure from these defaults requires calibration under Clause 3.15 (Clause 3.15.1(d)).

Table 3B.1 — Supplementary land covers, AMC II (layout)

Cover type	HSG A	HSG B	HSG C	HSG D	Ia ratio	Basis (catchments, events, years)
Kanhar fallow of the plain lowlands, dry and cracked at the start of the storm (before 15 July)						
Bunded paddy field (kanhar or dorsa), with the bund height recorded						
Talab, dabri or reservoir bed, dry or cultivated at the start of the storm						
Bare bhata upland, thin soil over murram or ferricrete						
Laterite crust and rock outcrop, Bastar plateau and northern hills						
Overburden dump, backfilled mine land, compacted bench or haul road (Korba, Raigarh and northern coalfields)						
Ash pond or ash dyke of a power station						

Cover type	HSG A	HSG B	HSG C	HSG D	Ia ratio	Basis (catchments, events, years)
Sal forest, scrub and grassland of the Bastar plateau and the hills of Surguja, Koriya and Jashpur						
Other (state)						

Table 3B.2 — CN calibration catchments (layout)

Catchment	District	Area (km²)	Dominant HSG and cover	Events used	Table 3.5 CN	Calibrated CN	Calibrated ÷ Table 3.5

Annex 3C — Benchmark test problems for software acceptance

Supports Clauses 3.5, 3.6, 3.8, 3.9, 3.11, 3.12 and 3.16 of Chapter 3.

Purpose. Software not listed in Clause 3.16.1 shall reproduce the results of the five problems below (Clause 3.16.3). Each problem is fully specified and uses only the methods and tables of Chapter 3, so every answer can be checked by hand. The acceptance tolerance is ±5% on each tabulated result, the equivalence threshold of Clause 3.16.2; for closed-form results (Problems 3C-1 to 3C-4), software should agree to the rounding shown. The submittal shall report the software result next to each answer below.

Illustrative — not design values. The inputs (areas, intensities, rainfall depths, slopes and the basin) are chosen for testing only. They are not design data for any site.

Problem 3C-1 — Rational Method peak (Clause 3.5)

Inputs. A 2.0 ha catchment draining to one inlet: 1.2 ha concrete paving and roofs, C = 0.90 (Table 3.2, upper half of 0.80–0.95); 0.8 ha lawns on sandy soil, average slope, C = 0.15 (Table 3.2, upper half of 0.10–0.15). Design AEP 1% (100-yr). Rainfall intensity at Tc, including the climate factor: i = 120 mm/h.

Part (b): a separate 0.5 ha fully paved catchment, C = 0.90, design AEP 1%, i = 150 mm/h.

Table 3C.1 — Problem 3C-1 solution

Step	Calculation	Result
Composite C (Clause 3.5.4)	(1.2 × 0.90 + 0.8 × 0.15) / 2.0	0.60
Frequency factor (Table 3.3)	1% AEP	Cf = 1.25
C · Cf (≤ 1.0)	0.60 × 1.25	0.75
Peak flow (Clause 3.5.1)	0.75 × 120 × 2.0 / 360	**Q = 0.500 m³/s**
Part (b): C · Cf	0.90 × 1.25 = 1.125, capped at 1.0 (Clause 3.5.3)	1.00
Part (b): peak flow	1.00 × 150 × 0.5 / 360	**Q = 0.208 m³/s**

Problem 3C-2 — Time of concentration by the segment method (Clause 3.6)

Inputs. Pre-development flow path: sheet flow over greenfield, n = 0.15 (Clause 3.6.3), L = 30 m, S = 0.01; P2 (50% AEP 24-hour rainfall, no climate factor) = 80 mm. Shallow concentrated flow, unpaved, L = 200 m, S = 0.01. Channel flow, L = 400 m, bank-full velocity V = 1.0 m/s (given).

Table 3C.2 — Problem 3C-2 solution

Segment	Calculation	Travel time
Sheet flow (Clause 3.6.2(a))	0.091 × (0.15 × 30)^0.8 / (80^0.5 × 0.01^0.4) = 0.2138 h	12.83 min
Shallow concentrated (Clause 3.6.2(c))	V = 4.918 × 0.01^0.5 = 0.4918 m/s; 200 / (3,600 × 0.4918) = 0.1130 h	6.78 min
Channel (Clause 3.6.2(d))	400 / (3,600 × 1.0) = 0.1111 h	6.67 min
Tc (Clause 3.6.1; ≥ 10 min)	12.83 + 6.78 + 6.67	**26.27 min**
Lag (Clause 3.6.5)	0.6 × 26.27	15.76 min

Problem 3C-3 — SCS-CN runoff depth, AMC and Ia sensitivity (Clause 3.8)

Inputs. Pre-development reference on HSG C: CN = 74 at AMC II (Table 3.5), all pervious. Cumulative rainfall P = 100 mm; second case P = 25 mm.

Table 3C.3 — Problem 3C-3 solution

Case	S (mm)	Ia (mm)	Runoff Q (mm), P = 100 mm	Runoff Q (mm), P = 25 mm
(a) AMC II, Ia = 0.2S: S = 25,400 / 74 − 254	89.24	17.85	**39.38**	**0.53**
(b) AMC III, Ia = 0.2S: CN(III) = 23 × 74 / (10 + 0.13 × 74) = 86.75	38.80	7.76	**64.93**	—
(c) AMC II, Ia = 0.05S: S0.05 = 25.4 × 1.33 × (89.24 / 25.4)^1.15	143.31	7.17	**36.49**	**1.97**

Runoff is Q = (P − Ia)² / (P − Ia + S). Case (c) gives less runoff than case (a) at 100 mm but more at 25 mm, as Clause 3.8.5 and Figure 3.3 describe.

Problem 3C-4 — Separate vs composite impervious representation, and SCS unit hydrograph peak (Clauses 3.9 and 3.11.2)

Inputs, part (a). A site that is 50% directly connected impervious (CN 98) and 50% open space on HSG B (CNp = 61), AMC II, Ia = 0.2S, P = 25 mm.

Inputs, part (b). A sub-area of A = 2.5 km² with Tc = 1.0 h and computation interval Δt = 0.1 h.

Table 3C.4 — Problem 3C-4 solution

Step	Calculation	Result
(a) Impervious half (Clause 3.9.2)	S = 5.18 mm, Ia = 1.04 mm; Q = 23.96² / (23.96 + 5.18)	19.70 mm
(a) Pervious half	S = 162.39 mm, Ia = 32.48 mm > P	0 mm
(a) Area-weighted runoff, separate sub-areas	0.5 × 19.70 + 0.5 × 0	**9.85 mm**

Step	Calculation	Result
(a) Composite CN (Clause 3.9.4)	61 + 0.50 × (98 − 61)	79.5
(a) Runoff with composite CN	S = 65.50 mm, Ia = 13.10 mm; Q = 11.90² / (11.90 + 65.50)	**1.83 mm**
(b) Interval check (Clause 3.11.3(a))	0.133 × 1.0 = 0.133 h ≥ Δt = 0.1 h	satisfied
(b) Lag and time to peak (Clause 3.11.2)	tL = 0.6 × 1.0 = 0.60 h; tp = 0.1 / 2 + 0.60	tp = 0.65 h
(b) Unit hydrograph peak, Q = 1 mm	0.208 × 2.5 × 1 / 0.65	**qp = 0.800 m³/s per mm**

Part (a) reproduces the comparison in the commentary to Clause 3.9 (about 9.9 mm vs 1.8 mm).

Problem 3C-5 — Level-pool (modified Puls) storage routing (Clause 3.12.1)

Inputs. A detention basin with vertical sides and plan area 500 m², with the stage–storage–discharge relationship of Table 3C.5. Triangular inflow hydrograph: 0 at t = 0, rising linearly to 0.50 m³/s at t = 30 min, falling linearly to 0 at t = 90 min. Basin empty at t = 0. Routing interval Δt = 10 min (600 s). No tailwater.

Table 3C.5 — Problem 3C-5 stage–storage–discharge relationship

Stage (m)	Storage S (m³)	Outflow O (m³/s)
0.0	0	0.00
0.5	250	0.05
1.0	500	0.10
1.5	750	0.15
2.0	1,000	0.20
2.5	1,250	0.25

Method. The storage-indication equation is (I1 + I2) + (2S1/Δt − O1) = (2S2/Δt + O2). The table gives O = S / 5,000 (S in m³, O in m³/s), so 2S/Δt + O = 17.667 O and 2S/Δt − O = 15.667 O, and each step is O2 = [(I1 + I2) + 15.667 O1] / 17.667. Software interpolating linearly in Table 3C.5 shall give the same result.

Table 3C.6 — Problem 3C-5 routed outflow

Time (min)	Inflow I (m³/s)	Outflow O (m³/s)	Storage (m³)	Stage (m)
0	0.0000	0.0000	0.0	0.000
10	0.1667	0.0094	47.2	0.094
20	0.3333	0.0367	183.3	0.367
30	0.5000	0.0797	398.4	0.797
40	0.4167	0.1226	612.8	1.226
50	0.3333	0.1511	755.7	1.511
60	0.2500	0.1670	835.2	1.670
70	0.1667	**0.1717**	**858.6**	**1.717**
80	0.0833	0.1664	832.1	1.664
90	0.0000	0.1523	761.5	1.523

Time (min)	Inflow I (m³/s)	Outflow O (m³/s)	Storage (m³)	Stage (m)
120	0.0000	0.1062	531.1	1.062
180	0.0000	0.0517	258.3	0.517

Results. Peak outflow 0.172 m³/s at 70 min; peak storage 859 m³; peak stage 1.72 m. The inflow peak of 0.50 m³/s is reduced to 34% of its value.

Table 3C.7 — Benchmark results to be reported by the applicant

Problem	Result	Benchmark	Software result	Difference (%)	Within ±5%?
3C-1	Q, main case (m³/s)	0.500			
3C-1	Q, part (b) (m³/s)	0.208			
3C-2	Tc (min)	26.27			
3C-3	Q, case (a), P = 100 mm (mm)	39.38			
3C-3	Q, case (b), P = 100 mm (mm)	64.93			
3C-3	Q, case (c), P = 100 mm (mm)	36.49			
3C-3	Q, case (c), P = 25 mm (mm)	1.97			
3C-4	Runoff, separate sub-areas (mm)	9.85			
3C-4	Runoff, composite CN (mm)	1.83			
3C-4	qp (m³/s per mm)	0.800			
3C-5	Peak outflow (m³/s)	0.172			
3C-5	Peak storage (m³)	859			

Annex 3D — Peak rate factor study for flat coastal land

Supports Clauses 3.11.5 and 3.15 of Chapter 3.

> **Not required in Chhattisgarh.** Chhattisgarh is landlocked and has no flat coastal plain, so no land in the State meets Clause 3.11.5(a), and the standard factor 484 (PRF' = 0.208) applies to all land (Clause 3.11.5). The kanhar and dorsa paddy lowlands of the plain, the river floodplains and the margins of the talabs and reservoirs are flat in places but are not coastal plain; their slow response is represented by storage, bunded fields and backwater (Clauses 3.10.4 and 3.12.4), not by a lower peak rate factor, while the Bastar plateau and the northern hills respond fast. The annex and the layout of Table 3D.1 are kept only so that the annex numbering is unchanged.

Table 3D.1 — Calibrated peak rate factors (layout)

Catchment	Area (km²)	Mean slope (m/m)	Drainage condition	Events used	Calibrated PRF (US customary)	PRF' (SI)	Range of applicability

Annex 3E — CWC SUH parameters for subzones 3(d), 3(g), 3(f), 1(d) and 3(c), with worked example

Supports Clauses 3.4.3 and 3.11.4 of Chapter 3.

To be prepared. This annex will tabulate, for CWC subzones 3(d) (Mahanadi), 3(g) (Indravati), 3(f) (Lower Godavari), 1(d) (Sone) and 3(c) (Upper Narmada and Tapi), the regression relationships for the synthetic unit hydrograph parameters, the design storm duration, loss rate, base flow, areal reduction and time distribution, each with its reference to the Flood Estimation Report, and a worked example for one catchment. It needs the five CWC Flood Estimation Reports that cover Chhattisgarh: the Mahanadi subzone 3(d) (revised, M-3(d)/R-3/25/1996, February 1997), which covers the Mahanadi basin, about 56% of the State, and the Brahmani headwaters in Jashpur, including Raipur, Nava Raipur, Durg, Bhilai, Rajnandgaon, Bilaspur, Korba, Janjgir–Champa, Raigarh, Dhamtari, Mahasamund, Kanker, Kawardha and Jashpur Nagar; the Indravati subzone 3(g) (I/21/1992, whose cover reads I-21/1993), which covers the Indravati on the Bastar plateau, including Jagdalpur, Kondagaon, Narayanpur, Dantewada and Bijapur; the Lower Godavari subzone 3(f) (revised, LG-3(f)/R-2/24/1995), which covers the Sabari and the Godavari frontage, including Sukma and Konta, and the Wainganga fringe of Rajnandgaon district; the Sone subzone 1(d) (S/15/1987), which covers the Rihand, the Kanhar river and the Gopad in the north, including Ambikapur, Bishrampur, Surajpur and Balrampur; and the Upper Narmada and Tapi subzone 3(c) (revised, UNT-3(c)/R-6/46/2002), which covers only a fringe of about 744 km² in Kawardha and Rajnandgaon districts with no town (see the References of Chapter 3); with the plates and annexes of the 3(f) report, which were missing from the copy read, the 3(g) unit hydrograph relationships, read by optical character recognition only, the 3(f) loss rate, printed as 2 mm/h in the 3(f) report but quoted as 5 and 2 mm/h in the 3(g) report, the 1(d) loss rate and time-to-peak exponent, the 3(d) loss-rate curve, read only by eye, the areal reduction and time-distribution tables of all five, and the subzone boundaries within Chhattisgarh, including those at Jagdalpur (3(g)/3(f)), Ambikapur (1(d)/3(d)), Kanker (3(d)/3(g)) and Rajnandgaon–Khairagarh (3(d)/3(f)), still to be confirmed on the reports' page images and index maps [TBC], transcribed and checked against the published reports, and a catchment with physiographic parameters measured from survey or a DEM. Until it is published, take all SUH inputs directly from the applicable Flood Estimation Report (Clause 3.11.4(a)), apply the 24-hour design rainfall of Clauses 2.3.3(b) and 2.7.3 with the climate factor of Clause 2.8 (Clause 3.11.4(b)), report the SUH result alongside the HEC-HMS or SWMM result for the existing natural condition (Clause 3.11.4(c)), and, where the hydrograph model gives a lower 1% AEP peak, either justify it by calibration under Clause 3.15 or adopt the SUH peak (Clause 3.11.4(d)).

Table 3E.1 — SUH relationships by subzone (layout)

Parameter	Symbol	Unit	Subzone 3(d) relationship	Subzone 3(g) relationship (unit hydrograph relationships by optical character recognition only) [TBC]	Subzone 3(f) relationship
Time from centre of unit rainfall to peak	tp	h	tp = 1.757 (L·Lc/√S)^0.261	tp = 0.353 (L·Lc/√S)^0.45	tp = 0.348 (L·Lc/√S)^0.454 (the table of the report prints L·Lc/S; see the note) [TBC]
Peak discharge per unit area	qp	m³/s per km²	qp = 1.260 tp^−0.725	qp = 1.968 tp^−0.842	qp = 1.842 tp^−0.804

Parameter	Symbol	Unit	Subzone 3(d) relationship	Subzone 3(g) relationship (unit hydrograph relationships by optical character recognition only) [TBC]	Subzone 3(f) relationship
Width of UH at 50% of peak	W50	h	W50 = 1.974 qp^−1.104	W50 = 2.30 qp^−1.018	W50 = 2.353 qp^−1.005
Width of UH at 75% of peak	W75	h	W75 = 0.961 qp^−1.125	W75 = 1.356 qp^−1.007	W75 = 1.351 qp^−0.992 (exponent hand-corrected in the scan) [TBC]
Width of rising limb at 50% of peak	WR50	h	WR50 = 1.150 qp^−0.829	WR50 = 0.954 qp^−1.078	WR50 = 0.936 qp^−1.047
Width of rising limb at 75% of peak	WR75	h	WR75 = 0.527 qp^−0.932	WR75 = 0.581 qp^−1.035	WR75 = 0.579 qp^−1.004
Base width of UH	TB	h	TB = 5.411 tp^0.826	TB = 4.572 tp^0.90	TB = 4.589 tp^0.894
Unit duration	tr	h	1	1	1
Design storm duration	TD	h	TD = 1.1 tp (it gave a higher 50-yr peak than TD = TB on all 16 catchments of the report)	TD = 1.1 tp [TBC]	TD = 1.1 tp
Design loss rate	—	mm/h	Falls with the duration of the storm (Fig. 13 of the report): about 3.5 for a 3-hour storm, falling to 1.5 for storms of about 13 hours or more, read by eye from the curve [TBC]; the worked example uses 2.1 for an 8-hour storm	5 on catchments larger than 75 km² and 2 on smaller ones (printed "0.5 cm/hr" and "0.2 cm/hr"), quoted by the report as the values of subzone 3(f); see the note	2 (printed "0.2 cm/hr"); see the note [TBC]
Design base flow	—	m³/s per km²	0.10	0.06	0.05
Ratio of t-hour to 24-hour rainfall	—	—	10-yr, from 6 self-recording gauges: 1 h 0.390; 3 h 0.610; 6 h 0.760; 9 h 0.820; 12 h 0.860; 15 h 0.880; 18 h 0.920; 24 h 1.000	10-yr, from 11 self-recording gauges: 1 h 0.320; 3 h 0.500; 6 h 0.635; 9 h 0.730; 12 h 0.800; 15 h 0.864; 18 h 0.917; 24 h 1.000	10-yr: 1 h 0.32; 3 h 0.52; 6 h 0.65; 9 h 0.73; 12 h 0.79; 15 h 0.85; 18 h 0.91; 24 h 1.00
Time distribution of 24-hour rainfall	—	cumulative fraction	Derived in the report from 2,688 storms at 28 self-recording gauges; not transcribed [TBC]	[TBC]	[TBC]
Conversion of 1-day to 24-hour rainfall	—	factor	1.15	1.15	1.15
Areal reduction factor	ARF	—	Not transcribed; the worked example uses 0.9144 for an 8-hour storm on 194 km² [TBC]	[TBC]	In an annex missing from the copy read [TBC]

Parameter	Symbol	Unit	Subzone 3(d) relationship	Subzone 3(g) relationship (unit hydrograph relationships by optical character recognition only) [TBC]	Subzone 3(f) relationship
50-yr 24-hour point rainfall at Chhattisgarh towns (100-yr in brackets)	—	mm	Plate 9 (Plate 10): Bilaspur about 200–240 (240–280); Durg, Bhilai and Korba about 240–280 (280–320); Raipur and Nava Raipur about 280–320 (320–360); Rajnandgaon about 280 (about 320); Kanker about 280–320; Dhamtari about 320; Raigarh about 320, inferred from the 360 line of Plate 10 through the Kelo bridge (about 360); Mahasamund and Rajim about 320–360, with a closed ">440" cell on the Gariaband–Nuapada border; Manendragarh and Jashpur Nagar about 200–280, unresolved [TBC]	Plate 10 (Plate 11): Jagdalpur about 240–280 (240–280, near 280), with a "<200" cell over the Indravati valley west of the city and the lines rising towards Koraput	Plates not in the copy read; Sukma about 200–240 at the edge of the 3(g) plate [TBC]
Gauged catchments in Chhattisgarh	—	—	5 of the 16 used: the Kelo at Raigarh (bridge 121, 1,150 km²), the Lilagar near Akaltara (bridge 12, 666 km²), the Parri near Rajnandgaon (bridge 332(ii), 225 km²), the Borai (bridge 69, 173 km²) and the Karwar (bridge 59(b), 136 km²); four more listed but not used	None: the report had no gauged catchment of its own	Not known from the copy read [TBC]
Catchment area for which the relationships apply	A	km²	25–1,000; up to 5,000 with judgement	[TBC]	25–1,000, with judgement above that
Report reference	—	—	Table 3.3 (p. 13), §3.5 (p. 14), §3.6 (p. 15), Annex 3.1 (p. 49), Annexes 4.3 and 4.4 (pp. 58–60), Annex 5.2 and Fig. 13	§3.3 to §3.5 (pp. 12–13), Annex 4.4 (p. 50)	[TBC]

Parameter	Symbol	Unit	Subzone 1(d) relationship	Subzone 3(c) relationship
Time from centre of unit rainfall to peak	tp	h	tp = 0.314 (L/√S)^1.012 (the exponent is printed 1.102 in the relationship table and 1.012 in the worked example and the design-storm formula; see the note) [TBC]	tp = 0.995 (L·Lc/√S)^0.2654
Peak discharge per unit area	qp	m³/s per km²	qp = 1.664 tp^−0.965	qp = 1.665 tp^−0.71678
Width of UH at 50% of peak	W50	h	W50 = 2.534 qp^−0.976	W50 = 1.9145 qp^−1.2582
Width of UH at 75% of peak	W75	h	W75 = 1.478 qp^−0.860	W75 = 1.1102 qp^−1.2088
Width of rising limb at 50% of peak	WR50	h	WR50 = 1.091 qp^−0.750	WR50 = 0.7060 qp^−1.3859
Width of rising limb at 75% of peak	WR75	h	WR75 = 0.672 qp^−0.719	WR75 = 0.45314 qp^−1.3916
Base width of UH	TB	h	TB = 5.526 tp^0.866	TB = 5.04537 tp^0.71637
Unit duration	tr	h	1	1

Parameter	Symbol	Unit	Subzone 1(d) relationship	Subzone 3(c) relationship
Design storm duration	TD	h	TD = 1.1 tp = 0.345 (L/√S)^1.012, rounded to the nearest hour	TD = 1.1 tp, rounded to the hour
Design loss rate	—	mm/h	2.5 (printed "modal loss rate of 0.25 cm/hr"; one passage reads "0.45cm/hr" by optical character recognition, to be checked against the page image [TBC])	1 (printed "1 mm/hr")
Design base flow	—	m³/s per km²	Not located in the copy read; the worked example's 15.33 m³/s on 340.64 km² implies about 0.045 [TBC]	0.05
Ratio of t-hour to 24-hour rainfall	—	—	50-yr: 1 h 0.37; 3 h 0.58; 6 h 0.70; 9 h 0.80; 12 h 0.89; 15 h 0.94; 18 h 0.98 (read by optical character recognition only [TBC]); 24 h 1.00	10-yr: 1 h 0.34; 3 h 0.50; 6 h 0.63; 9 h 0.72; 12 h 0.79; 15 h 0.86; 18 h 0.91; 24 h 1.00
Time distribution of 24-hour rainfall	—	cumulative fraction	Average curves of Fig. 12(a)–(e) for storms of 2–3, 4–6, 7–12, 13–18 and 19–24 hours, and coefficients of Table A-2; not transcribed [TBC]	Annexure 4.3 of the report; not transcribed [TBC]
Conversion of 1-day to 24-hour rainfall	—	factor	1.15	1.15
Areal reduction factor	ARF	—	Table 6 (p. 70), by optical character recognition: 50 km², 80% at 1 h, 88% at 3 h, 93% at 6 and 12 h and 97% at 24 h; 100 km², 73, 82, 88, 90 and 95%; from catchments with data up to 800 km², to be used with caution above that [TBC]	Annexure 4.4 and Fig. 12(a) of the report; not transcribed [TBC]
50-yr 24-hour point rainfall at Chhattisgarh towns (100-yr in brackets)	—	mm	Plate 9 (Plate 10): Ambikapur and Bishrampur about 280, in a ">280" cell (about 320, in a ">320" cell), with the bounding line just east of Ambikapur; lower towards Surajpur and Manendragarh	Not read: no town of the State lies in the subzone
Catchment area for which the relationships apply	A	km²	Recommended for 25–1,500, usable to 5,000 with judgement	[TBC]
Report reference	—	—	Relationship table (p. 40), worked example (pp. 15–23), design storm (p. 49), rainfall ratios (p. 50), loss rate (pp. 84–88)	Worked example for Railway Bridge No. 930; Plates 9 and 10

Note: relationships as printed in the five CWC reports that cover Chhattisgarh (References of Chapter 3); L is the length of the main stream in km, Lc the length to the point nearest the catchment centroid in km, S the equivalent slope in m/km and A the catchment area in km². The subzone 3(d) relationships, loss curve, base flow and rainfall ratios, and the 3(g) loss rates, base flow and rainfall ratios, were read from scanned copies and checked against the page images for the edition of this Manual that covers the rest of those subzones; the 3(g) unit hydrograph relationships were read by optical character recognition only and are to be confirmed against the page images [TBC]; the 1(d) and 3(c) entries were read for the edition that covers the rest of those subzones; and the 3(f) entries are from the main text of the revised report of 1995, whose annexes and plates were missing from the copy read. The 50-yr and 100-yr point rainfalls were read by eye from the isopluvial plates, re-rendered for Chhattisgarh, and are good to about one contour; the 50-yr contours near Raigarh are unlabelled on Plate 9 of the 3(d) report. Several printing problems are to be resolved before use [TBC]. The 3(g) report had no gauged catchment of its own and adopted the relationships and loss rates of subzone 3(f) as they stood before the revision of 1995; that is why its column differs from the 3(f) column, and why its quoted "3(f)" loss rates of 5 and 2 mm/h differ from the 2 mm/h of the revised 3(f) report. Each column shall be used only in its own subzone. In subzone 3(f) the table of the report prints the tp term as L·Lc/S, while its text uses L·Lc/√S, which is the form given here. In subzone 1(d) the relationship table prints tp = 0.314 (L/√S)^1.102, but the worked example computes "0.314 (34.94/√3.70)^1.012 = 5.90 hrs" and the design-storm formula is TD = 0.345 (L/√S)^1.012; this Manual uses 1.012 and treats 1.102 as a misprint until the report is checked. The 3(d) report of 1997 kept the loss

curve of its 1982 edition, although 28 new events it examined lie considerably outside the curve, and the curve was read by eye [TBC]. The rainfall annexes have their own problems: in the 3(d) Annex 4.4 the Champa row is printed with its durations out of order (a 12-hour value below the 1-hour and 6-hour values, and a February date), and is not used; in the 3(g) Annex 4.4 the Jagdalpur 1-hour and 3-hour maxima are both printed as 80.0 mm, on a date that reads as February; and Kanker's 1-hour maximum of 26 June 1974 is 79 mm in the 3(d) annex and 75.3 mm in the 3(g) annex. The 3(d) worked example (railway bridge 385 near Sondur, 194 km², L = 39.36 km, Lc = 15.13 km, S = 4.36 m/km) lies just across the border in Odisha, on a river that rises in Chhattisgarh; the relationships give tp = 7.68 h, which the report rounds to 7.5 h, and with a 50-yr 24-hour point rainfall of 32.0 cm, an 8-hour ratio of 0.780, an ARF of 0.9144, a loss of 0.21 cm/h and a base flow of 19.40 m³/s the report gives a 50-yr peak of 1,086.5 m³/s, about 5.6 m³/s per km². The 1(d) and 3(c) worked examples lie outside the State. The design loss rates, the 3(d) curve of 3.5 to 1.5 mm/h, 5 and 2 mm/h in 3(g), 2 mm/h in 3(f), 2.5 mm/h in 1(d) and 1 mm/h in 3(c), are φ-indices fitted to the floods of whole rural catchments; they are not soil infiltration capacities and are not used to assign HSG (Clause 3.7) or to size infiltration practices (Chapter 4). The 3(d) curve is two to five times below the basic infiltration rate of 7.3–7.6 mm/h measured on a matasi-type sandy loam at Raipur (Commentary to Clause 3.7), because it was fitted to monsoon floods on catchments whose kanhar and dorsa paddy lowlands were already wet. The reports were derived from gauged rural catchments at railway and road bridges, none of them urban. In Chhattisgarh they apply to the rural nalas and rivers that enter city limits, such as the Kharun and its tributary nalas above Raipur, the streams of Nava Raipur, the Arpa above Bilaspur, the Kelo above Raigarh, which is itself one of the 3(d) gauged catchments, the Parri near Rajnandgaon, the nalas that reach Durg and Bhilai from the Seonath plain, the streams that reach Korba from the coalfield, and the nalas that reach Jagdalpur and Ambikapur from the plateau; they are not a model of city catchments. Where a city catchment lies within 25–5,000 km², the SUH is computed only as the benchmark of its natural condition (Clause 3.11.4), and the catchment itself is designed by the methods of Clause 3.4 with the design rainfall of Chapter 2. The Mahanadi at Rajim and below, the Seonath at Durg, the Hasdeo at Korba and the Indravati at Jagdalpur may be near or above 5,000 km² at the cities [TBC]; reaches above 5,000 km² are outside Clause 3.11.4 (Clause 3.4.5), and below Gangrel, Bango, Darri, the Bhaisajhar and Mongra barrages and the dams on the Indravati in Odisha the flood is set by releases (Chapter 5). The ratios are not to be used to derive intensities for durations shorter than the TD of the SUH (Chapter 2): with the 3(d) ratios the 50-yr 1-hour depth at Raipur is about 109–125 mm, while the largest reliable 1-hour falls recorded by self-recording gauges in the State are 79 mm at Kanker and 80 mm at Jagdalpur (Annex 4.4 of the 3(d) and 3(g) reports), and Chapter 2 deals with the difference. At 24 hours the opposite holds: Raipur (370.3 mm on 4 August 1910), Rajnandgaon (392.0 mm on 21 September 1965), Korba (358.0 mm on 12 August 2004) and Durg (337.0 mm on 15 June 2004) have recorded more than the 100-yr plate values (IMD), so the plate values are a floor under Clause 2.3.3, not a ceiling. Subzone by basin, read from the plates by eye and to be confirmed on the index maps of the reports [TBC]: 3(d), the Mahanadi with the Seonath, the Kharun, the Arpa, the Hasdeo, the Mand and the Kelo, and the Brahmani headwaters in Jashpur (Raipur, Nava Raipur, Durg, Bhilai, Rajnandgaon, Bilaspur, Korba, Janjgir, Champa, Raigarh, Dhamtari, Mahasamund, Rajim, Kanker, Kawardha, Mungeli, Bemetara, Balod, Manendragarh and most of Jashpur district); 3(g), the Indravati (Jagdalpur, Kondagaon, Narayanpur, the Indravati side of Dantewada, Bijapur and the Kotri side of Kanker district); 3(f), the Sabari and the Godavari frontage (Sukma, Konta and the south of Bijapur district) and the Wainganga fringe of Rajnandgaon and Khairagarh; 1(d), the Rihand, the Kanhar river and the Gopad (Ambikapur, Bishrampur, Surajpur, Balrampur, Ramanujganj and Wadrafnagar); and 3(c), the Narmada fringe of Kawardha and Rajnandgaon districts. No part of the State lies in subzone 3(e). The subzone is chosen by the catchment's outfall, not by its district: Jagdalpur lies on the 3(g)/3(f) line and drains to the Indravati, so its catchments are in 3(g), while the land south of the city that drains to the Sabari is in 3(f); Ambikapur lies just inside 1(d), and catchments that drain south to the Mand or the Hasdeo are in 3(d); and Kanker is in 3(d), with the Kotri side in 3(g). Where a catchment drains across the boundary between two subzones, or its outfall lies near one, as at Jagdalpur, Ambikapur, Kanker and Rajnandgaon–Khairagarh, this Manual requires the SUH to be computed with the parameters of each subzone and the higher 1% AEP peak to be adopted, unless calibration under Clause 3.15 supports the other.

Table 3E.2 — Worked example outline (layout)

Step	Content	Clause
1	Catchment area, main stream length, length to centroid and equivalent slope, from survey or DEM	3.11.4(a)

Step	Content	Clause
2	SUH parameters from Table 3E.1 and the plotted unit hydrograph	3.11.4(a)
3	Design storm duration, point and areal 24-hour rainfall with climate factor, time distribution	3.11.4(a), (b)
4	Effective rainfall after the design loss rate; convolution with the SUH; base flow added	3.11.4(a)
5	1% AEP peak compared with the HEC-HMS or SWMM peak for the existing natural condition	3.11.4(c), (d)

Form 3-1 — Hydrologic Calculation Summary

Supports Clauses 3.4, 3.6, 3.7, 3.8, 3.10–3.13, 3.15–3.17 of Chapter 3.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. One form per submittal. Attach the documents listed in Section H.

A. Project

Table 3F.1 — Form 3-1: project

Field	Entry
A1. Project name and applicant	
A2. Location (village or ward, survey numbers, district)	
A3. Site area (ha) and number of points of comparison (POCs)	
A4. Catchment class: standard (Clause 2.11.2) or flood-sensitive (Clause 2.11.3)	
A5. Design Rainfall Sheet reference (Form 2-1)	

B. Method and software (Clauses 3.4, 3.11, 3.12, 3.16)

Table 3F.2 — Form 3-1: method and software

Field	Entry
B1. Purpose of each calculation (conveyance, storage, comparison, catchment study)	
B2. Contributing area for each purpose (ha or km²)	
B3. Method used and the row of Table 3.1 that permits it	
B4. Reason for the choice of method	
B5. Software and version; engine and version for packages built on SWMM or HEC (Clause 3.16.1, 3.16.2)	
B6. For other software: benchmark results of Annex 3C attached (Table 3C.7) and the 5% comparison of Clause 3.16.2	
B7. Runoff transform (Clause 3.11.1)	
B8. Computation interval Δt (h), with 0.133 Tc of the smallest sub-area and the hyetograph time step (Clause 3.11.3(a))	
B9. Storage routing and channel routing methods (Clauses 3.12.1, 3.12.2)	
B10. Dynamic-wave routing required? If yes, which condition of Clause 3.12.4	
B11. CWC SUH benchmark required (25–5,000 km², Clause 3.11.4)? Result reference	
B12. Continuous simulation required (Clause 3.13.1)? Record used and starting tank levels (Clause 3.13.5)	
B13. Calibration required (Clause 3.15.1)? Calibration report reference	

C. Soils and groundwater (Clauses 3.3.3(b), 3.7, 3.10)

Table 3F.3 — Form 3-1: soils and groundwater

Field	Entry
C1. HSG source map, scale and lookup used (Clause 3.7.1, Annex 3A) or NEH Part 630, Ch. 7 assignment (Clause 3.7.2)	
C2. Infiltration tests: number, locations, depths, results (Clause 3.7.3)	
C3. Seasonal high water table: depth below ground and source (Clause 3.10.2)	
C4. Areas within 0.6 m assigned HSG D (Clause 3.10.1)	
C5. Disturbed pervious areas and HSG downgrade applied (Clause 3.3.3(b))	

D. Parameters by sub-area and condition (Clauses 3.3, 3.5, 3.8, 3.9; Clause 3.17 item 4)

Table 3F.4 — Form 3-1: parameters by sub-area and condition

Sub-area	Condition (pre / post)	Area (ha)	Impervious: roofs / paved / roads (ha)	DCIA (ha)	Disconnected (ha)	HSG	CN, AMC II	CN, AMC III (pervious part)	C and Cf (if Rational)	Tc (min)

E. Time of concentration worksheet (Clause 3.6)

One worksheet per sub-area and condition.

Table 3F.5 — Form 3-1: time of concentration worksheet

Segment	Flow type	Surface and n (Table 3.4)	L (m)	S (m/m)	P2 (mm)	V (m/s)	Tt (min)
1	Sheet					—	
2	Shallow concentrated				—		
3	Channel or pipe				—		
	Tc (not less than 10 min)						

E1. Is the post-development Tc shorter than the pre-development Tc? If not, explain (Clause 3.6.6): _____

F. Rainfall and losses (Clauses 3.8.4, 3.8.5, 3.11.5, 3.12, 3.3.5)

Table 3F.6 — Form 3-1: rainfall, losses and starting levels

Field	Entry
F1. AEPs and durations run; climate factor applied (Chapter 2)	
F2. AMC used for each duration (Clause 3.8.4)	
F3. Ia ratio for design runs; Ia = 0.05S sensitivity run made (Clause 3.8.5)	
F4. Peak rate factor used; if lower than 484, the Annex 3D value and the land it applies to (Clause 3.11.5)	
F5. Starting water level of each tank or waterbody, and its basis (Clauses 3.12.5, 3.13.5)	

Field	Entry
F6. Off-site run-on: land use assumed for conveyance and for the comparison (Clause 3.3.5)	
F7. Incidental storage counted? If yes, the authority's acceptance (Clause 3.12.3)	

G. Departures

Table 3F.7 — Form 3-1: departures from Chapter 3

No.	Clause departed from	Value used in place of the Chapter 3 value	Justification
1			
2			

H. Attachments (Clause 3.17)

Table 3F.8 — Form 3-1: attachments

Item	Attached (yes / no / not applicable)	Reference
H1. Catchment maps, pre- and post-development, with sub-areas, Tc flow paths, POCs and off-site run-on		
H2. HSG map, water-table data and infiltration test results		
H3. Parameter tables (Section D) and Tc worksheets (Section E)		
H4. Design Rainfall Sheet (Form 2-1) and hyetographs		
H5. Hydrograph tables and plots; stage–storage and stage–discharge tables; routed outflows; peak water levels		
H6. Form 3-2 for each POC		
H7. Calibration report (Clause 3.15)		
H8. Native model input and output files (Clause 3.16.4)		

I. Statement by the responsible engineer (Clause 3.17 item 10)

I state that the methods and parameters of Chapter 3 of the Chhattisgarh Stormwater Design Manual (Draft v1.0) have been used in this submittal, and that every departure is listed and justified in Section G.

Table 3F.9 — Form 3-1: statement by the responsible engineer

Field	Entry
Name	
Qualification and registration number	
Organisation	
Signature	
Date	

Form 3-2 — Pre- vs Post-Development Comparison

Supports Clauses 3.8.5, 3.11.5 and 3.14 of Chapter 3.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. One form for each point of comparison (POC). Flows in m³/s, volumes in m³.

A. Point of comparison (Clause 3.14.2)

Table 3F.10 — Form 3-2: point of comparison

Field	Entry
A1. POC identifier and location	
A2. Area draining to the POC: site (ha) and off-site run-on (ha)	
A3. Off-site run-on passes through site storage? (Clause 3.14.2(c))	
A4. Catchment class: standard (Clause 2.11.2) or flood-sensitive (Clause 2.11.3)	
A5. Tailwater applied, including joint cases of Clause 2.12 (Clause 3.14.9(c))	
A6. Flood-sensitive only: largest 10% AEP pre-development peak over all durations, the cap of Clause 2.11.3 (Clause 3.14.7)	
A7. Flood-sensitive only: 50% AEP pre-development peak at the pre-development critical duration (Clause 2.11.3)	

B. Peak flows (Clauses 3.14.7–3.14.10)

Limit: standard catchments 0.9 × Q_pre for the same AEP and duration (Clause 3.14.10(a)); flood-sensitive catchments the caps A6 (for 1% AEP) and A7 (for 50% AEP) (Clause 3.14.10(b)).

Table 3F.11 — Form 3-2: peak flows by AEP and duration

AEP	Duration (h)	AMC	Q_pre	Q_post, uncontrolled	Q_post, routed	Limit	Met?
50% (2-yr)	1	II					
50% (2-yr)	2	II					
50% (2-yr)	3	II					
50% (2-yr)	6	II					
50% (2-yr)	12	III					
50% (2-yr)	24	III					
50% (2-yr)	48	III					
50% (2-yr)	72	III					
10% (10-yr)	1	II					
10% (10-yr)	2	II					
10% (10-yr)	3	II					
10% (10-yr)	6	II					

AEP	Duration (h)	AMC	Q_pre	Q_post, uncontrolled	Q_post, routed	Limit	Met?
10% (10-yr)	12	III					
10% (10-yr)	24	III					
10% (10-yr)	48	III					
10% (10-yr)	72	III					
1% (100-yr)	1	II					
1% (100-yr)	2	II					
1% (100-yr)	3	II					
1% (100-yr)	6	II					
1% (100-yr)	12	III					
1% (100-yr)	24	III					
1% (100-yr)	48	III					
1% (100-yr)	72	III					

C. Volumes and storage (Clauses 3.14.8–3.14.10)

Table 3F.12 — Form 3-2: volumes and storage

AEP	Governing duration (h)	Runoff volume, pre	Runoff volume, post uncontrolled	Retention volume	Detention storage required	Peak water level in storage (m)
50% (2-yr)						
10% (10-yr)						
1% (100-yr)						

C1. Governing duration for storage, and the 24, 48 and 72 h results (Clause 3.14.10): _____

D. Sensitivity runs (Clauses 3.8.5, 3.14.11)

Table 3F.13 — Form 3-2: sensitivity runs

Run	Governing storm (AEP, duration)	Change in Q_post, routed (%)	Change in required storage (%)	Retention volume or WQv governing (Clause 3.8.5(c))
Ia = 0.05S				
Standard peak rate factor (only where a lower factor is used)				—

E. Associated checks (Clause 3.14.12)

Table 3F.14 — Form 3-2: associated checks

Check	Result	Reference in submittal
Drawdown times (Chapter 4)		
Minimum orifice and trash screen (Chapter 4)		
Emergency spillway (Chapter 4)		
Outfall-closed storage check (Chapter 5)		
Downstream analysis (Chapter 5)		

F. Statement by the responsible engineer

I state that this comparison has been made by the procedure of Clause 3.14, with the same design storms in the pre- and post-development runs, and that no runoff has been diverted from one natural catchment to another to meet the criterion (Clause 3.14.2(b)).

Table 3F.15 — Form 3-2: statement by the responsible engineer

Field	Entry
Name	
Qualification and registration number	
Signature	
Date	

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

ANNEXES TO CHAPTER 4

Site Runoff Control: Retention and Detention

These annexes support Chapter 4. Annex 4C is a complete Tier A worked example with routing tables, built on the illustrative example of Clause 4.6.7. Annex 4B sets out the structure of the Tier B tables, the provisional fixed release coefficients and a standard orifice table computed from the Chapter 4 orifice equation; the values that need NIH-DHAARA depths are still to be derived. Annex 4A needs GIS and groundwater data that this draft does not yet contain; it gives the intended layout and the interim rules of Chapter 4. Forms 4-1 to 4-3 are the submittal forms required by Clause 4.17. They are draft forms and have no statutory status.

Annex 4A — Flood-sensitive catchment boundaries (GIS); shallow-groundwater zone map

Supports Clauses 4.5.3, 4.5.4 and 4.12 of Chapter 4.

4A.1 **Flood-sensitive catchment boundaries.** The catchments are those listed in Clause 4.5.3.2, with any catchment notified under Clause 4.5.4. Their maps and GIS layer are the same data set as Annex 1B, which lists the catchments and gives the attribute layout of the layer. The two annexes shall be issued together.

4A.2 **Shallow-groundwater zone map.** This map shows the areas where Clause 4.12 applies: the kanhar and dorsa paddy lowlands of the Chhattisgarh plain, where the water table rises close to the surface in the monsoon, the floodplains of the Kharun, the Seonath, the Arpa, the Hasdeo, the Kelo, the Mahanadi and the Indravati and their tributaries, the beds and margins of the talabs, reservoirs and barrage ponds, among them the talabs of Raipur and the lakes of Nava Raipur, and the Hirakud backwater at Raigarh and Sarangarh, and any area where the groundwater table lies within 3 m of the surface at any time of year (Clause 4.12.1).

To be prepared. This annex will contain (a) the boundaries of the flood-sensitive catchments as a GIS layer and as maps at a scale fit for deciding whether a plot lies inside them, and (b) a map of the shallow-groundwater zone of Clause 4.12.1, with the seasonal high groundwater depth during and after the SW monsoon. It needs basin and sub-basin boundaries from a digital elevation model and the drainage network (WRD, NRSC, Survey of India), the full reservoir levels of the dams, reservoirs and barrages of WRD, among them Ravishankar Sagar (Gangrel), Minimata Bango, the Hasdeo barrage at Darri, Kharang, Maniyari, Kelo, the Arpa Bhaisajhar barrage and the Mongra barrage, and the levels of the lakes and reservoirs of Nava Raipur, the high flood points that WRD gives for land next to a nala, canal or river (checklist of the Directorate of Town and Country Planning, 2011), the extent of the Hirakud reservoir and its backwater within the State, the talabs and nalas recorded in the revenue records under section 251 of the Chhattisgarh Land Revenue Code 1959 and in the development plans, the line 50 m from the highest flood level that the State Wetland Authority applies round the talabs of Raipur and the Jhanjh wetland, the boundary of the Kopra Jalashay Ramsar site, and the lease maps of the coal and power companies (SECL, NTPC, CSPGCL, BALCO and others) with their mine voids, overburden dumps, ash ponds and ash dykes, town boundaries (the Directorate of Town and Country Planning, NRDA, RDA, the Municipal Corporations, Nagar Palika Parishads and Nagar Panchayats, and the Urban Administration and Development Department), flood maps held by CGSDMA, observation-well records of the CGWB and the Water Resources Department, and the extent of the kanhar, dorsa, matasi and bhata soils of the Chhattisgarh plain, the Chandi and Charmuria limestone belt of Raipur, Durg and Rajnandgaon, the soils, laterite and rock of the Bastar plateau and the northern hills, and the overburden dumps, mine-disturbed ground and ash ponds of the coalfields from soil survey maps (NBSS&LUP, the Chhattisgarh Department of Agriculture (soil survey, with Indira Gandhi Krishi Vishwavidyalaya, Raipur) [TBC]). Until it is published: (a) the catchments named in Clause 4.5.3.2 apply, and whether a site lies in one is decided and recorded as set out in Annex 1B; and (b) every site to which Clause 4.12.1 may apply establishes its design groundwater level from site piezometer readings over one June–November season or, failing that, from the nearest CGWB or State observation well plus 0.5 m and the design flood level of any adjacent river, nala, talab, reservoir or mine void (Clause 4.12.1(b)). For the flotation check in the kanhar and dorsa paddy lowlands, the talab and river floodplain margins and the Hirakud backwater without piezometer readings, the design groundwater level is existing ground level (Clause 4.12.1).

Table 4A.1 — Attributes of the shallow-groundwater zone layer (layout)

Field	Content	Unit or format
zone_id	Unique identifier	text
zone_class	kanhar or dorsa paddy lowland; river floodplain (Kharun, Seonath, Arpa, Hasdeo, Kelo, Mahanadi, Indravati or other); talab, reservoir or barrage-pond bed and margin (Raipur talabs, Nava Raipur lakes or other); Hirakud backwater (Raigarh, Sarangarh); other area with groundwater within 3 m of the surface (Clause 4.12.1)	text
shgw_depth_m	Seasonal high groundwater depth below ground, SW monsoon season	m
shgw_basis	Wells or piezometers used, and period of record	text
tidal_influence	Whether the groundwater level follows an adjacent tidal water body (Clause 4.12.1(b))	yes / no
source	Source agency and date of the data	text
geometry	Zone boundary	polygon

Annex 4B — Tier B tables: PSD and SSR (Table 4.3), retention credit factor C_R, standard outlet sizes; confirmed fixed release coefficients (Table 4.3A)

Supports Clauses 4.4.6.3, 4.14.1, 4.14.2 and 4.14.4 to 4.14.6 of Chapter 4.

4B.1 This annex will hold four tables. Their status in this draft is:

Table 4B.1 — Contents of Annex 4B and their status

Table	Content	Clause	Status in this draft
4B.2	Tier B permissible site discharge (PSD) and site storage requirement (SSR), the values of Table 4.3	4.14.2	To be prepared
4B.3	Retention credit factor C_R	4.4.6.3	To be prepared; C_R = 0 until published
4B.4	Standard outlet sizes	4.14.2 step 4, 4.14.6	Computed in this annex from the orifice equation of Clause 4.6.5.2
4B.5	Fixed release coefficients C_10 and C_2 (Table 4.3A)	4.14.4	Provisional values of Table 4.3A; confirmed values to be prepared

4B.2 PSD and SSR (Table 4.3)

To be prepared. This table will give, for each rainfall zone, hydrologic soil group and post-development imperviousness, the PSD for the 2-yr and 100-yr stages and the SSR, for standard and flood-sensitive catchments, in the layout of Table 4.3. It needs NIH-DHAARA design depths with the climate factor for each zone (Chapter 2, Annex 2A), the sub-daily ratios of Annex 2B, and routing of a set of standard sites by the full method of Clause 4.6 for every AEP and duration of Table 4.1. Each PSD shall be the governing release cap and each SSR the governing storage per hectare, with a margin so that the simplified method is never cheaper than routing. Until it is published, Tier B sites use Clause 4.6, or the fixed-coefficient route of Clause 4.14.3 where the site is 1 ha or less (Clauses 4.14.1 and 4.14.3).

Table 4B.2 — Tier B PSD and SSR (layout of Table 4.3)

Zone (Annex 2A)	HSG	Imperviousness	PSD 2-yr (L/s/ha)	PSD 100-yr (L/s/ha), standard	PSD 100-yr (L/s/ha), flood-sensitive	SSR (m^3/ha), standard	SSR (m^3/ha), flood-sensitive
CG-Z1 Mahanadi plain: CG-A Raipur–Durg–Bhilai–Rajnandgaon plain (Seonath–Kharun, limestone)	B / C / D	50% / 70% / 90%	—	—	—	—	—
CG-Z1 Mahanadi plain: CG-B Bilaspur–Mungeli–Kawardha (Arpa–Maniari–Hamp)	B / C / D	50% / 70% / 90%	—	—	—	—	—
CG-Z1 Mahanadi plain: CG-C Korba–Janjgir–Champa–Raigarh (Hasdeo–Mand–Kelo coalfields)	B / C / D	50% / 70% / 90%	—	—	—	—	—

Zone (Annex 2A)	HSG	Imperviousness	PSD 2-yr (L/s/ha)	PSD 100-yr (L/s/ha), standard	PSD 100-yr (L/s/ha), flood-sensitive	SSR (m³/ha), standard	SSR (m³/ha), flood-sensitive
CG-Z1 Mahanadi plain: CG-D South-east plain (Dhamtari–Mahasamund–Gariaband–Rajim)	B / C / D	50% / 70% / 90%	—	—	—	—	—
CG-Z2 Bastar plateau: CG-E Bastar plateau and Godavari frontage	B / C / D	50% / 70% / 90%	—	—	—	—	—
CG-Z3 Northern hills: CG-F Northern hills (Surguja–Koriya–Jashpur)	B / C / D	50% / 70% / 90%	—	—	—	—	—

4B.3 Retention credit factor C_R

To be prepared. This table will give C_R for each rainfall zone, hydrologic soil group and design infiltration or drawdown rate of the retention practice (Clauses 4.4.3 and 4.4.6.3). It needs routing of the full duration set of Clause 2.5.2 with NIH-DHAARA depths, with the retention practice modelled as in Clause 4.4.6.2, and the credit expressed as the reduction in governing storage per cubic metre of retention provided. Until it is published, **C_R = 0** (Clause 4.4.6.3).

Table 4B.3 — Retention credit factor C_R (layout)

Zone (Annex 2A)	HSG	Design infiltration or drawdown rate (mm/h)	C_R (–)

4B.4 Standard outlet sizes

4B.4.1 Table 4B.4 gives the largest circular orifice, in 5 mm steps, whose discharge does not exceed a stated permissible discharge at a stated head. It is computed from the orifice equation with the discharge coefficient of Clause 4.6.5.2:

$$Q = 0.60 \times (\pi d^2 / 4) \times \sqrt{(2 g H)}$$

where Q is the discharge (m³/s), d the orifice diameter (m), g = 9.81 m/s² and H the head from the water level to the centre of the orifice (m). Sizes are rounded **down**, so that the orifice never passes more than the permissible discharge. Where the computed size is below 50 mm, the 50 mm minimum of Clause 4.8.2 is shown; it is deemed to comply under Clause 4.5.5.1.

4B.4.2 The table assumes free discharge. Where the tailwater can rise above the orifice (Clause 4.6.6), H is the difference between the upstream and downstream water levels and the orifice shall be checked by routing. The State Stormwater Technical Committee may replace this table when it issues Annex 4B.

Table 4B.4 — Largest orifice diameter (mm) for a permissible discharge at a given head (Cd = 0.60, free discharge)

Permissible discharge (L/s)	H = 0.5 m	H = 1.0 m	H = 1.5 m	H = 2.0 m
5	55	50	50	50
10	80	65	60	55
15	100	80	75	70
20	115	95	85	80
30	140	115	105	100
50	180	150	135	130
75	225	185	170	155
100	260	215	195	180
150	315	265	240	225
200	365	305	275	260
300	450	375	340	315

Check: for 50 L/s at H = 1.0 m, the area is 0.050 / (0.60 × √(2 × 9.81 × 1.0)) = 0.0188 m², so d = √(4 × 0.0188 / π) = 0.155 m, rounded down to 150 mm.

4B.5 Fixed release coefficients (Table 4.3A)

4B.5.1 Table 4B.5 repeats the provisional coefficients of Table 4.3A. They may be used now under the fixed-coefficient route (Clause 4.14.4).

Table 4B.5 — Fixed release coefficients C_10 and C_2 (provisional, Table 4.3A)

Hydrologic soil group	C_10	C_2	Clause	Status
A (sandy, well drained)	0.05	0.02	4.14.4	Provisional
B	0.08	0.03	4.14.4	Provisional
C	0.15	0.08	4.14.4	Provisional
D (clay, poorly drained)	0.25	0.15	4.14.4	Provisional

To be prepared. The confirmed coefficients will replace Table 4B.5. They need NIH-DHAARA depths for each zone and the greenfield curve numbers of Clause 3.3, as described in the commentary to Clause 4.14. Until they are published, the provisional values of Table 4.3A apply.

Annex 4C — Worked example (Tier A) with full routing tables

Supports Clauses 4.4 to 4.9, 4.15 and 4.17 of Chapter 4.

Illustrative — not design values. This example continues the illustrative example of Clause 4.6.7. Its inputs are invented to be plausible for a colony draining to the Kharun in Raipur; they are not NIH-DHAARA values and must not be used for design. Part 1 sets out the inputs, hydrographs and routing step by step, for a trial outlet. Part 2 finds that the trial outlet

fails the drawdown check with the blockage of Clause 4.8.3, and revises it to the outlet reported in Table 4.2. Part 3 completes the spillway, outlet, safety and retention checks. The outfall-closed check of Chapter 5 for the same facility on an outfall to a nala draining to the Kharun in Raipur is in Annex 5C.

Part 1 — Inputs, hydrographs and routing (Clauses 4.6.1 to 4.6.4)

4C.1 **Inputs.**

Table 4C.1 — Inputs (Illustrative — not design values)

Item	Value	Basis
Site area A	2.0 ha (0.02 km²), one outfall	Clause 4.6.7
Catchment class	Flood-sensitive (Clause 4.5.3)	Clause 4.6.7
Land cover and curve numbers, AMC II	Pre-development: all pervious, CN 70. Post-development, as two sub-areas: directly connected impervious 1.57 ha at CN 98, and pervious open space 0.43 ha at CN 70 (same soil as pre-development). Impervious fraction f = (92 − 70) / (98 − 70) = 0.786, so the area-weighted CN is 92	Clauses 3.9.2, 4.6.7
AMC III conversion	Pervious areas only, for storms of 12 h and longer: CN_III = 23·CN / (10 + 0.13·CN) = 84.3. The impervious CN 98 is not converted	Clause 3.8.4
Initial abstraction	Ia = 0.2S; Ia = 0.05S as a sensitivity run	Clause 3.8.5
Time of concentration Tc	45 min pre-development; 15 min post-development, used for both sub-areas, because the open space is small and drains through the same site system	Clause 4.6.7
Unit hydrograph	SCS triangular; tL = 0.6 Tc; tp = Δt/2 + tL; base 2.67 tp	Clauses 3.6.5, 3.11.2
Time step Δt	5 min (= Tc/3 of the post-development site)	Clause 4.6.2, step 8
24-h depths, 2-, 10-, 100-yr	110, 200, 350 mm, taken as including the climate factor	Clause 4.6.7
Shorter durations	P(t) = P24 · (t/24)^0.40 (assumed ratio set, not Annex 2B)	Clause 4.6.7
48-h and 72-h depths	1.25 and 1.40 × P24, with the 24-h depth on the last day	Clauses 4.6.7, 2.9.2
Storage	Open, vertical-walled, 2,500 m² plan area, excavated, no embankment, not publicly accessible	Clause 4.6.7
Trial outlet	Stage 1: 120 mm orifice at the floor. Stage 2: 470 mm orifice, invert 0.533 m above the floor. No Stage 3	Trial for this annex; Part 2 revises it
Outfall	Free outfall to a receiving drain whose level stays below the outlet invert	Illustrative input

The example follows Clauses 3.8.4 and 3.9.2: the runoff of each sub-area is computed separately and the two are added before routing.

4C.2 **Design rainfall.** Table 4C.2 gives the depths. For example, the 100-yr 6-hour depth is 350 × (6/24)^0.40 = 350 × 0.574 = 201.0 mm.

Table 4C.2 — Design rainfall depths, mm (Illustrative — not design values)

Duration (h)	1	2	3	6	12	24	48	72
50% AEP (2-yr)	30.9	40.7	47.9	63.2	83.4	110.0	137.5	154.0

Duration (h)	1	2	3	6	12	24	48	72
10% AEP (10-yr)	56.1	74.0	87.1	114.9	151.6	200.0	250.0	280.0
1% AEP (100-yr)	98.2	129.5	152.3	201.0	265.3	350.0	437.5	490.0

4C.3 **Hyetographs.** Storms up to 24 h use the alternating-block method in 5-minute blocks with the peak block at mid-storm (Clause 2.9.1). The 48- and 72-hour storms spread the depth in excess of the 24-hour depth uniformly over the first 24 or 48 h and place the 24-hour alternating-block storm on the last day (Clause 2.9.2). For the 100-yr 72-hour storm: (490.0 − 350.0) / 576 blocks = 0.243 mm per 5 minutes for the first 48 h; the peak 5-minute block on the last day is 350 × (5/1440)^0.40 = 36.3 mm, at 60:00–60:05 h.

4C.4 **Losses.** Table 4C.3 gives S = 25,400/CN − 254 (mm) and Ia = 0.2S. The runoff depth for a rainfall depth P is Q = (P − Ia)² / (P − Ia + S) for P > Ia, and zero otherwise.

Table 4C.3 — Curve numbers and abstractions (Illustrative — not design values)

Condition	AMC	CN	S (mm)	Ia = 0.2S (mm)
Pre-development	II (durations ≤ 6 h)	70	108.9	21.8
Pre-development	III (durations ≥ 12 h)	84.3	47.3	9.5
Post-development pervious open space (0.43 ha)	II (durations ≤ 6 h)	70	108.9	21.8
Post-development pervious open space (0.43 ha)	III (durations ≥ 12 h)	84.3	47.3	9.5
Post-development directly connected impervious (1.57 ha)	All durations (not converted)	98	5.2	1.0

Check: 100-yr 24 h, post-development. Impervious: Q = (350.0 − 1.0)² / (350.0 − 1.0 + 5.2) = 343.9 mm. Pervious, AMC III: Q = (350.0 − 9.5)² / (350.0 − 9.5 + 47.3) = 299.0 mm. Site: 0.786 × 343.9 + 0.214 × 299.0 = 334.2 mm. The post-development columns of Table 4C.4 are these area-weighted site depths.

Table 4C.4 — Storm runoff depth Q, mm (Illustrative — not design values)

Duration (h)	AMC	2-yr pre	2-yr post	10-yr pre	10-yr post	100-yr post
1	II	0.7	20.1	8.2	41.3	79.2
2	II	2.8	28.2	16.9	57.2	108.5
3	II	5.1	34.2	24.5	69.0	130.2
6	II	11.4	47.5	42.9	94.7	177.1
12	III	45.0	70.5	106.6	137.2	249.9
24	III	68.4	96.4	152.6	185.1	334.2
48	III	93.5	123.3	201.0	234.7	421.5
72	III	108.9	139.6	230.3	264.5	473.9

The runoff volume is 10 × Q × A = 20 × Q m³ for this 2.0 ha site: for example 9,478 m³ for the 100-yr 72-hour storm. The routed hydrographs carry up to 0.5% less, because the 5-minute alternating-block storm places 349.5 mm rather than 350.0 mm on the last day.

4C.5 **Unit hydrographs.** The runoff in each 5-minute block (for the post-development site, the sum of the two sub-areas) is convolved with the triangular unit hydrograph of Table 4C.5. The peak is qp = 0.208 × A × Q / tp, with A in km² and Q = 1 mm (Clause 3.11.2).

Table 4C.5 — Unit hydrograph parameters (Illustrative — not design values)

Condition	Tc (h)	tL = 0.6 Tc (h)	tp = Δt/2 + tL (h)	Base 2.67 tp (h)	qp (L/s per mm of runoff)
Pre-development	0.75	0.45	0.492	1.313	8.5
Post-development	0.25	0.15	0.192	0.512	21.7

4C.6 **Release caps (Clause 4.5.3).** The site is flood-sensitive, so the mixed-frequency rule applies. The 100-yr cap is the largest pre-development 10-yr peak over all durations: **356 L/s** (72 h; Table 4.2). The 2-yr cap is the largest pre-development 2-yr peak: **180 L/s** (72 h). Each cap applies to every post-development duration (Clause 4.5.3.1).

4C.7 **Stage–storage–discharge.** The basin is vertical-walled, so storage S = 2,500 × h (m³), where h is the stage above the floor (m). Outlet discharge follows Clause 4.6.5.2 with Cd = 0.60:

- Stage 1, for h ≥ d: Q1 = 0.60 × A1 × √(2g (h − d/2)). For h < d (orifice part-full), the head is taken as d/2 and the result is multiplied by h/d.
- Stage 2 is counted once the water is above its centre: Q2 = 0.60 × A2 × √(2g (h − z2 − d2/2)), where z2 is its invert. Below its centre its flow is ignored, which slightly over-states storage.

Table 4C.6 — Stage–storage–discharge, trial outlet (Illustrative — not design values)

Stage h (m)	Storage (m³)	Stage 1, 120 mm (L/s)	Stage 2, 470 mm (L/s)	Total (L/s)
0.00	0	0.0	0.0	0.0
0.06	150	3.7	0.0	3.7
0.12	300	7.4	0.0	7.4
0.20	500	11.2	0.0	11.2
0.30	750	14.7	0.0	14.7
0.40	1,000	17.5	0.0	17.5
0.50	1,250	19.9	0.0	19.9
0.60	1,500	22.1	0.0	22.1
0.70	1,750	24.0	0.0	24.0
0.80	2,000	25.9	83.0	108.9
0.90	2,250	27.5	167.8	195.3
1.00	2,500	29.1	222.3	251.4
1.10	2,750	30.7	265.8	296.5
1.20	3,000	32.1	303.2	335.3
1.30	3,250	33.5	336.4	369.9
1.40	3,500	34.8	366.7	401.5

Check at h = 1.00 m: Q1 = 0.60 × 0.01131 × √(19.62 × 0.94) = 0.0291 m³/s; Q2 = 0.60 × 0.1735 × √(19.62 × (1.00 − 0.533 − 0.235)) = 0.2223 m³/s.

4C.8 **Routing.** Each storm is routed by level-pool continuity (Clause 4.6.1): over each 5-minute step, the change in storage equals inflow volume minus outflow volume. The computation used six 50-second sub-steps per 5-minute step. Each row of Table 4C.7 can be checked by hand as ΔS ≈ (I − Ō) × 300 s, where I is the mean inflow over the 5-minute

step ending at the time shown and $\bar{O}$ is the mean of the outflows at the start and end of the step. For example, between 60:15 and 60:20: $\Delta S = 2{,}853.0 - 2{,}613.3 = 239.7$ m³, and $(1.0891 - (0.2655 + 0.3069)/2) \times 300 = 240.9$ m³.

Table 4C.7 — Routing of the 100-yr 72-hour storm through the trial outlet, around the peak (Illustrative — not design values)

Time (h:min)	Rainfall in step (mm)	Inflow I (L/s)	Outflow O (L/s)	Storage S (m³)	Stage h (m)
60:00	8.44	286.3	169.5	2,164.0	0.866
60:05	36.33	354.2	184.1	2,216.9	0.887
60:10	11.61	703.3	218.0	2,367.1	0.947
60:15	6.88	1,066.0	265.5	2,613.3	1.045
60:20	5.23	1,089.1	306.9	2,853.0	1.141
60:25	4.34	897.5	334.8	3,025.0	1.210
60:30	3.77	653.2	349.9	3,117.6	1.247
60:35	3.36	403.6	353.8	**3,132.9**	**1.253**
60:40	3.05	301.5	352.3	3,117.3	1.247
60:45	2.81	254.8	348.6	3,088.7	1.235
60:50	2.61	224.9	343.7	3,052.4	1.221
60:55	2.45	203.3	337.9	3,011.3	1.205

Storage is greatest at the end of the step ending at 60:35. In the next step the mean inflow (302 L/s) is below the outflow (about 352 L/s), so storage falls.

Table 4C.8 — Routing of the 100-yr 72-hour storm through the trial outlet, hourly (Illustrative — not design values)

Time (h)	Cumulative rainfall (mm)	Inflow (L/s)	Outflow (L/s)	Stage (m)	Storage (m³)
12	35.0	14	8	0.130	325
24	70.0	15	12	0.215	536
36	105.0	16	14	0.264	660
48	140.0	16	15	0.294	736
54	181.7	47	19	0.469	1,172
57	213.5	69	23	0.623	1,559
59	248.5	120	91	0.789	1,971
60	293.3	286	170	0.866	2,164
61	378.0	187	332	1.187	2,967
62	399.3	106	237	0.968	2,421
63	414.5	80	142	0.829	2,072
66	446.9	51	52	0.771	1,927
72	489.5	33	33	0.768	1,920
84	489.5	0	18	0.405	1,012

Time (h)	Cumulative rainfall (mm)	Inflow (L/s)	Outflow (L/s)	Stage (m)	Storage (m³)
96	489.5	0	10	0.167	418
108	489.5	0	4	0.058	146

The routed outflow volume equals the inflow volume (about 9,470 m³): detention delays the volume but does not remove it.

4C.9 **Results, trial outlet.** Table 4C.9 gives the peaks, maximum stages and drawdown times for the trial outlet.

Table 4C.9 — Results for the trial outlet (120 mm and 470 mm orifices) (Illustrative — not design values)

Duration (h)	2-yr detained (L/s)	2-yr max stage (m)	10-yr detained (L/s)	10-yr max stage (m)	100-yr detained (L/s)	100-yr max stage (m)	100-yr storage (m³)	10-yr 90% drawdown, clean (h)	10-yr 90% drawdown, 50% blocked (h)
1	9	0.15	15	0.30	22	0.59	1,468	27.2	54.2
2	11	0.20	18	0.42	89	0.79	1,966	28.3	57.0
3	13	0.24	20	0.50	135	0.82	2,058	29.3	59.1
6	15	0.31	23	0.64	190	0.89	2,229	31.1	63.4
12	17	0.40	49	0.77	279	1.06	2,647	36.2	69.2
24	19	0.45	87	0.79	334	1.20	2,991	42.8	75.4
48	20	0.51	144	0.83	353	1.25	3,129	42.5	72.7
72	20	0.52	147	0.84	354	1.25	**3,133**	42.5	71.9

The drawdown time is measured from the time of maximum 10-yr storage until 90% of that storage has been released (Clause 4.7.1).

Part 2 — Checks and revision of the outlet (Clauses 4.5, 4.7 and 4.8)

4C.10 **Release caps.** Every 100-yr detained peak is at or below 356 L/s, and every 2-yr detained peak is far below the 2-yr cap of 180 L/s. Both caps are met. The 2-yr peaks are not compared duration by duration with the pre-development peaks: for a flood-sensitive site the 2-yr cap is the single largest pre-development 2-yr peak (Clause 4.5.3.1).

4C.11 **Drawdown.** Clause 4.8.3 requires the low-flow orifice to be assumed **50% blocked** when drawdown is checked. With the 120 mm orifice at half its area, 90% of the 10-yr storage takes 54 to 75 h to drain (Table 4C.9), against the 48-hour limit of Clause 4.7.1. **The trial outlet fails.** With a clean orifice it would drain in about 42 h.

4C.12 **Revised outlet.** The low-flow orifice was increased in 10 mm steps until the blocked drawdown met 48 h for every duration, and Stage 2 was then re-sized so that the 100-yr cap was still met. The result:

- Stage 1: **170 mm** orifice at the floor;
- Stage 2: **460 mm** orifice, invert 0.53 m above the floor (centre at 0.76 m);
- no Stage 3.

The larger low-flow orifice releases more in frequent storms. That is acceptable here because the 2-yr cap of a flood-sensitive site is the largest pre-development 2-yr peak (Clause 4.5.3.1). The WQv is assumed to be treated in a separate practice upstream (Chapter 8), so Stage 1 is not required to hold the WQv for 24–48 h (Clause 4.6.5.1); where Stage 1 also releases the WQv, Clause 4.7.2 must be checked as well.

Table 4C.10 — Stage–storage–discharge, revised outlet (Illustrative — not design values)

Stage h (m)	Storage (m³)	Stage 1, 170 mm (L/s)	Stage 2, 460 mm (L/s)	Total (L/s)
0.00	0	0.0	0.0	0.0
0.085	213	8.8	0.0	8.8
0.17	425	17.6	0.0	17.6
0.30	750	28.0	0.0	28.0
0.40	1,000	33.9	0.0	33.9
0.50	1,250	38.9	0.0	38.9
0.60	1,500	43.3	0.0	43.3
0.70	1,750	47.3	0.0	47.3
0.76	1,900	49.6	0.0	49.6
0.80	2,000	51.0	88.3	139.3
0.90	2,250	54.5	165.3	219.7
1.00	2,500	57.7	216.4	274.1
1.10	2,750	60.8	257.5	318.3
1.17	2,925	62.8	282.8	345.6
1.20	3,000	63.7	293.0	356.7
1.30	3,250	66.5	324.6	391.1

Table 4C.11 — Results for the revised outlet (170 mm and 460 mm orifices) (Illustrative — not design values)

Duration (h)	2-yr detained (L/s)	2-yr max stage (m)	10-yr detained (L/s)	10-yr max stage (m)	10-yr storage (m³)	100-yr detained (L/s)	100-yr max stage (m)	100-yr storage (m³)	10-yr 90% drawdown, 50% blocked (h)
1	15	0.14	28	0.29	734	42	0.57	1,426	31.3
2	20	0.19	34	0.40	990	79	0.76	1,911	32.1
3	22	0.22	37	0.46	1,147	139	0.80	1,999	32.8
6	25	0.26	41	0.56	1,386	194	0.86	2,154	34.2
12	30	0.33	46	0.68	1,692	276	1.01	2,512	39.0
24	32	0.37	49	0.75	1,870	320	1.11	2,766	45.3
48	34	0.40	82	0.77	1,913	343	1.17	**2,913**	44.8
72	34	0.41	84	0.77	1,915	343	1.16	2,911	44.7

Findings:

- **100-yr cap:** largest detained peak 343 L/s ≤ 356 L/s. Met.
- **2-yr cap:** largest detained peak 34 L/s ≤ 180 L/s. Met.
- **Drawdown:** longest blocked drawdown 45.3 h ≤ 48 h (Clause 4.7.1). Met.

- **Governing storage: 2,913 m³** (100-yr, 48 h), at a maximum stage of **1.17 m**. The larger low-flow orifice releases more water early in the long storms, so the revised outlet needs about 220 m³ less storage than the trial outlet. Table 4.2 in Clause 4.6.7 reports this revised design.
- **Governing storms (Clause 4.6.2, step 12):** storage, the 100-yr 48-hour storm; Stage 1, the 10-yr 24-hour storm (drawdown); Stage 2, the 100-yr 48-hour storm (cap).

Part 3 — Sensitivity, retention, spillway, outlet details and safety

4C.13 **Ia = 0.05S sensitivity (Clause 3.8.5).** With S converted as S0.05 = 25.4 × 1.33 × (S0.20 / 25.4)^1.15, the governing 100-yr 48-hour run gives 2,898 m³ of storage and a 341 L/s peak, slightly below the default run. The pre-development 10-yr peak, and so the 100-yr cap, falls to 344 L/s. The design is based on the default run (Clause 3.8.5(c)); the sensitivity results are reported on Form 3-2.

4C.14 **Retention and retention credit (Clauses 4.4.2 and 4.4.6).** With an illustrative retention depth P_R = 40 mm, Q_R is computed for each sub-area at AMC II, because a composite CN is not used for retention (Clause 3.9.4). Impervious, CN 98: S = 5.2 mm, Ia = 1.0 mm, Q_R = (40 − 1.0)² / (40 − 1.0 + 5.2) = 34.4 mm. Pervious, CN 70: S = 108.9 mm, Ia = 21.8 mm, Q_R = (40 − 21.8)² / (40 − 21.8 + 108.9) = 2.6 mm. RRV = 10 × (34.4 × 1.571 + 2.6 × 0.429) = **552 m³**. The retention is modelled as a store that is empty at the start of the storm and captures all runoff until full, with no infiltration during the storm (Clause 4.4.6.2).

Table 4C.12 — Retention credit by routing, revised outlet (Illustrative — not design values)

100-yr storm	Storage without retention (m³)	Storage with retention (m³)	Reduction (m³)
24 h	2,766	2,628	138
48 h	2,913	2,906	7
72 h	2,911	2,911	0

The governing storage falls from 2,913 to 2,906 m³, so the retention credit is 7 m³, about 1% of the RRV. The retention is still required in full for its own purposes (Clause 4.4.1, Chapter 8).

4C.15 **Emergency spillway (Clause 4.9).** The facility is excavated, with no embankment, so Clause 4.9.7 applies: the overflow route is treated as the spillway and must meet Clause 4.9.1.

- Design flow: the 1% AEP peak inflow, 1,089 L/s, with the outlet blocked and the basin full to the crest.
- Crest: the 1% AEP maximum water level with the outlet working is 1.17 m; with the recommended 0.15 m clearance (Clause 4.9.3) the crest is set at **1.35 m**.
- Crest length: a broad-crested weir with Q = C × L × H^1.5, C = 1.7 (an illustrative coefficient for a broad-crested weir in SI units; Clause 4.6.5.2 requires a coefficient suited to the shape). With L = 4.0 m: H = (1.089 / (1.7 × 4.0))^(2/3) = 0.295 ≈ **0.30 m**.
- Maximum level with the spillway flowing: 1.35 + 0.30 = 1.65 m. With 0.3 m freeboard (Clause 4.9.1), the top of the walls is at least **1.95 m** above the floor.
- The spillway discharges to the 1% AEP overland flow path, away from buildings (Clause 4.9.4).

4C.16 **Outlet details (Clause 4.8).**

Table 4C.13 — Outlet checks, revised outlet (Illustrative — not design values)

Item	Requirement	Value	Result
Minimum orifice	≥ 50 mm (Clause 4.8.2)	170 mm	Met

Item	Requirement	Value	Result
Rack bar spacing	≤ half the smallest opening and ≥ 25 mm (Clause 4.8.3)	≤ 85 mm	Set at 75 mm
Rack open area	≥ 10 × area protected (Clause 4.8.3)	10 × (0.0227 + 0.1662) m² = 1.89 m² for one rack around both orifices	Provide ≥ 1.89 m²
Anti-vortex, Stage 1	Needed if head > 1.5 d (Clause 4.8.4)	Head up to 1.08 m > 0.26 m	Required
Anti-vortex, Stage 2	Needed if head > 1.5 d (Clause 4.8.4)	Head above centre up to 0.41 m < 0.69 m	Not required
Isolation	Facility serving > 1 ha (Clause 4.8.6)	Serves 2.0 ha	Stop-log slot or penstock, normally open
Outlet barrel	≥ 300 mm (Clause 4.8.5)	Sized under Chapter 9 for 343 L/s	To be designed

4C.17 **Safety (Clause 4.15).** The 10-yr depth (0.77 m) exceeds the 0.45 m limit for publicly accessible storage (Clause 4.15.1(a)), so the facility is not open to the public. It has vertical walls higher than 0.6 m and a depth of more than 1.2 m when the spillway flows, so a non-climbable fence at least 1.2 m high with a lockable gate is required (Clause 4.15.3). Signs and a depth marker are provided (Clause 4.15.5).

4C.18 **Summary of the design (Illustrative — not design values).**

Table 4C.14 — Summary of the revised design (Illustrative — not design values)

Item	Value
Storage	2,500 m² plan area; 2,913 m³ at the 1% AEP maximum level of 1.17 m
Stage 1	170 mm orifice at the floor, with trash rack and anti-vortex plate
Stage 2	460 mm orifice, invert 0.53 m
Spillway	Crest 1.35 m, length 4.0 m, maximum level 1.65 m
Top of walls	≥ 1.95 m above the floor
100-yr peak release	343 L/s (cap 356 L/s)
2-yr peak release	34 L/s (cap 180 L/s)
10-yr drawdown, 50% blocked	45.3 h (limit 48 h)
Retention	RRV 552 m³ provided in addition; credit 7 m³
Still to be checked	Outfall-closed check on an outfall to a nala draining to the Kharun in Raipur (Annex 5C); downstream analysis (Chapter 5); groundwater and flotation (Clause 4.12); WQv (Chapter 8)

Form 4-1 — Site Runoff Control Report

Supports Clauses 4.3 to 4.17 of Chapter 4.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. One form for each site. Where the site drains to more than one outfall, complete Sections E to J for each outfall (Clause 4.5.1.3). Tier B sites using Clause 4.14.2 attach Form 4-2 in place of Sections F and G; sites using the fixed-coefficient route attach Form 4-3 in place of Sections F and G (Clause 4.17).

A. Site and route

Table 4F.1 — Form 4-1: site and route

Field	Entry
A1. Project name and applicant	
A2. Location (village or ward, survey numbers, district)	
A3. Approving Authority and tier (Tier A / Tier B)	
A4. Site area, land disturbance and new or replaced impervious area (ha, ha, m²) (Clause 4.1.2)	
A5. Number of outfalls (Clause 4.5.1.3)	
A6. Sizing route: full method (Clause 4.6) / Tier B tables (Clause 4.14.2) / fixed-coefficient route (Clause 4.14.3)	
A7. Constrained redevelopment route used (Clause 4.5.6)? If yes, attach the Constraint Statement, the existing-condition model and the shortfall and fee computation	

B. Inputs (Clause 4.17, item 1)

Table 4F.2 — Form 4-1: inputs

Field	Entry
B1. Design Rainfall Sheet reference (Form 2-1)	
B2. Hydrologic Calculation Summary reference (Form 3-1)	
B3. HSG of each sub-area and the tests that confirm it (Clause 4.6.2, step 1)	
B4. Pre-development condition and CN (Clause 4.5.1.2)	
B5. Post-development CN of each sub-area, and Tc pre and post	
B6. Routing method and software, with version; time step (Clauses 4.6.1, 4.6.2)	

C. Order of measures (Clause 4.3.1; Clause 4.17, item 2)

Table 4F.3 — Form 4-1: order of measures

Step	What the design does	Why this step cannot do more
1. Preserve		
2. Retain		

Step	What the design does	Why this step cannot do more
3. Detain		
4. Convey		

D. Retention (Clause 4.4; Clause 4.17, items 3 and 4)

Table 4F.4 — Form 4-1: retention

Field	Entry
D1. Retention depth P_R (mm) and zone (Annex 2E)	
D2. Runoff depth Q_R (mm) by sub-area and RRV (m³) (Clause 4.4.2)	
D3. Retention practices and volume provided (m³)	
D4. Infiltration tests: number, locations and depths, measured rates (mm/h) (Clause 4.4.3(a))	
D5. Design infiltration rate (mm/h), not more than half the measured rate (Clause 4.4.3(b))	
D6. Seasonal high groundwater depth below the base of each practice (m), at least 1.0 m (Clause 4.4.3(c))	
D7. Reuse demand calculation, where retention is by reuse (Clause 4.4.3)	
D8. Retention drawdown time (h), not more than 72 h (Clause 4.4.5)	
D9. Shortfall in RRV (m³), if any, treated as WQv (Clause 4.4.4.2)	
D10. Governing storage without and with retention, and the retention credit (m³) (Clause 4.4.6)	
D11. Preservation credit areas and covenant reference (Clause 4.4.7)	

E. Release criteria (Clause 4.5; Clause 4.17, item 5)

Table 4F.5 — Form 4-1: release criteria

Field	Entry
E1. Catchment class: standard (Clause 4.5.2) or flood-sensitive (Clause 4.5.3), with the Annex 4A map extract	
E2. Release caps by AEP (L/s), and the duration that sets each cap	
E3. Clause 4.5.5 used (50 mm orifice deemed to comply, or the short-duration 2-yr relief)? Report any 2-yr exceedance	

F. Results (Clause 4.6; Clause 4.17, item 6)

Complete one row for each duration, in the format of Table 4.2. Mark the governing case for storage, for each outlet stage and for the spillway (Clause 4.6.2, step 12).

Table 4F.6 — Form 4-1: results by duration (L/s; m³)

Duration (h)	2-yr pre	2-yr post, detained	10-yr pre	10-yr post, detained	100-yr post, uncontrolled	100-yr post, detained	100-yr storage used
1							
2							

Duration (h)	2-yr pre	2-yr post, detained	10-yr pre	10-yr post, detained	100-yr post, uncontrolled	100-yr post, detained	100-yr storage used
3							
6							
12							
24							
48							
72							

G. Facility, drawdown and outlet (Clauses 4.7, 4.8, 4.10; Clause 4.17, items 7 to 9)

Table 4F.7 — Form 4-1: facility, drawdown and outlet

Field	Entry
G1. Facility type (Clause 4.10) and storage provided (m^3)	
G2. Stage–storage and stage–discharge tables and plots; inflow and outflow hydrographs for the governing cases (attach)	
G3. 10-yr drawdown: time to release 90% of the maximum stored volume (h), with the low-flow orifice 50% blocked (Clauses 4.7.1, 4.8.3)	
G4. WQv release time (h), 24 to 48 h (Clause 4.7.2)	
G5. Tidal outfall: drawdown with the tidal tailwater, and any pumped drawdown (Clause 4.7.4)	
G6. Each outlet stage: type, size (mm), invert level (m)	
G7. Trash racks: clear bar spacing (mm) and open area (m^2) (Clause 4.8.3)	
G8. Anti-vortex devices (Clause 4.8.4); outlet barrel diameter and seepage control (Clause 4.8.5); isolation (Clause 4.8.6)	
G9. Proprietary devices: certified head–discharge curve and minimum clear opening (Clause 4.6.5.3)	

H. Spillway, freeboard and embankment (Clause 4.9; Clause 4.17, item 10)

Table 4F.8 — Form 4-1: spillway and freeboard

Field	Entry
H1. Hazard classification, with reasons (Clause 4.9.2)	
H2. 1% AEP peak inflow (L/s); 0.2% AEP peak inflow for high-hazard facilities (L/s)	
H3. 1% AEP maximum water level with the outlet working (m); spillway crest level (m) (Clause 4.9.3)	
H4. Spillway or overflow route: type, dimensions, maximum level with the outlet blocked (m)	
H5. Freeboard to the embankment crest or wall top (m), at least 0.3 m (Clause 4.9.1)	
H6. Spillway location and discharge path (Clause 4.9.4)	
H7. Embankment height (m); geotechnical report where more than 2 m (Clause 4.9.6)	

I. RWH accounting, groundwater and flotation (Clauses 4.11, 4.12; Clause 4.17, items 11 and 12)

Table 4F.9 — Form 4-1: RWH, groundwater and flotation

Field	Entry
I1. RWH storage provided (m³), assumed full at the start of every storm (Clause 4.11.2)	
I2. Combined RWH and detention tanks: zone levels and the detention orifice level (Clause 4.11.3); attach tank sections	
I3. Design groundwater level and its basis (Clause 4.12.1)	
I4. Lined or unlined storage; volume below the design groundwater level not counted (Clause 4.12.2)	
I5. Flotation factor of safety, at least 1.2 (Clause 4.12.3)	

J. Downstream, tailwater, safety and maintenance (Clause 4.17, items 13 and 14)

Table 4F.10 — Form 4-1: downstream, tailwater, safety and maintenance

Field	Entry
J1. Downstream and Tailwater Report (Form 5-1) or Simplified Downstream Statement (Form 5-2) reference	
J2. Tailwater used in routing, and flap gate where tailwater governs (Clause 4.6.6)	
J3. Public access, depths in the 10% and 1% AEP storms, slopes, fencing, grates, signs and depth markers (Clause 4.15)	
J4. Draft maintenance declaration reference (Chapter 10)	

K. Statement by the registered professional (Clause 10.6)

I state that this report has been prepared under Chapter 4 of the Chhattisgarh Stormwater Design Manual (Draft v1.0), that the results reported are reproduced by the models submitted, and that every departure from Chapter 4 is listed with its reason. The Professional Design Certificate (Form 10-3) is attached.

Table 4F.11 — Form 4-1: statement by the registered professional

Field	Entry
Name	
Qualification and registration number	
Empanelment number, where Clause 10.6.1 requires it	
Organisation	
Signature	
Date	

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

Form 4-2 — Tier B Site Sheet

Supports Clauses 4.4.6.3, 4.14.1 and 4.14.2 of Chapter 4.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. This sheet replaces items 6 and 7 of the Site Runoff Control Report (Form 4-1) for Tier B sites using Clause 4.14.2. It can be used only after Table 4.3 is published in Annex 4B; until then, Tier B sites use Clause 4.6 or, for sites of 1 ha or less, the fixed-coefficient route (Clauses 4.14.1 and 4.14.3).

A. Eligibility (Clause 4.14.1)

Table 4F.12 — Form 4-2: eligibility

Field	Entry
A1. Local body (Tier B)	
A2. Site area A (ha): 2 ha or less outside flood-sensitive catchments; 0.5 ha or less inside them	
A3. Catchment class: standard or flood-sensitive (Annex 4A)	

B. Table 4.3 values (Clause 4.14.2, step 1)

Table 4F.13 — Form 4-2: values read from Table 4.3

Field	Entry
B1. Rainfall zone (Annex 2A)	
B2. Hydrologic soil group	
B3. Post-development imperviousness (%)	
B4. PSD 2-yr (L/s/ha)	
B5. PSD 100-yr (L/s/ha), for the catchment class	
B6. SSR (m^3/ha), for the catchment class	

C. Storage and release (Clause 4.14.2, steps 2 and 3)

Table 4F.14 — Form 4-2: storage and release

Line	Quantity	Computation	Value
C1	RRV provided (m^3)	Clause 4.4	
C2	C_R	Annex 4B; 0 until published	
C3	V_{credit} (m^3)	C2 × C1 (Clause 4.4.6.3)	
C4	Required storage V (m^3)	B6 × A − C3	
C5	Storage provided (m^3)	From drawings	
C6	$Q_{PSD,2}$ (L/s)	B4 × A	

Line	Quantity	Computation	Value
C7	Q_PSD,100 (L/s)	B5 × A	

D. Outlet and other provisions (Clause 4.14.2, steps 4 and 5)

Table 4F.15 — Form 4-2: outlet and other provisions

Field	Entry
D1. Low-flow orifice: diameter (mm) from the Annex 4B outlet table, head (m), at least 50 mm	
D2. Upper control: type, size and level	
D3. Spillway or overflow (Clause 4.9)	
D4. RWH accounting (Clause 4.11)	
D5. Safety provisions (Clause 4.15)	

E. Statement

I state that the values above are taken from Annex 4B for the site's zone, soil group, imperviousness and catchment class, and that the storage and outlets shown on the drawings match them.

Table 4F.16 — Form 4-2: statement

Field	Entry
Name	
Qualification and registration number	
Signature	
Date	

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

Form 4-3 — Fixed-coefficient Route Sheet (Clauses 4.14.3 to 4.14.7)

Supports Clauses 4.14.3 to 4.14.7 of Chapter 4.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. This sheet replaces items 6 and 7 of the Site Runoff Control Report (Form 4-1) for sites using the fixed-coefficient route. Complete it for each point where runoff leaves the site (Clause 4.14.4). Figure 4.3 shows the method.

A. Eligibility (Clause 4.14.3)

Table 4F.17 — Form 4-3: eligibility

Check	Yes / No
A1. Site area 1 ha or less	
A2. The design does not count tank or waterbody storage (Clause 4.14.3(a))	
A3. No part of the site lies within a waterbody setback (Clause 4.14.3(b))	
A4. The site does not drain to a tank cascade whose storage is counted in a downstream design (Clause 4.14.3(c))	

B. Release caps (Clause 4.14.4)

Table 4F.18 — Form 4-3: release caps

Line	Quantity	Source or computation	Value
B1	Site area A (ha)	Survey	
B2	Area of each hydrologic soil group (ha)	Clause 3.7	
B3	C_10 and C_2, area-weighted	Table 4.3A	
B4	i_10 and i_2 (mm/h), 15-minute duration, with the climate factor	Form 2-1; Clause 2.8	
B5	Q_cap,10 (L/s)	2.78 × C_10 × i_10 × A	
B6	Q_cap,2 (L/s)	2.78 × C_2 × i_2 × A	
B7	Discharge of a 50 mm orifice at the corresponding stage (L/s), where a cap is smaller (Clause 4.5.5.1)	Orifice equation, Clause 4.6.5.2	

C. Volumetric coefficient (Clause 4.14.5)

Table 4F.19 — Form 4-3: volumetric coefficient C_v

Line	Quantity	Value
C1	Composite C of Table 3.2 (upper half of each range)	
C2	Cf of Table 3.3 for the 1% AEP	
C3	C_v for durations below 12 h (C1 × C2)	
C4	C_v for durations of 12 h or more, with C for every pervious surface not less than 0.50	

D. Storage (Clause 4.14.5)

For each duration: inflow = 10 × C_v × P(d) × A (m³); release = 1.8 × Q_cap × d (m³); difference = inflow − release. V_100 and V_2 are the largest differences; V is the larger of the two.

Table 4F.20 — Form 4-3: storage by duration

d (h)	P_100(d) (mm)	100-yr inflow (m³)	1.8 × Q_cap,10 × d (m³)	100-yr difference (m³)	P_2(d) (mm)	2-yr inflow (m³)	1.8 × Q_cap,2 × d (m³)	2-yr difference (m³)
1								
2								
3								
6								
12								
24								
48								
72								

Table 4F.21 — Form 4-3: storage required and provided

Line	Quantity	Value
D1	V_100 (m³) and governing duration	
D2	V_2 (m³) and governing duration	
D3	V = larger of D1 and D2 (m³)	
D4	Storage provided (m³)	

E. Outlet, retention and other checks (Clauses 4.14.3, 4.14.6, 4.14.7)

Table 4F.22 — Form 4-3: outlet, retention and other checks

Field	Entry
E1. Low-flow orifice: diameter (mm) and invert; discharge (L/s) with the storage holding V_2 full, not more than Q_cap,2	
E2. Upper orifice or weir: size and level; total release (L/s) at the top of V, not more than Q_cap,10	
E3. Retention volume of Clause 4.4 provided in addition to V (m³); no retention credit taken	
E4. Drawdown (Clause 4.7), trash racks (Clause 4.8), spillway (Clause 4.9): reference to Form 4-1 Sections G and H	
E5. Downstream and tailwater checks (Chapter 5) and water quality (Chapter 8): references	

F. Statement

I state that the site meets the eligibility conditions of Clause 4.14.3 and that the caps, storage and outlets above have been computed by Clauses 4.14.4 to 4.14.7.

Table 4F.23 — Form 4-3: statement

Field	Entry
Name	
Qualification and registration number	
Signature	
Date	

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

ANNEXES TO CHAPTER 5

Downstream and Tailwater Analysis

These annexes support Chapter 5. Annex 5C is a worked example of the outfall-closed check of Clause 5.10, for the illustrative facility of Annex 4C. Annexes 5A and 5B need tidal, surge, survey and river data that this draft does not yet contain; each lists what the chapter already sets, gives the intended layout and states the interim rule of Chapter 5 that applies until it is published. Forms 5-1 and 5-2 are the submittal forms required by Clause 5.14. They are draft forms and have no statutory status.

Annex 5A — Tidal datums (MSL, MHWS, HAT vs CD and Survey of India datum), surge allowances and 1% still-water levels by coastal segment

Supports Clauses 5.7, 5.9, 5.10, 5.11 and 5.12 of Chapter 5.

5A.1 Table 5A.1 shows how the values of this annex enter the design tailwater levels of Table 5.1. The sea-level allowance is set by Clause 5.7.4 and does not depend on this annex.

Table 5A.1 — Use of the Annex 5A values in the design tailwater (Table 5.1)

Design case	Tailwater	Annex 5A values used	Chapter value
Minor-system design	MHWS + SLA	MHWS	SLA ≥ +0.3 m (Clause 5.7.4)
Major-system and storage design	HAT + surge + SLA	HAT; 10% AEP surge (Clause 5.7.3(a))	SLA ≥ +0.3 m
Joint case (Clause 2.12(b))	1% AEP still-water level + SLA	1% AEP still-water level (Clause 5.7.3(b))	SLA ≥ +0.3 m
Datum conversion	Chart datum to project datum	Offset from chart datum to the Survey of India datum (Clause 5.7.2(b))	—

Not prepared for Chhattisgarh. Chhattisgarh has no coast, so no outfall in the State has a tidal tailwater, and this annex has no Chhattisgarh content (Clause 5.1.4). Its tables are kept, empty, so that the annex and table numbers match the other editions of this Manual. Rows F1 to F7 of Form 5-1 are entered as "not applicable", and the tailwater is derived under rows F8 to F10; for an outfall in the Hirakud backwater, row F10 records the levels of Clause 5.8.5(d), and for an outfall to a mine void, pit lake or ash pond, the consents and the survey of Clause 5.8.5(e).

Table 5A.2 — Tidal datums, surge and still-water levels by coastal segment (layout)

Coastal segment	Reference port and tide-table edition	MSL (m CD)	MHWS (m CD)	HAT (m CD)	CD to Survey of India datum (m)	10% AEP surge (m)	1% AEP still-water level (m CD)	Wave set-up (m), open-sea outfalls	Sources

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Annex 5B — Major drains, riverine outfall reaches, and river mouths subject to sand-bar closure; map

Supports Clauses 5.2, 5.3, 5.8 and 5.13 of Chapter 5.

5B.1 Table 5B.1 lists the water bodies that Chapter 5 already names. It is not complete: the annex will add the extents of each reach, the other rivers, canals and mouths of the State, and a map.

Table 5B.1 — Water bodies named in Chapter 5

Water body	Region	Listed as	Clause
Kharun at Raipur, past Mahadev Ghat and the Pathardihi gauge to the Seonath; the nalas of Raipur, among them the Goverdhan nala	Raipur	Riverine outfall reach and major drains; design river level not below the transferred 281.68 m (Pathardihi, 29 August 2020; 279.21 m of 12 July 1994 before it); no CWC danger or warning level found [TBC]; flood marks at Mahadev Ghat (September 2024 and 2026) to be surveyed; datum [TBC]	5.2, 5.8.1, 5.8.5
Seonath at Rajnandgaon, Durg and Bhilai below the Mongra barrage; the Shankar and Pulgaon nalas of Durg; the Hamp at Andhiyar Khore	Rajnandgaon, Durg, Bemetara	Riverine outfall reaches and major drains; no CWC level found on the Seonath near the towns [TBC]; modelled or surveyed level required; Hamp at Andhiyar Khore not below the transferred 260.2 m (12 August 2022); Mongra releases included	5.2, 5.8.1, 5.8.3, 5.8.5
Mongra barrage on the Seonath	Rajnandgaon	Barrage-pond outfalls; tailwater 337.65 m (FRL 337.150 m + 0.5 m), or the river level at the barrage if higher, until WRD confirms [TBC]	5.8.3, 5.8.5
Arpa at Bilaspur below the Arpa Bhaisajhar barrage; the nalas of Bilaspur	Bilaspur	Riverine outfall reach and major drains; no CWC level found [TBC]; modelled or surveyed level required (flood marks of July 2026); Bhaisajhar releases included	5.2, 5.8.1, 5.8.3, 5.8.5
Arpa Bhaisajhar barrage; Kharang (Khutaghat) reservoir; Kopra Jalashay	Bilaspur	Barrage-pond and reservoir outfalls; tailwaters 302.50 m (Bhaisajhar, FRL 302.000 m + 0.5 m) and 294.61 m (Kharang, FRL 294.110 m + 0.5 m) until WRD confirms [TBC]; Kopra Jalashay a Ramsar site (8 August 2025), no level on a stated datum found [TBC], WRD's FTL or a surveyed crest required	5.8.3, 5.8.5
Maniari at Maniyari below the Maniyari dam	Mungeli	Riverine outfall reach; design river level not below the transferred 236.23 m (13 August 2022); reservoir tailwater 358.64 m (FRL 358.140 m + 0.5 m) [TBC]	5.8.1, 5.8.3, 5.8.5
Hasdeo at Korba below the Minimata Bango dam and the Hasdeo barrage at Darri; the nalas of Korba	Korba	Riverine outfall reach and major drains; design river level not below the transferred 313.58 m (Katghora, 20 July 2026) and 303.5 m (Bangodam, 17 August 2026); no danger level found [TBC]; Bango and Darri releases included (about 30,000 and 20,000 cusec, 26 August 2024, as reported)	5.2, 5.8.1, 5.8.3, 5.8.5

Water body	Region	Listed as	Clause
Minimata Bango reservoir; Hasdeo barrage at Darri	Korba	Reservoir and barrage-pond outfalls; tailwater 360.16 m (Bango, FRL 359.660 m + 0.5 m) until WRD confirms [TBC]; no level found for Darri [TBC], WRD's pond level or a surveyed crest required	5.8.3, 5.8.5
Mahanadi below the Gangrel dam at Dhamtari and Rajim, past the barrages from Samoda to Kalma, at Kalma and Surajgarh; the Pairi at Malgaon	Dhamtari, Gariaband, Raipur, Janjgir-Champa, Raigarh	Riverine outfall reaches; design river level not below the transferred 202.43 m (Kalma) and 201.06 m (Surajgarh) (both 29 August 2020) and 329.33 m (Malgaon, 30 July 2026); no danger level found [TBC]; no level found at Rajim, Seorinarayan or Basantpur [TBC]; lock of at least 120 hours below Rajim; Gangrel and barrage releases included	5.8.1, 5.8.3, 5.8.5
Ravishankar Sagar (Gangrel) reservoir; barrages on the Mahanadi (Samoda, Seorinarayan, Basantpur, Mirauni, Saradiha, Kalma)	Dhamtari; Mahanadi reach	Reservoir and barrage-pond outfalls; tailwater 349.20 m (Gangrel, FRL 348.700 m + 0.5 m) until WRD confirms [TBC]; barrage crest and pond levels not found [TBC]	5.8.3, 5.8.5
Mand at Kurubhata	Raigarh	Riverine outfall reach; design river level not below the transferred 220.4 m (28 August 2020); lock of at least 120 hours	5.8.1, 5.8.5
Kelo at Raigarh below the Kelo dam	Raigarh	Riverine outfall reach; no CWC level found [TBC]; modelled or surveyed level required; reservoir tailwater 233.50 m (FRL 233.000 m + 0.5 m) [TBC]; Kelo dam releases included	5.8.1, 5.8.3, 5.8.5
Backwater of the Hirakud reservoir (Odisha)	Raigarh, Sarangarh-Bilaigarh	Reservoir backwater outfalls; tailwater the higher of the river level and the reservoir level (MWL, FRL + surcharge or FRL + 0.5 m) carried up the backwater; Hirakud's levels to be obtained through WRD from Odisha [TBC]; not below 628.5 ft (about 191.6 m, as reported) until they are; outfall below it closed for the whole storm	5.8.5
Indravati at Jagdalpur and Chindnar, below the dams in Odisha; the Narangi at Sonarpal	Bastar, Dantewada	Riverine outfall reaches; design river level not below the transferred 544.891 m (Jagdalpur, 26 September 2026; 544.68 m of 9 July 1973 before it), 340.49 m (Chindnar) and 543.43 m (Sonarpal) (both 26 September 2026); danger 540.80 m, warning 539.49 m at Jagdalpur (CWC); local-gauge readings not used for design [TBC]; Odisha dam releases to be obtained through WRD [TBC]	5.8.1, 5.8.3, 5.8.5
Sabari at Talnar and Odinaguda (Konta)	Sukma	Riverine outfall reaches; design river level not below the transferred 213.81 m (Talnar, 26 September 2026) and 71.64 m (Odinaguda, 10 September 2024); Godavari backwater at Konta (2022, as reported)	5.8.1, 5.8.5
Talabs of Raipur (among them Budha Talab, Telibandha, Kankali, Maharajbandh, Karbala and Narhaiya) and of the other towns	Raipur and other towns	Talab outfalls; no level found [TBC]; spill level plus head; custodian's consent (custodian [TBC]); State Wetland Authority's 50 m line where it applies; tanks vested under s.251 of the Land Revenue Code to the Collector	5.8.5
Lakes and reservoirs of Nava Raipur Atal Nagar (among them Sendh, Jhanjh and Khandawa) and the four major streams that link them	Raipur	Lake outfalls and major drains; no level found [TBC]; NRDA's consent; spill level plus head; no storage credited below the spill level without NRDA's records; Jhanjh a deemed wetland for the State Wetland Authority (as reported)	5.2, 5.8.5
Mine voids, pit lakes and working mines of the Korba, Mand–Raigarh and Surguja coalfields; ash ponds and ash dykes of the power plants of Korba and Raigarh	Korba, Raigarh, Surguja	Not a design outfall without the written consent of the lessee and DGMS, or of the ash-pond operator, and a survey of level, spill and spill path; no drainage into a working mine or an ash pond; ash-dyke breach path mapped as a hazard	5.8.5

Water body	Region	Listed as	Clause
Irrigation canals of WRD; drains of Municipal Corporations and other urban local bodies, RDA, NRDA, the Chhattisgarh Housing Board and the Bhilai Steel Plant township 1.0 m wide or more; garland drains of mines; drains of ash ponds	Chhattisgarh	Major drains; owner's consent, design water level and flap gate	5.2, 5.8.5
Gangrel, Minimata Bango, Kharang, Maniyari and Kelo dams; the Hasdeo barrage at Darri, the Mongra and Arpa Bhaisajhar barrages and the barrages on the Mahanadi; the dams on the Indravati in Odisha; Hirakud (Odisha)	Mahanadi, Hasdeo, Arpa, Seonath, Kelo and Maniari; Indravati	Releases and gate operation to be included in river levels (operating rules and release records [TBC])	5.8.3, 5.8.5
River mouths subject to sand-bar closure	None in Chhattisgarh	Not applicable (Clause 5.1.4)	5.13.1

To be prepared. This annex will list and map (a) the major drains of each basin plan, (b) the riverine outfall reaches where backwater shall be modelled, with their upstream and downstream limits and the source of their design flood levels, and (c) the river mouths and creek inlets subject to seasonal sand-bar closure, with the most recent pre-monsoon bar crest levels and any WRD mouth-maintenance programme. It needs WRD river and canal records, design and recorded flood levels, reservoir operating rules, mouth-condition, dredging and bar-cutting records and pre-monsoon bar surveys (WRD), the drain inventories of RMC and other local bodies, basin stormwater master plans (Chapter 11), and base mapping from the Survey of India and NRSC. Until it is published: every river, WRD canal, the Kharun at Raipur and the nalas of the city that run to it, among them the Goverdhan nala at Mahadev Ghat, the streams and channels that link the lakes and reservoirs of Nava Raipur Atal Nagar, the Seonath and the Shankar and Pulgaon nalas of Durg, the nalas of Bhilai and of its steel-plant township, the Arpa at Bilaspur and the nalas that run to it, the Hasdeo and the nalas of Korba, the Kelo at Raigarh, the Indravati at Jagdalpur, any drain or canal of WRD, any drain built or held by a Municipal Corporation, a Nagar Palika Parishad, a Nagar Panchayat, RDA, NRDA, the Chhattisgarh Housing Board or the Bhilai Steel Plant with an internal width of 1.0 m or more, the garland drains and diversion channels of a mine, the drains of an ash pond, any irrigation canal that receives storm water and every tidal creek is a major drain (Clause 5.2); the water bodies of Table 5B.1 are riverine outfall reaches (Clause 5.8.1); and the mouths of the coastal States' rivers (there is none in Chhattisgarh) are assumed closed at the start of the SW monsoon unless WRD certifies otherwise (Clause 5.13.2).

Table 5B.2 — Attributes of the Annex 5B layers (layout)

Field	Content	Unit or format
feature_id	Unique identifier	text
name	Name of the drain, river, canal, creek or mouth	text
category	Major drain; riverine outfall reach; mouth subject to sand-bar closure	text
clause	Clause of Chapter 5 under which it is listed	text
reach_limits	Upstream and downstream limits of the reach	text
design_flood_level_source	WRD design or highest recorded flood level, or hydraulic model (Clause 5.8.2)	text
reservoir_release	Upstream reservoir or tank and its operating rule, where any (Clause 5.8.3)	text
bar_crest_level	Most recent pre-monsoon bar crest level, for mouths	m (Survey of India datum)

Field	Content	Unit or format
mouth_programme	WRD mouth-maintenance programme and certification (Clause 5.13.2)	text
source	Source agency and date	text
geometry	Line (drains and reaches) or point (mouths)	line / point

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Annex 5C — Worked example: outfall-closed check for a site on a nala draining to the Kharun in Raipur

Supports Clauses 5.9, 5.10, 5.11 and 5.12 of Chapter 5 and Clause 4.6.6 of Chapter 4.

Illustrative — not design values. This example applies the outfall-closed check of Clause 5.10 to the revised detention facility of Annex 4C. The site, its levels and its receiving drain are invented to show the method. No tidal level is used: the tide enters only through the illustrative statements in Table 5C.1 about whether the receiving drain is above or below the site outlet.

Inputs

5C.1 The site is the 2.0 ha flood-sensitive site of Annex 4C. It discharges through its own outlet chamber and a flap gate to a public storm drain that joins a nala running to the Kharun in Raipur, whose level rises when the nala runs full and when the Kharun is high, as it was at Pathardihi in August 2020 and at Mahadev Ghat in September 2026, so Clause 5.10.1 applies to the facility. To show the method, this example uses the two 6-hour lock windows of Clause 5.10.3, as the coastal editions of this Manual do. In Chhattisgarh a river-locked outfall has a single lock of at least 72 hours, or 120 hours on the Mahanadi, and an outfall below the design level of a reservoir, barrage pond, talab or the Hirakud backwater is closed for the whole storm, under Clause 5.8.5, so in each of those cases the facility must store or pump the whole storm volume (Clause 5.9.4); that needs more storage, or pumping, than this example shows. The routing, the volume balance of Clause 5.10.4 and the assessment are the same.

Table 5C.1 — Inputs (Illustrative — not design values)

Item	Value	Basis
Facility	Revised facility of Annex 4C: 2,500 m² plan area, vertical walls; 170 mm orifice at the floor; 460 mm orifice, invert 0.53 m	Annex 4C, Table 4C.14
Storm	1% AEP, 48 h, peak day last, AMC III; 72 h also run	Clause 5.10.2
Inflow	Post-development 1% AEP hydrographs of Annex 4C (48 h: 8,420 m³; 72 h: 9,468 m³)	Annex 4C
Receiving drain levels at the connection, J1, J2 and J3	Below the site outlet invert (from the drain owner's model)	Illustrative input
Receiving drain during each lock window	Backs up above the site outlet invert, so the site cannot discharge	Illustrative input; Clause 5.10.3(b)
Flap gate on the site outlet	Set 0.15 m below the orifice inverts in the outlet chamber, so that outside the lock windows its opening head and head loss do not affect the orifice control	Illustrative input; to be confirmed from the certified curve (Clause 5.11.2)
Lock windows	Two of 6 h each, starts 12.4 h apart (semi-diurnal tide)	Clause 5.10.3(a)
Pumps	None	—

5C.2 **Cases of Table 5.2.** Because the receiving drain stays below the site outlet in J1, J2 and J3 (Table 5C.1), those cases give the free-outfall results of Annex 4C: in J2 the 1% AEP maximum water level is **1.17 m** (2,913 m³), and in J3 (10% AEP, 48 h) it is 0.77 m. R1 and R2 do not apply, because the site does not drain to a river. J4, the outfall-closed check, is worked below.

Lock windows (Clause 5.10.3)

5C.3 The peak 1-hour block of the final 24 hours of the 48-hour storm is 35:35–36:35 h, with 98.2 mm (the 1-hour depth of Table 4C.2). Two runs are made:

- **Run A:** the first window is centred on that block: 33:05–39:05 h; the second starts 12.4 h later: 45:29–51:29 h.
- **Run B:** the first window starts at that block: 35:35–41:35 h; the second: 47:59–53:59 h.

For the 72-hour storm, every time is 24 h later.

5C.4 Gravity discharge is zero inside each window. Outside the windows the outlet discharges through the orifices of Table 4C.10. The routing is the same level-pool routing as in Annex 4C.

Results

Table 5C.2 — Outfall-closed check results (Illustrative — not design values)

Storm	Run	Lock windows (h:min)	Maximum stage (m)	Maximum storage (m³)	Time of maximum (h:min)	Largest release after a window (L/s)
1% AEP, 48 h	A	33:05–39:05; 45:29–51:29	2.01	5,019	39:05	566
1% AEP, 48 h	B	35:35–41:35; 47:59–53:59	**2.10**	**5,238**	41:35	584
1% AEP, 72 h	A	57:05–63:05; 69:29–75:29	2.00	5,012	63:05	565
1% AEP, 72 h	B	59:35–65:35; 71:59–77:59	2.09	5,233	65:35	584

Run B of the 48-hour storm governs. The first window closes the outlet just as the storm peak arrives, so the whole peak is stored. The second window arrives when the facility has drained to about 0.7 m, below the Stage 2 orifice, and adds little.

5C.5 **Volume balance (Clause 5.10.4).** Table 5C.3 gives the balance for Run B. The outflow in each period is found by continuity: outflow = storage at start + inflow − storage at end.

Table 5C.3 — Volume balance, 1% AEP 48-hour storm, Run B (Illustrative — not design values)

Period (h:min)	Inflow (m³)	Gravity outflow (m³)	Pumped (m³)	Storage at start (m³)	Storage at end (m³)
0:00–35:35, before window 1	3,817	2,213	0	0	1,604
35:35–41:35, window 1	3,634	0	0	1,604	5,238
41:35–47:59, between windows	941	4,423	0	5,238	1,756
47:59–53:59, window 2	28	0	0	1,756	1,784
After 53:59	0	1,784	0	1,784	0
Total	8,420	8,420	0	—	—

Check for window 1: V_inflow = 3,634 m³ ≤ V_storage available + V_pumped + V_gravity = (5,238 − 1,604) + 0 + 0 = 3,634 m³. The facility therefore needs **5,238 m³** of storage, which the routing confirms.

Table 5C.4 — Stage hydrograph, 1% AEP 48-hour storm, Run B (Illustrative — not design values)

Time (h)	Inflow (L/s)	Outlet	Outflow (L/s)	Stage (m)	Storage (m³)
24	20	Open	18	0.18	445
28	39	Open	24	0.25	621
32	59	Open	32	0.36	906
34	85	Open	37	0.47	1,166
36	286	Closed	0	0.78	1,944
38	106	Closed	0	1.73	4,321
40	66	Closed	0	1.96	4,904
42	51	Open	529	1.79	4,472
44	42	Open	204	0.87	2,181
46	37	Open	49	0.74	1,854
48	33	Open	47	0.70	1,756
50	0	Closed	0	0.71	1,784
54	0	Closed	0	0.71	1,784
56	0	Open	43	0.58	1,459
60	0	Open	32	0.37	921
66	0	Open	16	0.16	397
72	0	Open	7	0.06	162

Assessment

5C.6 **Clause 5.10.4(a).** The facility of Annex 4C reaches 1.17 m in J2, and its spillway crest is at 1.35 m (Annex 4C). In the outfall-closed check the stage reaches 2.10 m, so the spillway would operate. **The facility of Annex 4C fails the check.**

5C.7 **Remedy.** Pumping (Clause 5.12) does not help here: during the lock windows the receiving drain is itself closed, and pumping into it would raise its level, which Clause 5.12.3 does not permit. The remedy is more storage. Keeping the plan area of 2,500 m² and the outlets of Annex 4C:

- the facility must hold 5,238 m³, to a stage of **2.10 m**;
- the spillway crest is set above that stage, with the 0.15 m clearance recommended in Clause 4.9.3: **2.25 m**;
- with the 4.0 m spillway of Annex 4C (head 0.30 m), the maximum level with the spillway flowing is 2.55 m, and with 0.3 m freeboard the top of the walls is at least **2.85 m** above the floor.

The outfall-closed check therefore governs the storage: 5,238 m³ against 2,913 m³ from the release-rate checks, about 80% more. A larger plan area would make the facility shallower; the outlet, caps and drawdown would then have to be re-checked, because the stage–discharge relationship changes.

5C.8 **Release after the lock.** When the first window ends, the stored water is released under a head of about 2 m, at up to 584 L/s. This is above the 100-yr cap of 356 L/s. Clause 5.10.4 allows this, because the release reaches a drain that has itself just reopened, and requires it to be included in the J4 run of the downstream analysis (Clauses 5.4 and 5.9) and reported on Form 5-1, Section H.

5C.9 **Drawdown after the storm (Clause 5.10.5).** The J1 tidal curve stays below the site outlet (Table 5C.1), so the drawdown of Annex 4C applies: 45.3 h for 90% of the 10-yr storage with the low-flow orifice 50% blocked, within the 48-hour limit of Clause 4.7. In Run B, 90% of the maximum J4 storage is released 22.6 h after the maximum.

5C.10 **Other requirements.**

- The flap gate on the site outlet is inspected and operated before 10 June each year and after every event in which it closed against a storm flow (Clause 5.11.5).
- The deeper facility is checked for flotation with groundwater at the design groundwater level; in the kanhar and dorsa paddy lowlands, the talab and river floodplain margins and the Hirakud backwater without piezometer readings that level is existing ground level (Clauses 4.12.1 and 4.12.3).
- The fence and signs of Annex 4C apply to the deeper facility (Clause 4.15).

Table 5C.5 — Summary (Illustrative — not design values)

Item	Value
Governing case	J4, 1% AEP 48-hour storm, Run B (first window starting at the peak 1-hour block)
Storage required	5,238 m³ (Annex 4C release-rate design: 2,913 m³)
Maximum stage	2.10 m, with 2,500 m² plan area
Spillway crest	2.25 m
Top of walls	≥ 2.85 m above the floor
Largest release after a lock window	584 L/s, to be included in the downstream J4 run
Drawdown	Annex 4C result applies: 45.3 h with the low-flow orifice 50% blocked

Form 5-1 — Downstream and Tailwater Report

Supports Clauses 5.3 to 5.14 of Chapter 5.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. One report for each project. Sites of less than 0.5 ha that meet Clause 5.3.4 may submit Form 5-2 instead, where the Authority accepts it. Attach the plans, model files and hydrographs referred to.

A. Project

Table 5F.1 — Form 5-1: project

Field	Entry
A1. Project name, applicant and location	
A2. Project type: site under Chapter 4; drainage project under Chapter 6; regional detention or tank restoration (Clause 5.1.2)	
A3. Site or project area (ha) and outfalls	
A4. Site Runoff Control Report reference (Form 4-1), where applicable	

B. Extent of the analysis (Clause 5.3; Clause 5.14, item 1)

Table 5F.2 — Form 5-1: analysis points

Point	Location	Type (Clause 5.3.3 (a) to (e))	Contributing catchment (ha)	Project area as % of it

Table 5F.3 — Form 5-1: end point

Field	Entry
B1. End point and reason: 10% point, tank or talab, or major drain or the sea (Clause 5.3.1 (a) to (c))	
B2. Where the end point is a tank or talab: change in maximum water level (m) and in surplus-weir discharge (m³/s); continuation below the weir if the level rises (Clause 5.3.2)	
B3. Plan of the analysis extent, showing the site, each point with its contributing area and percentage, and the end point (attach)	

C. Model (Clause 5.4.4; Clause 5.14, item 2)

Table 5F.4 — Form 5-1: model

Field	Entry
C1. Software and version; hydrologic and hydraulic methods (Clause 5.4.4)	
C2. Network schematic, with the survey sources for drains and crossings (attach)	
C3. Land use and CN of off-site areas (present land use)	

Field	Entry
C4. Upstream detention included, with evidence that it is built, maintained and documented	
C5. Calibration or validation evidence, where available	
C6. Scenarios run: existing, with project, and future where the Authority requires it (Clause 5.4.1)	
C7. Storms run: AEPs and durations, AMC and areal reduction (Clause 5.4.3)	

D. Results at each analysis point (Clause 5.4.2; Clause 5.14, item 3)

Complete one row for each analysis point, AEP and duration. E = existing; P = with project.

Table 5F.5 — Form 5-1: results by analysis point

Point	AEP	Duration (h)	Peak flow E / P (m^3/s)	Time to peak E / P (h)	Maximum water level E / P (m)	Peak velocity E / P (m/s)	Duration of inundation E / P (h)

E. Acceptance (Clause 5.5; Clause 5.14, item 4)

Table 5F.6 — Form 5-1: acceptance criteria

Criterion	Met at every point?	Where not met, the remedy adopted (Clause 5.5.3)
E1. 1% AEP water level rise ≤ 10 mm at buildings, lowest openings and carriageways (Clause 5.5.1(a))		
E2. No building newly flooded; no building or road newly failing the Chapter 6 limits (Clause 5.5.1(b))		
E3. Peak flow not above existing at the minor-system AEP of each element or at 1% AEP (Clause 5.5.1(c))		
E4. Velocity in lined channels within the lining limit, 3.0 m/s maximum (Clause 5.5.1(d))		
E5. Velocity in unlined channels, 50% and 10% AEP, not increased by more than 10% (Clause 5.5.1(e))		
E6. Change in duration of road closure on arterial or emergency routes (Clause 5.5.2)		
E7. Timing exception requested (Clause 5.5.4)? Give the conditions met		

F. Tailwater derivation (Clauses 5.6 to 5.8; Clause 5.14, item 5)

Table 5F.7 — Form 5-1: tailwater derivation

Field	Entry
F1. Tide-table edition and port, or port authority source (Clause 5.7.2(a))	

Field	Entry
F2. MHWS, HAT and MSL (m above chart datum)	
F3. Connection from chart datum to the Survey of India benchmark and the project datum: offset (m) and source (Clause 5.7.2(b))	
F4. Estuary, creek or canal outfall: levels used and any evidence for a departure from open-coast levels (Clause 5.7.2(c))	
F5. 10% AEP surge and 1% AEP still-water level: values, source, method and record length (Clause 5.7.3)	
F6. Wave set-up, for outfalls facing the open sea (Clause 5.7.3(d))	
F7. SLA (m), at least +0.3 m; design life; sensitivity run at twice the SLA for design lives over 50 years (Clause 5.7.4)	
F8. Non-tidal tailwater: receiving-system level and source (Clauses 5.6.2, 5.6.3)	
F9. River levels and sources; reservoir or tank releases included (Clauses 5.8.2, 5.8.3)	
F10. River-locked outfalls, reservoir, barrage and talab outfalls, rivers below dams and barrages, the Hirakud backwater, mine voids, ash ponds and canals or drains of other bodies: operator of each gate and pumping station identified, gate sills, bank or embankment crest, installed capacity, operating levels and gate-closing level obtained, with every level tied to the datum of the nearest CWC or WRD gauge; 1% AEP river level at the outfall from a published study or a model, with the dam and barrage releases, and not below the highest recorded level at the nearest CWC gauge transferred to the outfall (544.891 m at Jagdalpur, 281.68 m at Pathardihi on the Kharun, 202.43 m at Kalma, and the other levels of Clause 5.8.5(a)), with the profile used, or, where no gauge level exists (the Seonath near Durg and Bhilai, the Arpa at Bilaspur, the Kelo at Raigarh, the nalas of the towns), a modelled or surveyed flood level and its source; no local gauge reading (Indravati at Jagdalpur) used as a design level; 10% AEP river level, not below the CWC danger level transferred where one exists (540.80 m at Jagdalpur); drain behind the gate routed with its whole catchment and present storage, pumping confirmed available (permanent pumps only, not more than installed capacity less the largest pump), pumps operable at the design river level; single river lock of not less than 72 hours, or 120 hours on the Mahanadi from Rajim to the Hirakud backwater and on the Mand below Kurubhata, or the duration of the largest recorded release below a dam or barrage if longer, and its source; gate-failure and breach checks; for a reservoir, barrage-pond or talab outfall, custodian's consent, State Wetland Authority permission where required, MWL or FTL plus design-flood surcharge (FTL + 0.5 m where neither is published, as for Gangrel 349.20 m, Bango 360.16 m, Arpa Bhaisajhar 302.50 m and Mongra 337.65 m; spill level plus head for a talab or lake; the river level at a barrage if higher), outfall below it treated as closed for the whole storm, waterbody starting at FTL or spill level, no gate operation credited without the custodian's written rule; for an outfall below a dam or barrage, the design storm with the largest recorded release, placed before, during and after the storm; for an outfall in the Hirakud backwater, the higher of the river level and the reservoir level obtained through WRD from Odisha, and not below 628.5 ft (about 191.6 m) until it is; for a mine void, pit lake or ash pond, the written consent of the lessee and DGMS or of the ash-pond operator, the surveyed level, spill level and spill path, the stability statement, a statement that no part of the site drains to a working mine or an ash pond, and the mapped path of an ash-dyke breach; WRD consent and the building-rule, wetland, revenue-record and court lines checked; owner's consent, permitted rate, design water level or computed channel level, and flap gate (Clause 5.8.5)	

G. Joint cases (Clause 5.9; Clause 5.14, item 6)

Table 5F.8 — Form 5-1: joint-case results

Case	Rainfall	Tailwater used (m, project datum)	Applies?	Governing result (storage, level, pipe size or pump capacity)
J1				
J2				
J3				
J4				
R1				
R2				

H. Outfall-closed check (Clause 5.10; Clause 5.14, item 7)

Table 5F.9 — Form 5-1: outfall-closed check

Field	Entry
H1. Reason the check applies (Clause 5.10.1)	
H2. Storm: AEP, 48 h and 72 h, AMC III (Clause 5.10.2)	
H3. Lock duration and basis: default of two 6-hour windows, or site study (Clause 5.10.6); extension for sand-bar closure (Clause 5.13.5)	
H4. Lock windows of the two runs (start and end times) (Clause 5.10.3(d))	
H5. Maximum stage in each facility, against its 1% AEP maximum water level (Clause 5.10.4(a))	
H6. Maximum stage in the minor and major systems, against the Chapter 6 limits (Clause 5.10.4(b))	
H7. Property connections: surcharge level, or non-return valves fitted (Clause 5.10.4(c))	
H8. Stage hydrographs (attach)	
H9. Drawdown after the storm under the J1 tidal curve, and pumping where needed (Clause 5.10.5)	

Table 5F.10 — Form 5-1: volume balance for each lock window (Clause 5.10.4)

Period	Inflow (m^3)	Gravity outflow (m^3)	Pumped (m^3)	Storage at start (m^3)	Storage at end (m^3)
Before window 1					
Window 1					
Between windows					
Window 2					
After window 2					

I. Gates and valves (Clause 5.11; Clause 5.14, item 8)

Table 5F.11 — Form 5-1: schedule of flap gates, tide gates and non-return valves

Location	Type	Size (mm)	Invert (m, project datum)	Certified head–discharge data (reference)	Stop-log or penstock (outfalls > 600 mm)

J. Pumped outfalls (Clause 5.12; Clause 5.14, item 9)

Table 5F.12 — Form 5-1: pumped outfalls

Field	Entry
J1. Reason pumping is needed (Clause 5.12.1)	
J2. Duty capacity (m^3/s) at the static head for the J2 tailwater; standby capacity (Clause 5.12.2)	
J3. Effect on the receiving water level (Clause 5.12.3)	

Field	Entry
J4. Power supply: two independent supplies, or standby generation with 72 h of fuel; automatic changeover (Clause 5.12.4)	
J5. Level of electrical panels, controls, generators and fuel, against the higher of the 1% AEP flood level + 0.5 m and HAT + surge + SLA + 0.5 m (Clause 5.12.5)	
J6. Discharge arrangement (Clause 5.12.6); controls, alarms and logging (Clause 5.12.7); screens (Clause 5.12.8)	

K. Sand bar (Clause 5.13; Clause 5.14, item 10)

Table 5F.13 — Form 5-1: sand-bar assumption

Field	Entry
K1. River mouth or creek inlet concerned	
K2. Mouth assumed closed, or open with WRD certification attached (Clause 5.13.2)	
K3. Bar crest level from the most recent pre-monsoon survey (m) and the closed-mouth tailwater (Clause 5.13.3)	
K4. Extension of the lock duration, where required (Clause 5.13.5)	

L. Inspection and maintenance (Clause 5.14, item 11)

Table 5F.14 — Form 5-1: inspection and maintenance

Field	Entry
L1. Gate and valve inspection before 10 June and after closure events (Clause 5.11.5)	
L2. Pump test runs before 10 June (Clause 5.12.9)	
L3. Other requirements for the maintenance plan (Chapter 10)	

M. Statement by the registered professional (Clause 10.6)

I state that this report has been prepared under Chapter 5 of the Chhattisgarh Stormwater Design Manual (Draft v1.0), that the results reported are reproduced by the models submitted, and that every departure from Chapter 5 is listed with its reason. The Professional Design Certificate (Form 10-3) is attached.

Table 5F.15 — Form 5-1: statement by the registered professional

Field	Entry
Name	
Qualification and registration number	
Empanelment number, where Clause 10.6.1 requires it	
Organisation	
Signature	
Date	

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

Form 5-2 — Simplified Downstream Statement (small sites)

Supports Clause 5.3.4 of Chapter 5.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. For sites of less than 0.5 ha. The Authority may accept this statement in place of the full analysis only where every answer in Section B is "Yes". Otherwise submit Form 5-1.

A. Site

Table 5F.16 — Form 5-2: site

Field	Entry
A1. Project name, applicant and location	
A2. Site area (ha), less than 0.5 ha	
A3. Receiving public drain: name or reference, type and size	
A4. Site Runoff Control Report reference (Form 4-1)	

B. Conditions (Clause 5.3.4)

Table 5F.17 — Form 5-2: conditions

Condition	Yes / No	Evidence
B1. The receiving public drain has documented capacity for its minor-system design flow (Chapter 6)		
B2. No flood-prone location is recorded within 500 m downstream of the site outfall. Records checked: local body, RMC, CGSDMA, WRD (Clause 5.3.3(d))		
B3. The site does not drain to a tidal or backwater-affected outfall		

C. Route plan

Table 5F.18 — Form 5-2: attachments

Attachment	Attached?
C1. Plan of the drainage route from the site outfall for 500 m downstream, with the receiving drain and any recorded flood-prone locations	
C2. Copy or reference of the drain capacity record	

D. Statement

I state that the site meets the conditions of Clause 5.3.4 as shown above, and that the records listed have been checked.

Table 5F.19 — Form 5-2: statement

Field	Entry
Name	
Qualification and registration number	
Signature	
Date	

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

ANNEXES TO CHAPTER 6

Minor and Major Drainage Systems

These annexes support Chapter 6. Annex 6A (standard inlets and their capacity curves) and Annex 6B (sediment yield) need test and survey data that are not yet available; each states what it will contain and the interim rule to use until it is published. Annex 6C gives standard drawings that carry only the dimensions Chapter 6 and Chapter 9 already set; every other dimension is marked "to be confirmed in design". Form 6-1 is a draft form: it lists what Clause 6.15 requires and does not add to it.

Annex 6A — Standard inlet types for Chhattisgarh with capacity curves (on grade and sag)

Supports Clauses 6.6.1 to 6.6.7 of Chapter 6 and Clause 9.11 of Chapter 9.

To be prepared. This annex will contain a short list of standard inlet types for Chhattisgarh streets (kerb-opening, grate and combination inlets, including the road-level wall opening into a covered drain) and, for each type, capacity curves on grade and in a sag. It needs a fixed set of standard inlet geometries agreed with the road-owning agencies, and rating tests of those geometries (laboratory or full-scale), or HEC-22 calculations for each geometry checked against tests (data and likely sources: Public Works Department (PWD), NHAI and Raipur Municipal Corporation standard details, NIT Raipur, IIT Bhilai or other hydraulic laboratories, manufacturers' tested ratings). Until it is published, compute the clean capacity of the inlet actually proposed by the methods of FHWA HEC-22 (4th edition, 2024) or from the manufacturer's tested rating for that exact geometry (Clause 6.6.2), reduce it by the clogging factors of Table 6.5 (Clause 6.6.3), and meet the interim requirements of Table 6A.1.

Table 6A.1 — Interim requirements for inlets until Annex 6A is published (from Chapters 6 and 9)

Item	Requirement	Clause
Inlet type on public streets	Kerb-opening or combination inlets, unless the Approving Authority accepts grate-only inlets for a location	6.6.1(a)
Inlet level	Openings at road level; gutter depression where practicable	6.6.1(b)
Clean capacity	FHWA HEC-22 (4th ed., 2024) or the manufacturer's tested rating for the exact geometry; CPHEEO §5.6.2 equations as a cross-check only	6.6.2
Clogging, on grade	Grate 50%; kerb opening 10%	6.6.3, Table 6.5
Clogging, in a sag	Grate 50%; kerb opening 20%	6.6.3, Table 6.5
Spacing	By sequential calculation from the top of each gutter run; not more than 30 m on kerbed roads	6.6.4
Kerb opening size	Clear height at least 100 mm; length at least 1.0 m	9.11.4
Grate pattern	Openings parallel to the direction of travel not wider than 25 mm	9.11.3(a)

Item	Requirement	Clause
Load class	D400 in carriageways and medians; C250 within 0.5 m of the kerb; B125 in footpaths and cycle tracks (or the equivalent IS class)	9.11.2
Connection pipe	At least 300 mm diameter; gradient giving at least 0.9 m/s at the inlet's design flow	6.6.6
Silt sump	At least 300 mm deep below the outlet pipe invert, cleanable from the surface	6.6.7, 9.11.6

The intended layout of the annex is given in Tables 6A.2 to 6A.4.

Table 6A.2 — Register of standard inlet types (layout)

Type code	Description	Grate size or kerb opening (mm)	Gutter depression (mm)	Load class	Standard drawing
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____

Table 6A.3 — Capacity on grade (layout; one table per inlet type)

Cross slope Sx (m/m)	Longitudinal slope SL (m/m)	Gutter flow Q (L/s)	Spread T (m)	Clean intercepted flow (L/s)	Design intercepted flow after clogging (L/s)	Bypass flow (L/s)
_____	_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____	_____

Table 6A.4 — Capacity in a sag (layout; one table per inlet type)

Ponded depth at the inlet (mm)	Clean capacity (L/s)	Clogging factor (Table 6.5)	Design capacity (L/s)	Flow regime (weir, orifice or transition)
_____	_____	_____	_____	_____
_____	_____	_____	_____	_____

Annex 6B — Sediment yield for silt-trap sizing and calibration of the sediment allowance

Supports Clauses 6.7.4, 6.7.8, 6.13.3 and 6.13.10 of Chapter 6.

To be prepared. This annex will contain sediment yields for urban drains in Chhattisgarh, by land use and drain type, for sizing the silt traps of Clause 6.13.3, and a calibration of the sediment allowance of Clause 6.7.8 against measured silt depths. It needs desilting records (volume or mass removed per drain length and per catchment area, with dates), silt-depth surveys before and after the south-west monsoon, and catchment land-use and soil data (data and likely sources: desilting records of Raipur Municipal Corporation and other urban local bodies, WRD records for channels, land cover from NRSC, soils from the Chhattisgarh soil survey and NBSS&LUP, rainfall from IMD). Until it is published, size each silt trap from the designer's stated estimate of the sediment expected between scheduled cleanings, based on the desilting records of the same drain or of similar drains in the same local body, and state the cleaning interval assumed; the interval shall not be longer than the yearly pre-monsoon desilting of Clause 6.13.10. Inlet silt sumps remain at least 300 mm deep (Clause 6.6.7), and the sediment allowance of Clause 6.7.8 applies unchanged.

The intended method is a simple volume balance, with every term to be stated in the design:

> $V_{trap} = Y \cdot A \cdot t_c$

where V_{trap} is the sediment storage volume of the trap below its outlet invert (m^3), Y is the sediment yield for the catchment's land use and drain type (m^3/ha per year, from Table 6B.2 once published), A is the catchment area draining to the trap (ha), and t_c is the scheduled cleaning interval (years). The annex will state whether a safety margin is added.

Table 6B.1 — Sediment allowances already set by Chapter 6 (provisional, to be calibrated)

Element	Sediment allowance at the invert	Clause
Open and covered drains	Greater of 75 mm and 10% of the clear internal depth	6.7.8(a)
Pipes of 600 mm diameter or more	5% of the internal diameter	6.7.8(b)
Pipes below 600 mm	No allowance; minimum velocity of Clause 6.7.3(a) applies	6.7.8(c)
Drains and pipes that cannot reach the minimum velocity	Allowance doubled; mechanical desilting access provided	6.7.4(b), (c)
Inlet silt sumps	At least 300 mm below the outlet pipe invert	6.6.7

Table 6B.2 — Sediment yield by land use and drain type (layout)

Land use or catchment type	Drain type	Sediment yield Y (m^3/ha per year)	Bulk density (t/m^3)	Local body and records used	Years of record
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____

Table 6B.3 — Calibration of the sediment allowance (layout)

Drain or pipe ID	Clear internal depth or diameter (mm)	Allowance under Clause 6.7.8 (mm)	Measured silt depth before pre-monsoon desilting (mm)	Measured silt depth at the end of the monsoon (mm)	Ratio of measured to allowance
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____

Annex 6C — Standard drawings: covered roadside drain with removable covers, access chamber, inlet with silt sump

Supports Clauses 6.6, 6.7.6 to 6.7.10 and 6.13 of Chapter 6, and Clauses 9.11 and 9.12 of Chapter 9.

The drawings below are schematic standard drawings. They show only the dimensions that Chapter 6 and Chapter 9 set. Wall and slab thicknesses, reinforcement, concrete grade and cover, chamber plan sizes and frame details are to be confirmed in design: they come from the structural design (Clause 9.1.2) and the exposure class of Clause 9.13. Table 6C.1 lists the dimensions shown and their source clauses.

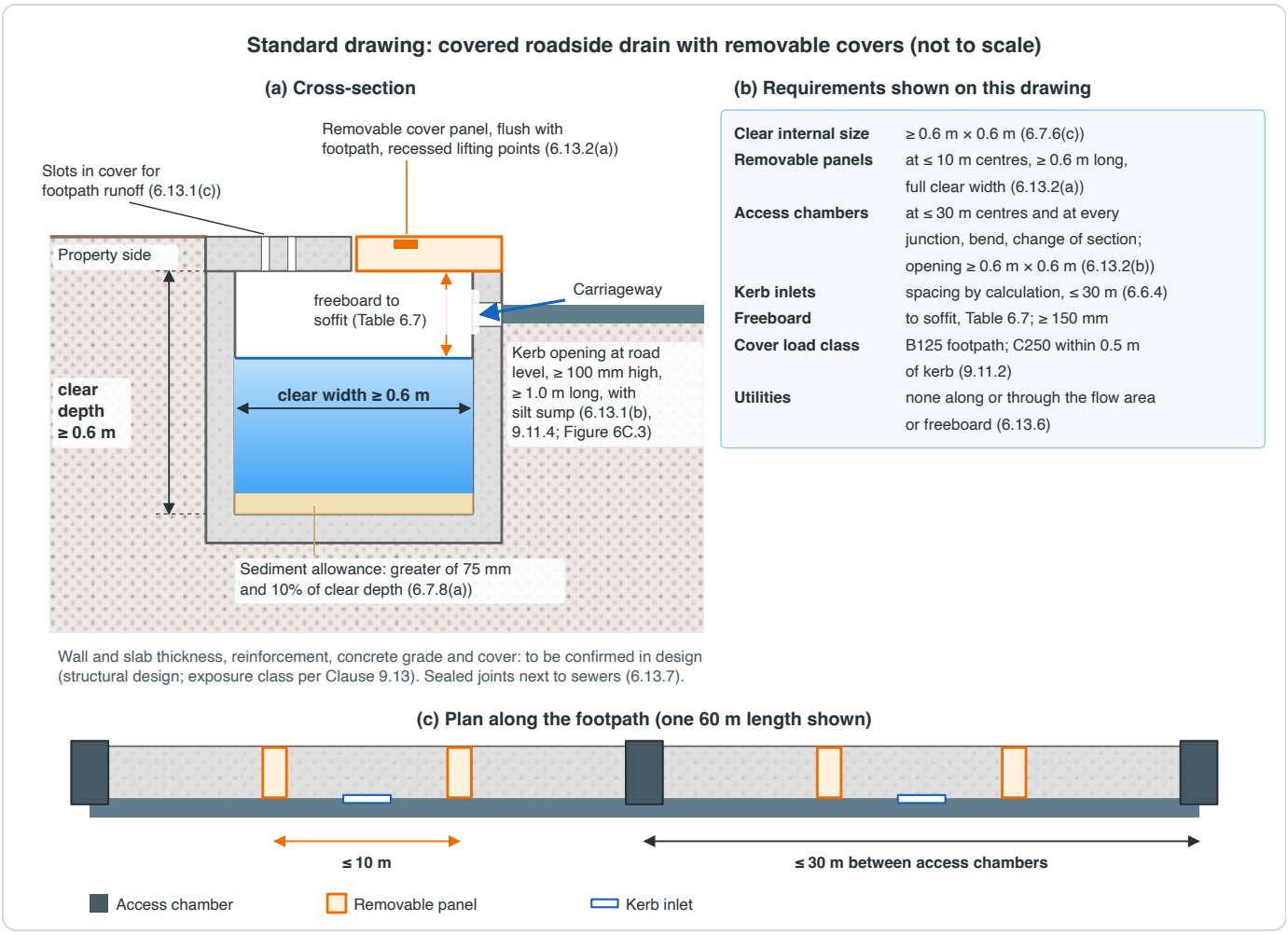

Figure 6C.1 — Standard drawing: covered roadside drain with removable covers, kerb inlets at road level, footpath slots, freeboard to the soffit and sediment allowance, with a plan of cover panels and access chambers along the footpath. Original drawing for this Manual, adapted from CPHEEO, Manual on Storm Water Drainage Systems (2019), Chapter 5, and IRC:SP:50-2013.

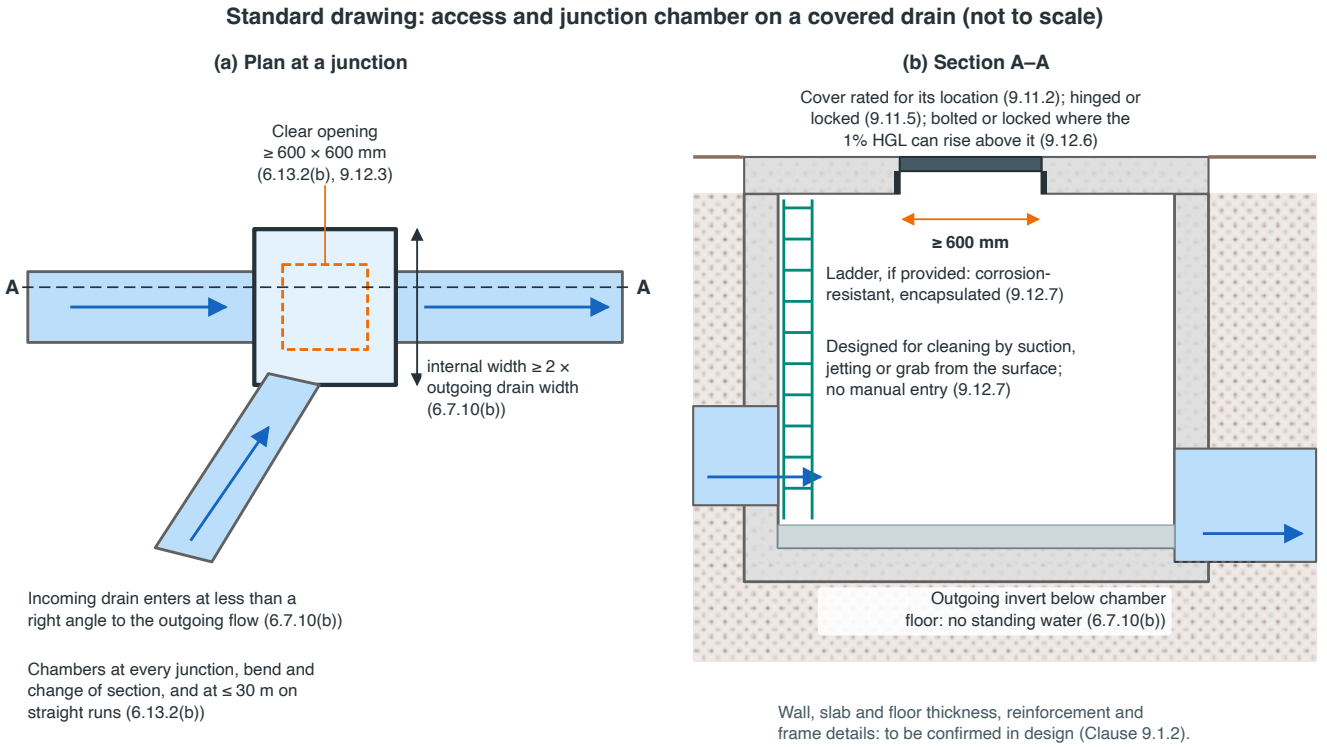

Figure 6C.2 — Standard drawing: access and junction chamber on a covered drain, with the clear opening, junction width, entry angle of the incoming drain, outgoing invert below the chamber floor, and cover requirements. Original drawing for this Manual, adapted from CPHEEO, Manual on Storm Water Drainage Systems (2019), §5.3.4 and Chapter 11.

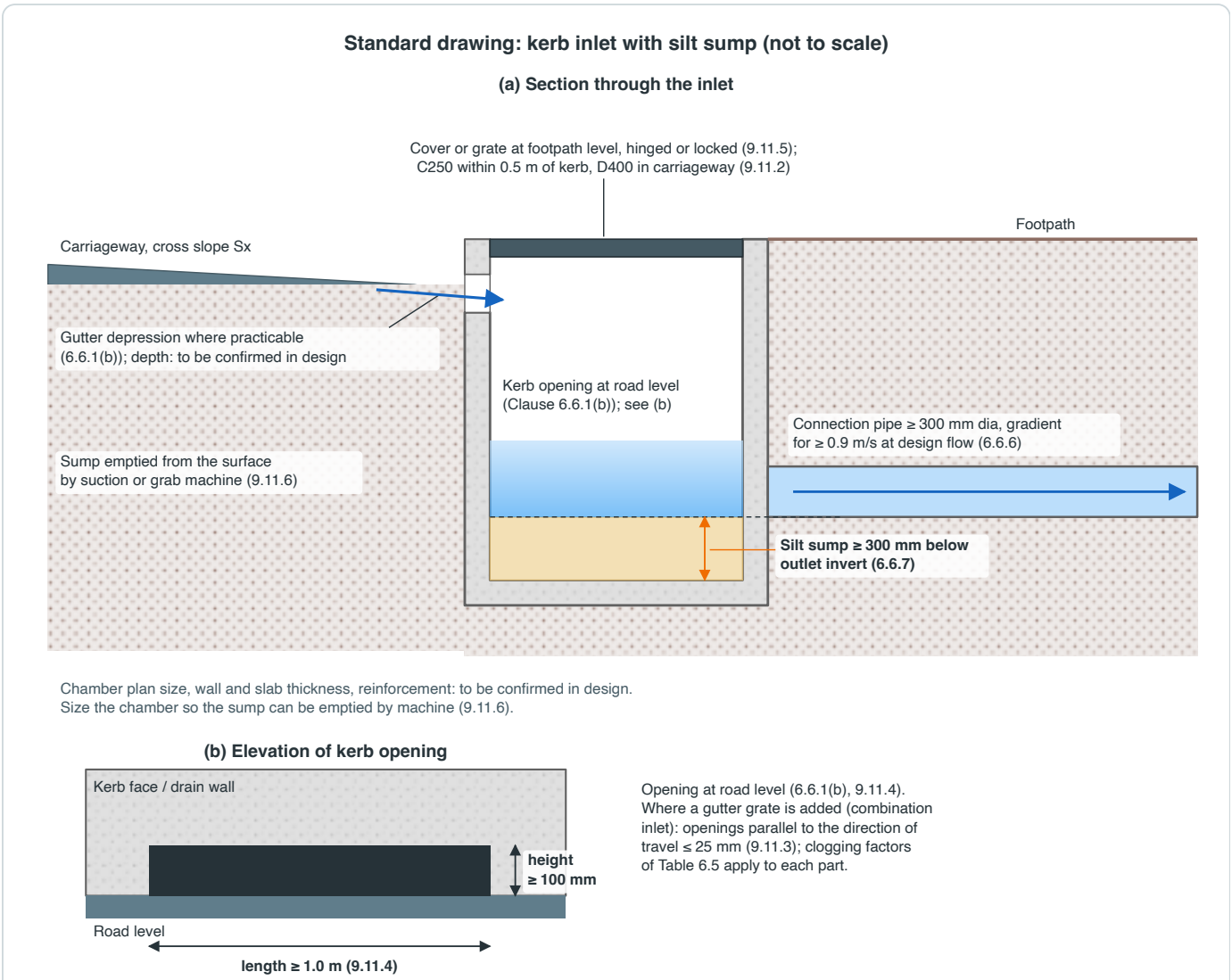

Figure 6C.3 — Standard drawing: kerb inlet at road level with silt sump, connection pipe and cover, and an elevation of the kerb opening. Original drawing for this Manual, adapted from CPHEEO, Manual on Storm Water Drainage Systems (2019), Figure 5.9, and FHWA, HEC-22 Urban Drainage Design Manual (4th ed., 2024).

Table 6C.1 — Dimensions on the standard drawings and their source clauses

Drawing	Item	Dimension or rule	Clause
6C.1	Clear internal size of a covered public drain	At least 0.6 m wide and 0.6 m deep	6.7.6(c)
6C.1	Freeboard to the cover soffit	Table 6.7; not less than 150 mm for any roadside drain	6.7.9, 6.13.1(a)
6C.1	Sediment allowance	Greater of 75 mm and 10% of clear depth	6.7.8(a)
6C.1	Removable cover panels	At not more than 10 m centres; each at least 0.6 m long and spanning the full clear width; recessed lifting points; flush with the footpath	6.13.2(a)
6C.1	Footpath runoff	Through slots or grated openings in the cover	6.13.1(c)
6C.1	Kerb inlets	Openings in the drain wall at road level, each with a silt sump; spacing by calculation, not more than 30 m	6.13.1(b), 6.6.4
6C.1, 6C.2	Access chambers	At every junction, bend and change of section, and at not more than 30 m centres on straight runs	6.13.2(b)

Drawing	Item	Dimension or rule	Clause
6C.2	Clear access opening	At least 0.6 m × 0.6 m (600 mm)	6.13.2(b), 9.12.3
6C.2	Junction chamber	Internal width at least twice the outgoing drain width; incoming drains at less than a right angle; outgoing invert below the chamber floor	6.7.10(b)
6C.2	Covers	Load class for the location; hinged or locked; bolted or locked where the 1% AEP HGL can rise above the cover	9.11.2, 9.11.5, 9.12.6
6C.2	Cleaning	By machine from the surface; ladders, where provided, corrosion-resistant and encapsulated	9.12.7
6C.3	Kerb opening	At road level; clear height at least 100 mm; length at least 1.0 m	6.6.1(b), 9.11.4
6C.3	Silt sump	At least 300 mm below the outlet pipe invert; emptied by suction or grab machine	6.6.7, 9.11.6
6C.3	Connection pipe	At least 300 mm diameter; at least 0.9 m/s at the design flow	6.6.6
6C.3	Grate in a combination inlet	Openings parallel to travel not wider than 25 mm; Table 6.5 clogging on each part	9.11.3, 6.6.3
All	Wall and slab thickness, reinforcement, concrete grade and cover, chamber plan size, gutter depression depth	To be confirmed in design	9.1.2, 9.13

Form 6-1 — Conveyance Design Report checklist

Supports Clause 6.15 of Chapter 6.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Not for statutory use.

Attach this checklist to the front of the Conveyance Design Report. For each item, enter Yes, No or Not applicable, and the drawing or page where it is shown.

Section A — Project

Table 6F.1 — Form 6-1, Section A: project

Field	Entry
Project name and location	_____
Local body, and Tier A or Tier B (Chapter 1)	_____
Applicant	_____
Designer (name and registration)	_____
Total catchment area served (ha)	_____
High-density area status for the 10% AEP minor system (Clause 6.4.4)	_____
Within the coastal band of Clause 6.11.6 (Yes / No)	_____
Date and revision	_____

Section B — Contents required by Clause 6.15

Table 6F.2 — Form 6-1, Section B: contents required by Clause 6.15

No.	Item	Clause	Provided (Yes / No / NA)	Drawing or page
1	Catchment plan: sub-catchments, minor system, inlet locations and the 1% AEP major-system overland flow paths	6.3.2		
2	Land-use and road-class AEP adopted for each element, and the high-density area status	Table 6.1, 6.4.4		
3	Design flows, with the rainfall sheet of Chapter 2 and the method of Chapter 3	6.3.4		
4	Gutter spread, depth and D·V checks for the minor, check and 1% events	Tables 6.2, 6.3		
5	Inlet schedule: clean capacity, clogging factor, design capacity and bypass flow	6.6		
6	Pipe and drain design tables: n, velocity, sediment allowance and freeboard	6.7		
7	HGL profiles for the minor and check events, with the tailwater used	6.8		
8	Sag ponding levels, including the fully-blocked check	6.10		
9	Floor-level schedule for every building	6.11		
10	Culvert and crossing calculations (with Form 9-1)	6.12, Chapter 9		

No.	Item	Clause	Provided (Yes / No / NA)	Drawing or page
11	Utilities-crossing schedule and access-opening layout for covered drains	6.13		

Section C — Design basis

Table 6F.3 — Form 6-1, Section C: design basis

Field	Entry	Clause
Minor-system design AEP adopted	_____	Table 6.1
Road class(es) and design / check AEPs for road drainage and crossings	_____	Table 6.1, 6.4.2
Rainfall source and climate factor applied	_____	6.3.4, Chapter 2
Method: Rational Method (catchment ≤ 16 ha, single element) or hydrograph method with routing	_____	6.3.4(a), (b)
Minimum time of concentration used (not less than 10 min)	_____	6.3.4(c)
Tailwater at the outfall for the HGL (MHWS + 0.3 m at tidal outfalls, or the receiving level)	_____	6.8.2
Any transfer of flow between natural catchments (none unless accepted under 6.3.5)	_____	6.3.5

Section D — Compliance checks

Table 6F.4 — Form 6-1, Section D: compliance checks

No.	Check	Limit	Clause	Complies (Yes / No / NA)
D1	Every inlet, sag, pipe, drain, culvert and storage element has a 1% AEP overland path	Shown on plan	6.3.2	
D2	Major-system flow = 1% AEP peak less the clogged minor-system capacity	Computed at each point	6.3.3	
D3	Spread in the minor event	Table 6.2	6.5.2	
D4	Depth in the check and 1% events	≤ 150 mm at the crown; ≤ 350 mm at the deepest point	Table 6.2	
D5	Flow hazard (D·V) by location	Table 6.3	6.5.3	
D6	Inlet clogging factors applied	Table 6.5	6.6.3	
D7	Inlet spacing on kerbed roads	≤ 30 m	6.6.4(a)	
D8	Sag inlet design capacity ≥ minor design flow arriving; flanking inlets where required	—	6.6.5	
D9	Inlet connection pipes and silt sumps	≥ 300 mm dia, ≥ 0.9 m/s; sump ≥ 300 mm	6.6.6, 6.6.7	
D10	Manning's n not less than the minimum design values	Table 6.6	6.7.2	
D11	Minimum velocity at the minor design flow	0.6 m/s drains; 0.9 m/s pipes	6.7.3(a)	
D12	Maximum velocity and Froude number	3.0 m/s lined drains and pipes; Fr ≤ 0.8	6.7.3(b), (d)	

No.	Check	Limit	Clause	Complies (Yes / No / NA)
D13	Where minimum velocity cannot be met: velocity reported, sediment allowance doubled, desilting access provided	—	6.7.4	
D14	Pipes sized for pipe-full capacity, not the 0.95-depth peak	—	6.7.5	
D15	Minimum sizes	Pipes 300 mm (450 mm public mains); open drain bed 0.3 m; covered public drains 0.6 m × 0.6 m	6.7.6	
D16	Cover to pipes; flotation check where groundwater is within 1 m of the crown	≥ 0.6 m; ≥ 0.9 m under carriageways	6.7.7	
D17	Sediment allowance in capacity	6.7.8 values	6.7.8	
D18	Freeboard, including superelevation at bends	Table 6.7	6.7.9	
D19	Bend radii and junction sumps	6.7.10	6.7.10	
D20	Manhole spacing on straight runs	Table 6.8	6.7.11	
D21	HGL below inlet rims and manhole covers in the minor event	≥ 0.3 m	6.8.3	
D22	Manholes that can surcharge above ground: bolted or locked covers	—	6.8.4	
D23	Soffits matched or outgoing soffit lower; outgoing invert not above the lowest incoming invert	—	6.8.6	
D24	Gate and valve losses included in the HGL; non-return valves on private connections where required	—	6.8.7, 6.8.8	
D25	Overland flow paths outside streets: width, tenure, no obstruction, continuous fall	≥ 1% top width + 0.5 m each side, ≥ 3.0 m	6.9.2	
D26	Compound walls across mapped flow paths: openings pass the 1% flow at 50% blockage	—	6.9.3	
D27	Access strip along open drains with top width ≥ 2 m	≥ 3.5 m, linked to a road at ≤ 500 m	6.9.4	
D28	Drainage reserve where required: width, separate plot, no structures, markers, services, 2 m stability check	Table 6.9	6.9.6–6.9.8	
D29	Sag ponding in the 1% event, overflow route, and fully-blocked check	≤ 350 mm at the deepest point	6.10.2, 6.10.3	
D30	Trapped sags and road underpasses	6.10.4, 6.10.5	6.10.4, 6.10.5	
D31	Floor levels	≥ 1% AEP level + 0.5 m; defaults of 6.11.3 where no level is computed	6.11.1–6.11.3	
D32	Basements and building services	6.11.4, 6.11.5	6.11.4, 6.11.5	
D33	Coastal band: floor ≥ 1% still-water level + 2 × SLA (+ wave set-up where applicable) + 0.5 m; no basement rooms or plant	—	6.11.6	
D34	Culverts: design AEP, overtopping, 50% / 20% blockage check, minimum size	≥ 900 mm	6.12	

No.	Check	Limit	Clause	Complies (Yes / No / NA)
D35	Covered drains: removable panels and access chambers	≤ 10 m; ≤ 30 m	6.13.1, 6.13.2	
D36	Silt traps, screens with bypass and 50% blockage check	—	6.13.3, 6.13.4	
D37	No building over drains; no utilities in the flow area or freeboard; existing crossings deducted with 50% blockage	—	6.13.5, 6.13.6	
D38	No sewage connections; sealed joints next to sewers; edge protection on open drains deeper than 1.0 m	—	6.13.7, 6.13.8	
D39	Outfalls: tailwater, flap gates or non-return valves below the 1% still-water level, outlet protection	—	6.14	

Section E — Variances

Table 6F.5 — Form 6-1, Section E: variances

Clause	Requirement not met	Reason and proposed alternative	Approved by and date
_____	_____	_____	_____

Section F — Signatures

The report shall be prepared and signed by a registered professional, and the Professional Design Certificate (Form 10-3) attached (Clauses 10.6.1 and 10.6.3 of Chapter 10). Where a structure stands within 2 m of the edge of a drainage reserve, the stability design shall also be signed by a qualified structural engineer (Clause 6.9.8(f)).

Table 6F.6 — Form 6-1, Section F: signatures

Role	Name	Registration or empanelment no.	Signature	Date
Designer	_____	_____	_____	_____
Structural engineer (Clause 6.9.8(f), where applicable)	_____	_____	_____	_____

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

ANNEXES TO CHAPTER 7

Waterbodies, Talabs (Ponds) and Setbacks

These annexes support Chapter 7. They give the inventory field specification, the fixed-width setback table, a template operating rule, the desilting and restoration specification, a glossary of Hindi and Chhattisgarhi waterbody and soil terms, and the three forms that Clause 7.15 requires. Where an annex cannot yet be completed it says so and gives the rule to use in the meantime. The forms are draft forms; the template and model text are for consideration by a competent authority and are not in force.

Annex 7A — Statewide waterbody GIS layer and inventory fields

Supports Clauses 7.3.1, 7.3.3 and 7.3.5 of Chapter 7.

To be prepared. This annex will contain the statewide GIS layer of waterbodies required by Clause 7.3.1, prepared by each Authority with WRD and the Revenue and Disaster Management Department. It needs the Revenue records (khasra) and village maps of the Revenue Department, in which talabs, dabris, nalas and land covered by water are recorded, including the tanks vested in the State under section 251 of the Chhattisgarh Land Revenue Code, 1959 and the land set apart for nistar under section 237, and land "recorded as a pond, a water body" whose unauthorised occupation is proceeded against under section 248 (as at Baloda, NGT O.A. 78/2024); the public tanks, streams, drains and watercourses vested in a Municipal Corporation or other local body under the Municipal Acts as adapted in Chhattisgarh; the water bodies, rivers and nalas shown in a Development Plan under the 1973 Adhiniyam, such as the Raipur Development Plan (Revised) 2021, the Naya Raipur Development Plan-2031 and the Arpa Special Area Development Plan 2033, and the waterlogging areas, nalas and areas left for nalas recorded in the T&CP site reports and checklists (2011), with the high flood points supplied by WRD; the boundary of Kopra Jalashay as designated under the Ramsar Convention, of any wetland notified under the Wetlands (Conservation and Management) Rules, 2017, and of the talabs and wetlands that the Chhattisgarh State Wetland Authority treats as protected, with the survey of the water bodies of Raipur district that it directed on 8 May 2023; the mining plans and mine closure plans of mine lessees, which show voids, pit lakes, settling ponds and garland drains; and the ash pond and ash dyke layouts of the power plants; the WRD and Gram Panchayat tank registers with FTL and MWL; surveyed levels on a common datum; the ISRO National Wetland Inventory and Assessment (NRSC); Survey of India topographic maps; and any list of waterbodies published under court directions. Until it is published, each Applicant identifies waterbodies from all the sources of Clause 7.3.3, with the larger extent governing, and records them on the Waterbody Sheet (Form 7-1).

The field specification below is complete. It is written so that every field the chapter asks for (Clause 7.3.1(a) to (g)) has a defined name, type and source, and so that the same fields can be filled from a project's Form 7-1.

7A.1 Layers

- **WB_POLY** — one polygon per waterbody: the Revenue record boundary (Clause 7.3.1(c)).
- **WB_FTL** — the surveyed FTL contour (tanks, lakes, ponds) or top of bank (channels), as a polygon or line.
- **WB_MWL** — the MWL contour of each tank, as a polygon.

- **WB_FLOOD** — the 1% AEP flood extent, where mapped (Clause 7.3.1(f)), with the floodway where delineated (Clause 7.6.1).
- **WB_STRUCT** — points for surplus weirs, sluices, regulators, inlets and drain outfalls.
- **WB_LINK** — lines for supply channels, surplus courses and the links between tanks in a cascade (Clause 7.3.1(e)).
- **WB_ENCR** — polygons of known encroachments, each with the date recorded (Clause 7.3.1(g)).
- **WB_SETBACK** — Zone 1 and Zone 2 polygons (Clause 7.4), derived from the layers above.

All layers shall carry the waterbody identifier (WB_ID) so that they can be joined. All levels shall be stated to one datum (Clause 7.3.5), named in the layer metadata.

Table 7A.1 — Attribute fields of the waterbody inventory (WB_POLY)

Field	Type	Unit / domain	Required	Content	Clause
WB_ID	Text	District code + serial	Yes	Unique identifier, never reused	7.3.1
WB_NAME	Text	—	Yes	Name as in Revenue and WRD records, with local name	7.3.1(a)
WB_NAME_HI	Text	Hindi script	Yes	Name in Hindi	7.3.1(a)
WB_TYPE	Code	Table 7A.2	Yes	Waterbody type	7.2, 7.3.1(a)
SURVEY_NOS	Text	Village, survey and subdivision numbers	Yes	Revenue survey numbers of the waterbody	7.3.1(a)
VILLAGE	Text	—	Yes	Revenue village, tehsil and district	7.3.1(a)
CUSTODIAN	Code	Table 7A.2	Yes	Custodian body	7.3.1(b)
AREA_REC	Number	ha	Yes	Area within the Revenue record boundary	7.3.1(c)
AREA_FTL	Number	ha	Tanks	Waterspread at FTL, surveyed	7.2, 7.3.1(d)
FTL	Number	m (datum)	Tanks	Full tank level	7.3.1(d)
MWL	Number	m (datum)	Tanks	Maximum water level (Clause 7.2 definition)	7.3.1(d)
MWL_BASIS	Code	WRD record / 1% AEP computed	Tanks	Which value governs MWL	7.2
TBL	Number	m (datum)	Tanks	Top bund level, lowest point	7.3.1(d)
WEIR_CREST	Number	m (datum)	Tanks	Surplus weir crest level	7.3.1(d)
WEIR_LEN	Number	m	Tanks	Surplus weir crest length	7.3.1(d)
WEIR_TYPE	Text	—	Tanks	Surplus weir type (for example fixed crest, with shutters or falling boards)	7.3.1(d)
SLUICE_N	Integer	—	Tanks	Number of sluices; details in WB_STRUCT	7.3.1(d)
SUPPLY_CH	Text	WB_ID(s)	Tanks	Supply channel(s) feeding the tank	7.3.1(d)
SURPLUS_CS	Text	WB_ID(s)	Tanks	Surplus course(s) leaving the tank	7.3.1(d)
UPSTR_WB	Text	WB_ID(s)	Cascades	Upstream tank(s) in the cascade	7.3.1(e)
DNSTR_WB	Text	WB_ID	Cascades	Downstream tank, river, drain or sea outfall	7.3.1(e)
CATCH_AREA	Number	ha	Channels	Drainage area at the downstream end of the reach (for the 8 ha trigger of Clause 7.4.1(b) and Table 7.1)	7.4.1

Field	Type	Unit / domain	Required	Content	Clause
DATUM	Text	—	Yes	Datum of all levels; conversion from any older WRD datum	7.3.5
LEVEL_DATE	Date	—	Tanks	Date of the level survey	7.7.1(a)
SSC_DATE	Date	—	Tanks	Date of the latest stage–storage survey	7.7.1(b)
FLOOD_SRC	Code	Adopted map / site study / none	Yes	Source of the 1% AEP extent	7.3.4
FLOODWAY	Code	Delineated / whole 1% extent / below MWL	Yes	Basis of the floodway	7.6.1
ENCR_KNOWN	Code	Yes / No	Yes	Known encroachment; details in WB_ENCR	7.3.1(g)
CREDITED	Code	Yes / No	Tanks	Storage counted under Clause 7.7, with the scheme reference	7.7.1
WETLAND	Code	Notified / listed / other	Wetlands	Status under Clause 7.12.1	7.12.1
SOURCES	Text	—	Yes	Sources used, from the list in Clause 7.3.3(a) to (e)	7.3.3
UPDATED	Date	—	Yes	Date of the last edit, and by whom	7.3.1

Table 7A.2 — Code lists

Code list	Values	Clause
WB_TYPE	River; natural drain or nala (narwa); canal; reservoir, jalashay or barrage pond; lake; talab or pond; bandh; dabri (small dug pond); kund; marsh or low wet land; floodplain wetland or depression; mine void or pit lake; ash pond; abandoned quarry or borrow pit; supply channel; surplus course	7.2
CUSTODIAN	Municipal Corporation, Nagar Palika Parishad or Nagar Panchayat (Municipal Acts as adapted); NRDA (Nava Raipur Atal Nagar); RDA, Chhattisgarh Housing Board or developer (until handed over); Bhilai Steel Plant (township); WRD (division); State Government through the Collector or Tehsildar (LRC ss. 237, 251); Gram Panchayat; mine lessee (SECL or other; DGMS); power company (NTPC, CSPGCL, BALCO or other; ash pond); Chhattisgarh State Wetland Authority (wetland) or WRD (Kopra Ramsar site); private owner; other (name)	7.3.1(b)
Structure (WB_STRUCT)	Surplus weir; sluice; regulator; inlet; drain outfall; gross pollutant trap; crossing	7.3.1(d), 7.10
Encroachment (WB_ENCR)	Building; fill; road; wall; dumping; cultivation; other — each with date recorded, area (m²) and source	7.3.1(g), 7.13

Table 7A.3 — Attribute fields of structures (WB_STRUCT)

Field	Type	Unit	Content	Clause
STR_ID	Text	—	Unique identifier	7.3.1(d)
WB_ID	Text	—	Waterbody served	7.3.1(d)
STR_TYPE	Code	Table 7A.2	Structure type	7.3.1(d)
CREST_SILL	Number	m (datum)	Weir crest or sluice sill level	7.7.1(a)
LENGTH_SIZE	Text	m	Weir crest length, or sluice opening size	7.7.1(a)
GATED	Code	Yes / No	Controllable outlet (needed for a flood cushion, Clause 7.8.2(a))	7.8.2
CONDITION	Code	Serviceable / defective	Result of the latest pre-monsoon inspection, with date	7.7.1(d), 7.14.4

Annex 7B — Fixed-width setback table (alternative S-3, with Annex 1C)

Supports Clauses 7.3.4, 7.4.3, 7.4.6 and 7.4.7 of Chapter 7, and alternative S-3 of the Policy Supplement (Annex 1C).

7B.1 When the table may be used

The fixed-width setback may be used only where all of the following apply:

- (a) the Authority has not adopted a 1% AEP flood extent map for the waterbody (Clause 7.3.4);
- (b) Clause 7.3.4 does not require the Applicant to determine the 1% AEP extent by a hydraulic study: the site is not in a flood-sensitive catchment, is not a site of 1 ha or more adjoining a waterbody, and the Authority has not asked for a study; and
- (c) the waterbody is one to which setbacks apply (Clause 7.4.1): any recorded or found waterbody, and any natural channel draining 8 ha or more at the site.

Where an Authority adopts alternative S-3 under Clause 1.7, the table below is the S-3 table. Clause 1.7.3(e) provides for the S-3 table to be issued by the State Stormwater Technical Committee. Until it is issued, the values are those of Clause 7.4.6, repeated here without change.

Table 7B.1 — Fixed-width setback by waterbody class (alternative S-3)

Waterbody class	Drainage area at the site	Total setback (Zone 1 + Zone 2)	Of which Zone 1	Measured from
Channel, small	8 ha to 65 ha	25 m	15 m	Top of bank (Clause 7.4.3)
Channel, medium	65 ha to 20 km²	40 m	15 m	Top of bank
Channel, large, and rivers	More than 20 km²	50 m	20 m	Top of bank
Tank, lake or pond	Any	MWL contour + 25 m	To the MWL contour, and not less than 15 m from FTL	FTL contour (Zone 1); MWL contour (total)
Marsh, estuary or backwater	Any	As notified by the Authority, and not less than 50 m	15 m	The waterbody boundary of Clause 7.13.1

Values from Clause 7.4.6 (Table 7.1). Zone 2 is the total setback less Zone 1.

7B.2 Rules of application

- (a) **Measurement.** Measure horizontally on plan, outward from the boundary of Clause 7.13.1: the larger of the Revenue record boundary and the surveyed FTL (or top of bank). Where FTL and top of bank both apply, use whichever gives the wider zone (Clause 7.4.3).
- (b) **Drainage area.** For a channel, use the drainage area at the downstream end of the reach that adjoins the site. Where a reach crosses a class limit within the site, apply the wider setback to the whole reach within the site.
- (c) **Both banks.** The setback applies on each bank of a channel and all round a tank, lake, pond or marsh.
- (d) **Minimum set by this Manual.** Zone 1 is never less than 15 m (Clause 7.4.7). A wider statutory or approval setback governs for the uses it restricts (Clause 7.4.5).
- (e) **Stricter of the two.** Where a hydraulic study is made voluntarily, the setback is the wider of Table 7B.1 and Clauses 7.4.3 and 7.4.4.
- (f) **Record.** The basis of the setback (fixed width) shall be stated on the Waterbody Sheet (Form 7-1) and in the approval (Clause 7.4, Disclosure).
- (g) **Review.** The table shall be reviewed within 2 years of the Authority adopting 1% AEP flood maps (Clause 7.16.1).

7B.3 Source of the channel widths

The channel widths were set by this Manual with reference to the 2006 Kansas City APWA default stream buffers, rounded up and floored at the 15 m minimum (Clause 7.4.6, commentary). The source values, as cited in the chapter, are:

Table 7B.2 — Source buffers cited in Clause 7.4.6 (KC APWA 5600, 2006, §5605.3)

Drainage area (source, acres)	Approximate SI equivalent	Source buffer
Less than 40 ac	Less than 16 ha	40 ft (12 m)
40–160 ac	16–65 ha	60 ft (18 m)
160–5,000 ac	65 ha – 20 km²	100 ft (30 m)
More than 5,000 ac	More than 20 km²	120 ft (37 m)

The 65 ha and 20 km² class limits in Table 7B.1 are the SI equivalents of 160 ac and 5,000 ac. The 8 ha lower limit is the 20-acre trigger of APWA 2025.01 (Clause 7.4.1(b)).

Annex 7C — Template tank operating rule, with flood cushion

Supports Clauses 7.7.1(c), 7.7.3, 7.8 and 7.14.4 of Chapter 7.

Model text for consideration by a competent authority. Not in force. For tanks under WRD control, the operating rule is WRD's rule (Clause 7.8.4); this template sets out the content that Clause 7.8 asks for, so that the custodian's rule can be checked against it. Text in square brackets is to be completed for each tank.

7C.1 Identification

1. Tank: [name, WB_ID of Annex 7A, village, tehsil, district].
2. Custodian: [WRD division / ULB / Gram Panchayat / other].
3. Cascade position: upstream tank(s) [ ]; downstream tank, river or drain [ ].
4. Datum: [ ]. Surveyed levels (Clause 7.7.1(a)), date of survey [ ]: FTL [ ] m; MWL [ ] m; TBL (lowest point) [ ] m; surplus weir crest [ ] m, length [ ] m; sluice sills [ ] m.
5. Stage–storage curve (Clause 7.7.1(b)): survey date [ ]; attached as Schedule 1.
6. Controllable outlets (Clause 7.8.2(a)): [gated sluice / regulator / none]; capacity at FTL [ ] m³/s.

7C.2 Purposes and priority

7. The tank serves [irrigation of a command area of [ ] ha / drinking water supply / recharge / flood storage]. Irrigation and water supply remain primary unless the custodian records otherwise (Clause 7.8.2(d)).

7C.3 Levels and dates through the year (Clause 7.8.1(a))

8. The tank shall be operated to the levels in Table 7C.1.

Table 7C.1 — Rule levels by period (to be completed for each tank)

Period	Target level (m)	Upper limit (m)	Lower limit (m)	Purpose
[1 Jan – date]	[ ]	FTL	[ ]	Holding for irrigation / supply

Period	Target level (m)	Upper limit (m)	Lower limit (m)	Purpose
[date – start of cushion period]	[ ]	FTL	[ ]	Holding
[Flood-cushion period, if any]	Cushion level [ ]	Cushion level [ ]	[ ]	Flood cushion (Clause 7.8.2)
[End of cushion period – 31 Dec]	[ ]	FTL	[ ]	Refilling

9. Where no flood cushion is adopted, the starting level for design is FTL (Clause 7.7.3) and the row for the cushion period is deleted.

7C.4 Flood cushion (Clause 7.8.2)

10. Cushion level: [ ] m, which is [ ] m below FTL, giving a cushion volume of [ ] m³ from Schedule 1.
11. Dates: the cushion applies from [ ] to [ ].
12. Drawdown: the tank can be drawn down from FTL to the cushion level through [outlet] in [ ] hours, with a release not exceeding the maximum release rate of paragraph 18.
13. Reliability: continuous simulation (Clause 3.13) over the record [years] shows that the cushion is available at the start of a design storm in [ ]% of SW monsoon seasons, which shall be at least 90%. The simulation report is Schedule 2.
14. Yield: the custodian accepts the loss of irrigation or supply yield stated in Schedule 3 [and the compensation recorded there].
15. Where the tank has no controllable outlet, this section does not apply and no flood cushion is assumed (Clause 7.8.3).

7C.5 Releases (Clause 7.8.1(b) to (d))

16. Sluice and regulator settings at each level: Table 7C.2.
17. Operator: the sluices and regulators are operated by [post]. A release above [ ] m³/s may be ordered only by [post], or, in an emergency, by [post].
18. Maximum release rate: [ ] m³/s. This rate, added to the flow from the downstream catchment, does not flood buildings downstream (Schedule 4 gives the downstream check).
19. No person shall cut, lower or breach the bund or surplus weir except under the custodian's emergency procedure (Clause 7.14.5).

Table 7C.2 — Outlet settings by level (to be completed for each tank)

Tank level (m)	Sluice 1	Sluice 2	Regulator	Expected outflow (m³/s)	Notes
Below cushion level	[ ]	[ ]	[ ]	[ ]	
Cushion level to FTL	[ ]	[ ]	[ ]	[ ]	
FTL (weir begins to spill)	[ ]	[ ]	[ ]	[ ]	
Above FTL to MWL	[ ]	[ ]	[ ]	[ ]	Check freeboard to TBL (Clause 7.7.4(b))

7C.6 Forecasts, warnings and communication (Clause 7.8.1(e) and (f))

20. The triggers and actions are in Table 7C.3.
21. Before any release under this rule, the operator shall inform the ULBs and the district administration downstream, and residents by the means stated in Schedule 5, at least [ ] hours before the release where the forecast allows.

Table 7C.3 — Triggers and actions (to be completed for each tank)

Trigger	Source	Action	By whom
IMD district heavy-rainfall warning for the catchment	IMD	Read gauges [ ] hourly; check outlets are free; confirm the tank is at or below its rule level	[ ]
IMD or CGSDMA red alert	IMD, CGSDMA	Begin releases needed to reach the rule level, within the maximum release rate; notify downstream bodies (paragraph 21)	[ ]
City forecast system warning, where available	[ ]	[ ]	[ ]
Tank level reaches FTL	Gauge	Record outflow hourly; inspect bund and weir	[ ]
Tank level reaches [MWL − [ ] m]	Gauge	Emergency procedure; warning to the breach area of any emergency action plan (Clause 7.14.3(c))	[ ]

7C.7 Records and review (Clause 7.8.1(g))

22. During the SW monsoon (10 June to 20 October) the operator shall record daily: the tank level, every release (time, rate and duration), and every gate setting. Outside the SW monsoon, records shall be kept [weekly].
23. Records shall be kept with the O&M plan (Chapter 10) and given to the Authority on request.
24. The tank shall be inspected before the SW monsoon (by 31 May) and after every heavy-rainfall event (Clause 7.14.4).
25. This rule shall be reviewed after any event in which the tank surplused at a level above [ ] m, after any change to the weir, sluices or bund (Clause 7.7.6), and in any case with the review of the Manual.

Schedules: 1 stage–storage curve; 2 continuous simulation report; 3 yield and compensation; 4 downstream check of the maximum release; 5 communication list.

Annex 7D — Desilting and restoration specification

Supports Clauses 7.7.1(b), 7.9 and 7.14.4 of Chapter 7.

This specification applies to every desilting, restoration or deepening of a tank, pond or other waterbody under Clause 7.9. Values are those of Chapter 7. Where the chapter does not set a value, it is marked "to be confirmed in design" and shall be stated in the works design.

7D.1 Pre-works survey (Clause 7.9.1)

- (a) **Topographic and bathymetric survey** of the waterspread, foreshore (to the MWL contour), bund, surplus weir and sluices, levelled to the datum of Clause 7.3.5. Survey grid and point density: to be confirmed in design.
- (b) **Original bed level** found by test pits or augering on a grid (grid spacing to be confirmed in design), and from historical records where available. Each pit log shall show the silt thickness, the level of the original bed, and any clay or silt layer below it.
- (c) **Silt testing** where the tank receives urban runoff or sewage: samples from the pits of (b), tested for heavy metals and other contaminants before disposal is planned. The parameters and limits shall be those the Chhattisgarh Environment Conservation Board (CECB) directs.
- (d) **Structures:** condition of the bund (seepage, slips, cracks, animal burrows, trees), surplus weir and sluices, as in Clause 7.14.4.
- (e) **Existing stage–storage curve** from the survey of (a).

7D.2 Limits of excavation (Clauses 7.9.2 and 7.9.3)

- (a) Desilting removes accumulated sediment down to the original bed level of 7D.1(b), and no deeper.
- (b) Deepening below the original bed only with a hydrogeological report showing each of Clause 7.9.3(a) to (c).
- (c) Storage created below the lowest sluice sill is not flood storage, and shall not be shown as such in the revised stage–storage curve.
- (d) Excavation shall stay clear of the bund toe by a distance to be confirmed in design, so that the bund's stability is not reduced.

7D.3 Edges and safety (Clause 7.9.4)

Where the public has access to the edge:

- (a) side slopes no steeper than **1V:4H** from **0.5 m above FTL** down to **1.2 m below FTL**; or
- (b) a level safety bench at least **3 m** wide, between **0.3 m and 0.5 m below FTL**; and
- (c) no isolated deep pit or vertical cut left in the bed. Borrow pits shall be regraded to the bed.

7D.4 Storage not reduced (Clause 7.9.5)

- (a) Islands, walkways, bund widening into the waterspread, ring roads, parking, buildings and lake-front platforms within the waterspread or foreshore are lost storage.
- (b) Lost storage shall be offset by compensatory storage at equivalent levels, band by band, using Form 7-3.

7D.5 Silt handling and disposal (Clause 7.9.6)

- (a) Desilted material shall not be placed within the setback, on the bund (except approved bund strengthening), or on any flood path.
- (b) Contaminated silt shall be disposed of as the Chhattisgarh Environment Conservation Board (CECB) directs.
- (c) Haul routes and temporary stockpiles shall meet the construction erosion and sediment control rules of Clause 8.12, including stockpiles outside Zone 1.

7D.6 Inlets (Clause 7.9.7)

Every inlet that brings urban runoff shall have a sediment forebay, a constructed wetland inlet zone or a gross pollutant trap, designed under Chapter 8 (Annex 8D).

7D.7 Timing (Clause 7.9.8)

Desilting shall be complete, and the waterbody made safe, by **31 May**. No excavation shall be left open, and no sluice or weir left dismantled, during the SW monsoon.

7D.8 Completion records

- (a) As-built survey and revised stage–storage curve. The curve is the one that Clause 7.7.1(b) requires "after the last desilting".
- (b) Volume removed, its disposal site, and the silt test results.
- (c) Photographs of the bund, weir, sluices and inlet works.
- (d) Update of the inventory record (Annex 7A: SSC_DATE, LEVEL_DATE, CONDITION).

Table 7D.1 — Hold points for desilting and restoration works

Hold point	Check	Clause
1. Before excavation	Pre-works survey, pit logs, silt tests, and hydrogeological report if deepening	7.9.1, 7.9.3
2. Original bed reached	Levels checked against pit logs; no excavation below the original bed unless approved	7.9.2
3. Edges formed	Side slopes or safety bench as 7D.3	7.9.4
4. Inlet works	Forebay, wetland inlet zone or GPT built	7.9.7
5. Completion (by 31 May)	As-built survey, stage–storage curve, silt disposal record	7.9.8, 7.7.1(b)

Annex 7E — Glossary of Hindi and Chhattisgarhi waterbody and soil terms and regional variants

Supports Clause 7.2 and Clause 7.3 of Chapter 7.

The terms below are Hindi words, and Chhattisgarhi words where they are in use, for waterbodies, drains and land in Chhattisgarh, given in Devanagari and, where Platts gives it, Urdu script. Many of the words appear in the names of talabs, reservoirs and nalas (Budha Talab, Maharajbandh, Kopra Jalashay, the Shankar nala). The name used in the Revenue, WRD, municipal or mine record for each waterbody should be recorded in the inventory (Annex 7A) alongside the Manual's term. Only terms whose meaning was confirmed in J. T. Platts, *A Dictionary of Urdu, Classical Hindi, and English* (1884), or in Syamasundara Dasa, *Hindi Shabdasagara* (1965–1975), both as served by the Digital South Asia Library, or in a statute, are listed in Tables 7E.1 to 7E.3; the meanings are Platts's English glosses, or a translation of the Shabdasagara, with the page shown by the Digital South Asia Library, and modern usage may be narrower or wider. **No Chhattisgarhi dictionary was consulted for this Manual.** The four soil classes of the plain, which are not in either dictionary, are given in Table 7E.4 with their meanings from agronomy papers; dhodi, and every other term whose meaning or regional use was not confirmed, are in Table 7E.5 as not yet verified. The list is to be extended by the State Stormwater Technical Committee, with modern Revenue, WRD and municipal usage and with a Chhattisgarhi dictionary.

The Manual's term for a tank or pond in Chhattisgarh is **talab (तालाब)**: it is the word of the State's own urban sources (Budha Talab, Karbala Talab, Narhaiya Talab), of the State Wetland Authority's orders and of the heading of section 251 of the Land Revenue Code ("तालाबों का राज्य सरकार में निहित होना"), and it is glossed by Platts as "Pond, pool, tank, reservoir of water"; dabri names a small dug pond, too small for urban detention; and bandh is the right word for a bund or dam and the name of embanked tanks such as Maharajbandh and Telibandha, but it also means a strike or shutdown in everyday speech, so it is a poor word on its own for a detention pond. Dabri, bandh, kund, nala, narwa and nistar keep their own meanings in this Manual.

Table 7E.1 — Waterbodies

Term	Meaning	Source of the term	Manual term (Clause 7.2)
तालाब / تالاب (tālāb; talab)	"Pond, pool, tank, reservoir of water (= talā'o ; cf. tāl)"; in the Shabdasagara, "जलाशय । सरोवर । पोखरा ।", a reservoir; a lake; a pond	Platts, p. 306; Shabdasagara, p. 2071	Talab (tank or pond), see the definitions after Clause 7.2
डबरी (ḍabrī; dabri)	In the Shabdasagara, "छोटा गड्ढा या ताल", a small pit or pond; Platts's डबरी is a different word, "An earthen water-vessel, a cup, bowl"	Shabdasagara, p. 1938; Platts, p. 564	Dabri (small dug pond; waterbody for Clause 7.3, storage not counted), see the definitions after Clause 7.2

Term	Meaning	Source of the term	Manual term (Clause 7.2)
डबरा (ḍabrā; dabra)	In the Shabdasagara, "छिछला लंबा गड्ढा जिसमें पानी जमा रहे । कुंड । हौज । … वह नीची भूमि का टूकड़ा जिसमें पानी लगता हो", a shallow long pit in which water collects; a kund; a cistern; a piece of low land in which water stands; Platts gives the related डाबर as "Low or hollow ground where water collects (= ḍabrā …), a marsh, swamp, a pool, puddle, a small pond, pit, tank"	Shabdasagara, p. 1938; Platts (page not recorded)	Talab (tank or pond), or low wet land (Clause 7.12)
कुंड (kuṇḍ; kund)	In the Shabdasagara (कुंड ३), "छोटा बँधा हुआ जलाशय । बहुत छोटा तालाब ।", a small embanked reservoir; a very small tank	Shabdasagara, p. 964	Kund (tank or pond where it receives runoff), see the definitions after Clause 7.2

Table 7E.2 — Channels, embankments and parts of the system

Term	Meaning	Source of the term	Manual term (Clause 7.2)
नाला / نالا (nālā; nala, nallah, nullah)	In the Shabdasagara, "पृथ्वी पर लकीर के रूप में दूर तक गया हुआ गड्ढा जिससे होकर बरसाती पानी किसी नदी आदि में जाता है । जलप्रणाली ।", a channel running a long way across the land like a line, by which rainwater goes to a river; a water channel	Shabdasagara, p. 2596	Natural drain or nala, see the definitions after Clause 7.2. In Chhattisgarh the word covers both natural watercourses and large drains that carry sewage, such as the Shankar and Pulgaon nalas of Durg; the definition, not the word, decides which this chapter applies to
नरवा (narvā; narwa)	In the Shabdasagara (नरवा २), "[हिं० नाला] दे० 'नाला'", the same as nala	Shabdasagara, p. 2541	Natural drain or nala; narwa is the stream word of the State's Narwa–Garuwa–Ghuruwa–Bari programme (as reported)
नाली (nālī; nali)	The diminutive of nala, a small channel	Shabdasagara, under नाला	Drain (minor system); where it carries sewage or sullage into a storm drain, a cross-connection (Clause 8.9)
बाँध, बँधा / باندھ (bāṅdh, baṅdhā; bandh, bund)	"… embankment; dam"; in the Shabdasagara, an embankment of earth or stone made along a river or tank to hold back floods	Platts, p. 127; Shabdasagara, p. 3433	Tank bund or dam (Clause 7.14); an embanked tank (bandh) is a tank, and its bund a tank bund (Clause 7.14.2)

Table 7E.3 — Statutory terms

Term	Meaning
nistar (निस्तार)	The rights of common use of village land and tanks by the village community. A tank on unoccupied land over which the community exercised rights of irrigation or nistar vests in the State from 6 April 1959 without taking away those rights (Chhattisgarh Land Revenue Code, 1959, s. 251, "तालाबों का राज्य सरकार में निहित होना"), and the Collector may set land apart for nistar, tanks among it (s. 237); see the definitions after Clause 7.2
tank (Land Revenue Code)	A tank vested in the State under section 251 includes the trees on its embankments, but not buildings or temples on them (Explanation to s. 251, as read in a third-party copy)
unauthorised occupation (Land Revenue Code)	"अप्राधिकृत रूप से भूमि पर कब्जा कर लेने के लिए शास्ति", the penalty for taking possession of land without authority, with summary eviction by the Tehsildar (s. 248), used against encroachment on ponds recorded in the land records (NGT O.A. 78/2024)
जलाशय (jalāśay; jalashay)	Reservoir or water body: the word of the Chhattisgarh Act of 2002 on the regularisation of unauthorised development, which bars regularisation where the construction is "जलाशय के तल या जलाशय के किनारे या किसी प्राकृतिक जल निकास पर" (on the bed or edge of a jalashay or on a natural drain) (s. 7(1)(iv)), and the name of Kopra Jalashay; the Shabdasagara uses it to gloss तालाब (p. 2071). In this Manual a jalashay is a reservoir or tank under Clause 7.2, and a "प्राकृतिक जल निकास" a natural drain

Term	Meaning
highest water mark; average high flood mark (CGBVN 1984, Rule 50(b), inherited text)	The levels from which the inherited distances of 9 m and 15 m are measured; not defined, and the Chhattisgarh text not read [TBC]. This Manual's reading is in the definitions after Clause 7.2
high flood point (उच्च बाढ़ बिन्दु)	The level that T&CP asks WRD to give for land next to a nala, canal, river or watercourse (checklist No. 1569/ स्टेनो/नग्रानि./2011, item 15); not defined
nalah (major drain)	The 2018 inspection form asks "Is Nalah (Major Drain) adjacent to plot or not" and its nearest distance (amendment of 6 October 2018, Appendix Q, item 9); not defined

Table 7E.4 — Soil and land classes of the Chhattisgarh plain

These four words are not in Platts or the Shabdasagara with these senses; their meanings are from agronomy papers on the plain (Mandal et al., 2018, for Durg district; Wakode et al., 2017, for a toposequence near Raipur), and the local names do not map one-to-one onto soil taxonomy. They are used in this Manual as land classes for the soil groups of Chapter 3 and the infiltration limits of Clause 8.8.1.

Term	Meaning in the agronomy papers	Dictionary check	Manual use
कन्हार (kanhār; kanhar)	Lowland deep black clay, more than 150 cm deep, slope under 2% (Vertisol); in a CGWB district report, "heavy soil with clay content of 40 to 50 percent … The permeability is generally low … locally known as Kanhar"	Platts's कन्हार is an unrelated word (p. 856); not in the Shabdasagara	HSG D; no infiltration facility (Clause 8.8.1); flood storage when in the 1% AEP extent (Clause 7.6)
डोरसा (ḍorsā; dorsa)	Lower-slope sandy clay loam to clay loam, 80 to 150 cm deep (Alfisol; vertic in places, and classed as a Vertisol near Raipur)	Not found	HSG C, or D where vertic or puddled paddy (Chapter 3)
मटासी (maṭāsī; matasi)	Mid-slope sandy loam to silt loam, 30 to 80 cm deep (Inceptisol)	Not found (the nearest headword, मटियार, is clayey land, Shabdasagara, p. 3746)	HSG B, or C where shallow over murram or puddled (Chapter 3)
भाठा (bhāṭhā; bhata)	Upland gravelly soil less than 30 cm deep over murram or ferricrete, dense (Entisol)	Shabdasagara: "१. दे० 'भाटा' । २. गर्त । गड्ढा । ३. पत्थर", and भाटा ३, "पथरीली भूमि", stony land (p. 3639); Platts भाटा, "= bhāṭhā" (p. 177)	HSG C, or D where rock or ferricrete is within 50 cm; never A (Chapter 3)

Table 7E.5 — Terms not yet verified

The following terms are in use for Chhattisgarh's waterbodies, but their meaning or regional use was not confirmed in Platts, the Shabdasagara or an official source during research for this Manual. They shall not be used in a record or a drawing as defined terms until verified.

Term	Reported meaning or use	Status
ढोढ़ी, ढोंढी (ḍhoṛhī, ḍhoṇḍhī; dhodi)	A small spring-fed pit or seepage pool	Not found in the Shabdasagara (four spellings) or in Platts; unsourced
narwa in Chhattisgarhi usage	A stream or seasonal watercourse, as in the Narwa–Garuwa–Ghuruwa–Bari programme	The dictionary sense (= nala) is in Table 7E.2; the Chhattisgarhi sense was not checked in a regional dictionary
dabri in Chhattisgarh's schemes	A farm or village pond dug under rural programmes	The dictionary sense is in Table 7E.1; the size and form in the schemes were not read
bandh as a city tank	An embanked tank, as in the names Maharajbandh and Telibandha	The dictionary sense (embankment, dam) is in Table 7E.2; the sense in names was not confirmed

Term	Reported meaning or use	Status
बूढ़ा तालाब, विवेकानंद सरोवर (Budha Talab, Vivekananda Sarovar)	The names of the large talab at Budhapara, Raipur	The official name was not confirmed
Chhattisgarhi forms of talab, nala and the soil classes	Regional words and spellings	Not checked in a Chhattisgarhi dictionary

The Manual uses the terms in the last column of Tables 7E.1 and 7E.2, the land classes of Table 7E.4, and the Chhattisgarh terms defined after Clause 7.2.

Form 7-1 — Waterbody Sheet

Supports Clauses 7.3.2, 7.4 and 7.15(1) of Chapter 7.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. One sheet per waterbody. Attach the plan of Clause 7.3.2 at 1:1,000 or larger.

Table 7F.1 — Form 7-1, Section A: Project and waterbody

No.	Field	Entry	Clause
A1	Project name and Authority file number	_____	—
A2	Waterbody name (English and Hindi) and inventory ID (Annex 7A), if any	_____	7.3.1
A3	Waterbody type (Clause 7.2)	_____	7.2
A4	Position: within the site / within 100 m of the site boundary / downstream within the Chapter 5 extent	_____	7.3.2
A5	Survey numbers and village	_____	7.3.1(a)
A6	Custodian	_____	7.3.1(b)
A7	Sources checked (tick): Revenue records (khasra) and village maps (LRC ss. 237, 248, 251), municipal tank and drain records, Development Plans and T&CP site reports and checklists (2011), WRD high flood points, Kopra Ramsar, wetland and State Wetland Authority boundaries, mining and closure plans, ash pond layouts □; WRD / Gram Panchayat registers □; Authority inventory or court list □; historical maps and imagery, including NWIA □; site survey □	_____	7.3.3
A8	Where sources conflict: which extent governs (the larger)	_____	7.3.3
A9	Datum of all levels, and conversion from any WRD datum	_____	7.3.5

Table 7F.2 — Form 7-1, Section B: Levels and features

No.	Field	Entry	Clause
B1	FTL (tanks, lakes, ponds) or top of bank (channels), m	_____	7.2
B2	MWL (tanks), m, and basis (WRD record or 1% AEP computed)	_____	7.2
B3	TBL, lowest point (tanks), m	_____	7.3.1(d)
B4	1% AEP flood level, m, and source (adopted map / hydraulic study / not available)	_____	7.3.4
B5	Drainage area at the site (channels), ha	_____	7.4.1(b)
B6	Supply channels, surplus courses, sluices and surplus weirs shown on plan (yes / no)	_____	7.3.2
B7	Known encroachments, with date recorded	_____	7.3.1(g)

Table 7F.3 — Form 7-1, Section C: Setback

No.	Field	Entry	Clause
C1	Basis of setback: mapped extent / hydraulic study / fixed width (Annex 7B)	_____	7.15(1)
C2	Is a hydraulic study required under Clause 7.3.4 (a) / (b) / (c)?	_____	7.3.4
C3	Floodway outer edge, m from FTL or top of bank, and how delineated	_____	7.6.1
C4	Zone 1 width, m (greatest of 15 m, floodway edge, MWL contour)	_____	7.4.3

No.	Field	Entry	Clause
C5	Zone 2 width, m (to 1% AEP extent + 10 m, or 10 m)	_____	7.4.4
C6	Wider statutory or approval setback, if any, and its source	_____	7.4.5
C7	Setback applied: Manual standard / a wider statutory or approval line: a distance under Rule 50 of the CGBVN 1984 once its Chhattisgarh text is confirmed (inherited 1984 text: 9 m from the average high flood mark or 15 m from the defined boundary of a major watercourse, whichever is more [TBC]) or as prescribed in a development plan; 50 m from the highest flood level of a talab or wetland that the Chhattisgarh State Wetland Authority treats as protected, or of a notified wetland; the boundary of Kopra Jalashay (Ramsar site); a WRD, DGMS (mine) or ash-dyke line; a court or Tribunal line; or other (state which)	_____	7.4, Disclosure
C8	Uses proposed in Zone 1 and Zone 2, and the custodian's NOC for any work in Zone 1	_____	7.5, 7.15(5)
C9	Fill proposed in the flood fringe (yes / no); if yes, Form 7-3 attached	_____	7.6.3
C10	Zone 1 marker positions shown (not more than 30 m apart)	_____	7.13.4

Table 7F.4 — Form 7-1, Section D: Prepared by

Field	Entry
Name, registration and empanelment number (Clause 10.6)	_____
Signature and date	_____
Licensed surveyor for the levels (name, licence number)	_____

Form 7-2 — Tank Storage Credit Sheet

Supports Clauses 7.7, 7.8, 7.11, 7.14 and 7.15(4) of Chapter 7.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. One sheet per tank whose storage is counted. Storage that fails any item in Section B shall not be counted (Clause 7.7.1).

Table 7F.5 — Form 7-2, Section A: Tank and scheme

No.	Field	Entry	Clause
A1	Tank name and inventory ID (Annex 7A); custodian	_____	7.3.1
A2	Scheme in which storage is counted: basin plan / regional facility / fee-in-lieu (S-5)	_____	7.7.1, 7.11
A3	Cascade modelled: uppermost tank _____ to outfall _____	_____	7.7.2
A4	Datum	_____	7.3.5

Table 7F.6 — Form 7-2, Section B: Conditions for counting storage

No.	Condition	Date / reference	Met (yes / no)	Clause
B1	FTL, MWL, TBL, weir crest and length, sluice sills surveyed within the previous 5 years	_____	_____	7.7.1(a)
B2	Stage–storage curve from a bathymetric and topographic survey within 5 years or after the last desilting, whichever is later	_____	_____	7.7.1(b)
B3	Documented operating rule agreed with the custodian (Annex 7C)	_____	_____	7.7.1(c), 7.8
B4	Weir, sluices and bund under an O&M plan; latest pre-monsoon inspection found them serviceable	_____	_____	7.7.1(d), 7.14.4
B5	Custodian's written concurrence	_____	_____	7.7.1(e)

Table 7F.7 — Form 7-2, Section C: Levels and creditable volume

No.	Item	Value	Clause
C1	Starting level: FTL, or cushion level under a rule meeting Clause 7.8.2, m	_____	7.7.3
C2	If a cushion is used: controllable outlet (type) and drawdown time, h	_____	7.8.2(a)
C3	If a cushion is used: reliability from continuous simulation, % of SW monsoon seasons (≥ 90%)	_____	7.8.2(c)
C4	Computed maximum level, 1% AEP inflow with climate factor routed through the cascade, m	_____	7.7.4
C5	MWL, m; is C4 ≤ MWL?	_____	7.7.4(a)
C6	Freeboard C4 to TBL, m; WRD freeboard standard, m; met?	_____	7.7.4(b)
C7	Storage between C1 and C4, from the stage–storage curve, m^3	_____	7.7.5
C8	Volume already committed to another facility or scheme, m^3	_____	7.7.5
C9	Creditable volume = C7 − C8, m^3	_____	7.7.5
C10	Register entry under Clause 10.17 (fee-in-lieu storage)	_____	7.11.2(d)

Table 7F.8 — Form 7-2, Section D: Bund safety

No.	Item	Entry	Clause
D1	Specified dam under the Dam Safety Act, 2021? Height, crest length, capacity, maximum flood discharge	_____	7.14.1
D2	Hazard class: high / low	_____	7.14.2
D3	High hazard: weir passes 1% AEP at MWL with freeboard; 0.2% AEP check without overtopping	_____	7.14.3(a)
D4	High hazard: breach analysis and emergency action plan (references)	_____	7.14.3(b), (c)
D5	High hazard: independent check under Clause 10.6 (name)	_____	7.14.3(d)

Table 7F.9 — Form 7-2, Section E: Signatures

Role	Name and designation	Signature and date
Designer (Clause 10.6)	_____	_____
Independent checker, where Clause 7.14.3(d) applies	_____	_____
Custodian's concurrence (Clause 7.7.1(e))	_____	_____

Form 7-3 — Compensatory Storage Calculation (by elevation band)

Supports Clauses 7.6.3 to 7.6.5, 7.9.5 and 7.15(3) of Chapter 7.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

Method

1. Fix the **lower limit**: the normal water level of the waterbody, or the 50% AEP (2-yr) level where higher (Clause 7.6.4(b)).
2. Fix the **upper limit**: the 1% AEP flood level (with the climate factor).
3. Divide the range into **bands of not more than 0.3 m**. The top band may be thinner.
4. For each level z at a band boundary, measure on plan:
    - A_fill(z): the area of the fill footprint where existing ground is below z; and
    - A_cut(z): the area of the excavation where finished ground is below z and existing ground was above z (new flooded area).
5. For each band, by the average end-area method: V = (A(z_bottom) + A(z_top)) / 2 × band thickness. Do this for fill (volume lost) and for excavation (volume gained).
6. **Check each band:** volume gained ≥ volume lost. A surplus in one band does not make up a shortfall in another (Clause 7.6.4(b)).
7. Check the other conditions of Clause 7.6.4(c) to (f) in Section C.

Figure 7.2 in Chapter 7 shows the method.

Table 7F.10 — Form 7-3, Section A: Levels

No.	Field	Entry	Clause
A1	Waterbody, reach, and inventory ID	_____	7.6.4(c)
A2	Normal water level, m; 50% AEP level, m; lower limit (the higher), m	_____	7.6.4(b)
A3	1% AEP flood level with climate factor, m (upper limit)	_____	7.6.4(b)
A4	Band thickness, m (≤ 0.3) and number of bands	_____	7.6.4(b)
A5	Datum	_____	7.3.5

Table 7F.11 — Form 7-3, Section B: Band-by-band balance

Band	From (m)	To (m)	A_fill bottom (m²)	A_fill top (m²)	Volume lost (m³)	A_cut bottom (m²)	A_cut top (m²)	Volume gained (m³)	Gained ≥ lost?
1	_____	_____	_____	_____	_____	_____	_____	_____	_____
2	_____	_____	_____	_____	_____	_____	_____	_____	_____
3	_____	_____	_____	_____	_____	_____	_____	_____	_____
4	_____	_____	_____	_____	_____	_____	_____	_____	_____
Total					_____			_____	

Add rows as needed.

Table 7F.12 — Form 7-3, Section C: Other conditions

No.	Condition	Met (yes / no) and evidence	Clause
C1	Fill is in the flood fringe, not the floodway, and for a permitted use	_____	7.6.2, 7.6.3
C2	Excavation is in the same reach of the same waterbody, hydraulically connected, and not further downstream than the fill	_____	7.6.4(c)
C3	Excavation drains by gravity as the flood recedes; no pumping; no permanent pool below normal water level	_____	7.6.4(d)
C4	Excavation built before, or at the same time as, the fill	_____	7.6.4(e)
C5	Covenant or handover so that it is never filled (reference)	_____	7.6.4(f)
C6	Not counted as detention or retention under Chapter 4	_____	7.6.5
C7	Signature of designer (Clause 10.6) and date	_____	10.6

Worked example

Illustrative — not design values. A site in the flood fringe of a channel needs a fill pad for a permitted Zone 2 use (Clause 7.5.2). The inputs are assumed.

- Normal water level 10.00 m; 50% AEP level 9.80 m. Lower limit = 10.00 m (the higher).
- 1% AEP flood level 11.20 m. Range 1.20 m, so four bands of 0.30 m.
- The fill footprint is 1,600 m². Existing ground across the footprint rises evenly from 10.00 m to 11.20 m, so the area of footprint below each level is: 0 m² at 10.00 m; 400 m² at 10.30 m; 800 m² at 10.60 m; 1,200 m² at 10.90 m; 1,600 m² at 11.20 m.
- The excavation is a graded cut beside the channel, with its floor at 10.00 m. The new flooded area below each level (first trial) is: 300 m² at 10.00 m; 900 m² at 10.30 m; 1,100 m² at 10.60 m; 1,500 m² at 10.90 m; 1,500 m² at 11.20 m.

Step 1 — volume lost, band by band (V = average area × 0.3 m):

- Band 1: (0 + 400) / 2 × 0.3 = 60 m³
- Band 2: (400 + 800) / 2 × 0.3 = 180 m³
- Band 3: (800 + 1,200) / 2 × 0.3 = 300 m³
- Band 4: (1,200 + 1,600) / 2 × 0.3 = 420 m³
- Total: 960 m³

Step 2 — volume gained, first trial:

Table 7F.13 — Worked example: first trial (illustrative)

Band	From (m)	To (m)	Volume lost (m³)	A_cut bottom (m²)	A_cut top (m²)	Volume gained (m³)	Gained ≥ lost?
1	10.00	10.30	60	300	900	180	Yes
2	10.30	10.60	180	900	1,100	300	Yes
3	10.60	10.90	300	1,100	1,500	390	Yes
4	10.90	11.20	420	1,500	1,500	450	Yes
Total			960			1,320	

Every band passes. Note that a simple total balance would hide a failure if, for example, the cut were made deeper and narrower: suppose the designer instead offers a cut with new flooded areas of 1,400 m² at 10.00 m, 1,400 m² at 10.30 m, 1,000 m² at 10.60 m, 600 m² at 10.90 m and 200 m² at 11.20 m.

Table 7F.14 — Worked example: deeper, narrower cut (illustrative)

Band	From (m)	To (m)	Volume lost (m³)	A_cut bottom (m²)	A_cut top (m²)	Volume gained (m³)	Gained ≥ lost?
1	10.00	10.30	60	1,400	1,400	420	Yes
2	10.30	10.60	180	1,400	1,000	360	Yes
3	10.60	10.90	300	1,000	600	240	**No**
4	10.90	11.20	420	600	200	120	**No**
Total			960			1,140	

The total gained (1,140 m³) exceeds the total lost (960 m³), but bands 3 and 4 are short by 60 m³ and 300 m³. The surplus low down cannot be counted against them: the low storage fills early in every flood and is not available when the fill takes away storage near the peak. This design fails Clause 7.6.4(b) and must be regraded so that the upper bands gain at least as much as they lose, as in the first trial.

ANNEXES TO CHAPTER 8

Water Quality and Recharge

These annexes support Chapter 8. They give the layout of the water quality rainfall tables, the Level of Service and Value Rating method in detail, the layout of the BMP calibration programme, design sheets for each BMP, standard details for construction erosion and sediment control, and the two forms that Clause 8.14 requires. Annexes 8A and 8C need data that are not yet available; they say so and give the rule to use in the meantime. The forms are draft forms.

Annex 8A — 3-month ARI 24-hour depth and IDF by zone

Supports Clauses 8.2, 8.4.1 and 8.4.5 of Chapter 8.

To be prepared. This annex will contain, for each rainfall zone of Chapter 2, the 24-hour 3-month ARI depth P_3m and the 3-month ARI intensity–duration relationship used for Q_wq. It needs long-term daily and sub-daily rainfall records (IMD station and gridded data, with NIH as the Chapter 2 primary source), analysed as a partial-duration series. Until it is published, the designer shall compute P_3m from the longest available daily record for the site's zone, as the 24-hour depth equalled or exceeded on average four times a year (Clause 8.2), state the record, its length and the method on the Water Quality Sheet (Form 8-1), and use P_wq = max (P_ret, P_3m) (Clause 8.4.1). The climate factor is not applied (Clause 8.4.3).

8A.1 Method to be used in preparing the annex

- (a) Use daily records of at least 30 years where available (as for station IDF in Chapter 2).
- (b) Form the partial-duration series of 24-hour depths, with independent events separated by at least one dry day.
- (c) Choose the threshold so that the number of events above it averages four per year; P_3m is that threshold.
- (d) Derive the 3-month ARI intensities for durations from 10 minutes to 24 hours from sub-daily records, or by the Chapter 2 method of scaling from the daily value, stating which.
- (e) Do not apply the climate factor (Clause 8.4.3).

Table 8A.1 — Water quality rainfall depths by zone (layout)

Zone (Chapter 2)	Representative stations or grid	Record (years)	P_ret, Annex 2E (mm)	P_3m (mm)	P_wq = max (mm)
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____

Table 8A.2 — 3-month ARI intensity by duration (layout)

Zone	10 min (mm/h)	15 min (mm/h)	30 min (mm/h)	1 h (mm/h)	2 h (mm/h)	6 h (mm/h)	24 h (mm/h)
_____	_____	_____	_____	_____	_____	_____	_____

Zone	10 min (mm/h)	15 min (mm/h)	30 min (mm/h)	1 h (mm/h)	2 h (mm/h)	6 h (mm/h)	24 h (mm/h)
_____	_____	_____	_____	_____	_____	_____	_____

Annex 8B — Redevelopment LoS table and treatment-train composite VR

Supports Clauses 8.6.2 to 8.6.5 of Chapter 8.

8B.1 Required Level of Service for redevelopment

To be prepared. This part will give the required Level of Service for redevelopment sites (existing impervious cover of 20% or more of the site, Clause 8.6.2). The MARC/APWA BMP Manual (2012, Table 4.3) sets the redevelopment LoS from the percentage of impervious cover within the disturbed area; the Chhattisgarh values need to be set by the State Stormwater Technical Committee and checked against local redevelopment cases. Until it is published, the required LoS for a redevelopment site is **6** for the disturbed area. Where Table 8.2, applied to the change in CN from the existing condition to post-development, gives a higher LoS, that higher LoS applies. This interim value is set by this Manual. A redevelopment site that retains its whole WQv on site under Chapter 4 needs no further treatment for that area, apart from pretreatment, hotspot controls and the outfall requirements (Clause 8.4.4).

Table 8B.1 — Required LoS for redevelopment (layout)

Existing impervious cover within the disturbed area (%)	Change in impervious cover	Required LoS
_____	_____	_____
_____	_____	_____
_____	_____	_____

8B.2 Composite Value Rating: area-weighted method

The composite VR of a site is the area-weighted average of the VRs of the BMPs serving each part of the site (Clause 8.6.3):

$$VR_{site} = \Sigma (A_i \times VR_i) / \Sigma A_i + VR_{signage}$$

where:

- A_i = the area of part i of the site, ha;
- VR_i = the Value Rating (Table 8.3) of the BMP serving part i, which counts only if the BMP meets Clause 8.6.4 for the whole of A_i;
- ΣA_i = the whole site area: every part of the site is assigned to one BMP, or to preserved native vegetation; a part served by no BMP is entered with $VR_i = 0$; and
- $VR_{signage} = 0.25$ where public education signage is provided (Table 8.3), otherwise 0.

The site complies where $VR_{site} \geq$ required LoS (Clause 8.6.3).

Illustrative — not design values. A 1.0 ha site that is not redevelopment, with ΔCN = 18. Required LoS = 8 (Table 8.2).

Table 8B.2 — Worked example: composite VR (illustrative)

Part	Area A_i (ha)	BMP	VR_i (Table 8.3)	A_i × VR_i
Roofs and paving draining to bioretention	0.60	Bioretention	8.5	5.100
Car park	0.25	Permeable pavement	7.5	1.875
Preserved strip along boundary	0.15	Native vegetation (preserved)	9.25	1.388
Total	1.00			8.363

- Area-weighted VR = 8.363 / 1.00 = 8.36
- Signage provided: + 0.25
- VR_site = 8.61 ≥ 8. The site complies.

Without the permeable pavement (car park draining untreated, VR = 0): (5.100 + 0 + 1.388) / 1.00 + 0.25 = 6.74 < 8. The site would not comply.

8B.3 Treatment trains

To be prepared. This part will give the composite VR of BMPs in series (a treatment train), following the approach of the MARC/APWA BMP Manual (2012, Table 4.6), with values to be confirmed for Chhattisgarh by the calibration programme of Clause 8.13.4 (Annex 8C). Until it is published, each part of the site is credited with the VR of one BMP in the train that meets Clause 8.6.4 for that part, normally the last BMP before discharge, and no extra credit is given for BMPs in series. Pretreatment devices, silt traps and oil-grit separators upstream of the credited BMP earn no VR of their own (Clauses 8.6.4(c), 8.7.11(c) and 8.7.12(c)).

Table 8B.3 — Composite VR of treatment trains (layout)

First BMP	Second BMP	Composite VR	Conditions
_____	_____	_____	_____
_____	_____	_____	_____

Annex 8C — Chhattisgarh BMP performance calibration

Supports Clauses 8.6.3, 8.13 and 8.15.1 of Chapter 8.

To be prepared. This annex will report the BMP performance calibration programme of the State Stormwater Technical Committee (Clause 8.13.4): measured inflow and outflow concentrations and volumes for BMPs built in Chhattisgarh, and the revised Value Ratings that follow from them. It needs wet-weather monitoring of BMPs over several SW monsoon seasons, with laboratory analysis to the CPCB methods, and the outfall monitoring of Clause 8.13.1 published under Clause 10.17. Until it is published, the Value Ratings of Table 8.3 (MARC/APWA 2012) apply, and a proprietary device earns a VR only from verified test data (Table 8.3).

The layouts below set the minimum records the programme shall keep, so that results from different authorities can be pooled.

Table 8C.1 — Monitoring site register (layout)

Site ID	BMP type (Table 8.3)	Location and authority	Catchment area (ha)	Impervious (%)	Soil and groundwater setting	Date commissioned	Pretreatment
_____	_____	_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____	_____	_____

Table 8C.2 — Event record (layout; parameters of Clause 8.13.2)

Site ID	Event date	Rainfall (mm)	Inflow volume (m^3)	Outflow volume (m^3)	TSS in / out (mg/L)	BOD in / out (mg/L)	COD in / out (mg/L)	Faecal coliform in / out (MPN/100 mL)	TN in / out (mg/L)	TP in / out (mg/L)	Oil and grease in / out (mg/L)	Heavy metals (industrial catchments)
_____	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____

Table 8C.3 — Performance summary and proposed VR (layout)

BMP type	Sites	Events	Volume reduction (%)	Median effluent TSS (mg/L)	Other findings	VR in Table 8.3	Proposed VR
_____	_____	_____	_____	_____	_____	_____	_____

Annex 8D — BMP design sheets, media specification, plant lists

Supports Clauses 8.5, 8.7, 8.8 and 8.11 of Chapter 8.

Each design sheet collects the requirements of Chapter 8 for one BMP. Values marked "recommended starting value" in Clause 8.7.1 may be departed from with justification. Values that the Manual does not set are marked "to be confirmed in design"; the designer shall state and justify them. Every BMP also needs: sizing for the WQv or Q_wq (Clause 8.6.4(a)), pretreatment (Clause 8.5.3), a safe bypass for flows above Q_wq (Clause 8.4.6) and maintenance access (Chapter 10).

8D.1 Design sheets

Table 8D.1 — Pretreatment (all BMPs)

Item	Requirement	Clause
Where needed	Every bioretention cell, infiltration facility, permeable pavement receiving run-on, wet pond and constructed wetland, at each concentrated inflow	8.5.3
Types	Sediment forebay, grass filter strip, silt trap or proprietary device	8.5.3
Size	Holds at least 10% of the WQv of its contributing area; or sized for Q_wq (flow-through devices)	8.5.3
Access	Cleanable without entering or disturbing the main BMP	8.5.4
Silt traps and catch pits	At every inlet receiving runoff from unpaved ground, and at the inlet of every drain discharging to a BMP or waterbody; sump at least 300 mm below the outlet invert; earn no VR	8.7.12

Table 8D.2 — Bioretention and rain gardens (Figure 8.2)

Item	Requirement	Clause
Ponding depth	≤ 300 mm above the mulch, with an overflow at maximum ponding level	8.7.2(a)
Ponding drawdown	Within 48 h	8.7.2(b)
Filter media	600–900 mm engineered sandy loam, low fines, low phosphorus (Table 8D.12)	8.7.2(c)
Mulch	50–75 mm	8.7.2(c)
Transition (choker) and gravel drainage layers	Required; thickness and grading to be confirmed in design	8.7.2(d)
Underdrain	Perforated; required where the design infiltration rate cannot meet the 48 h drawdown, or where lined; diameter to be confirmed in design	8.7.2(e)
Lining	Required where Clause 8.8 cannot be met, and at hotspots	8.7.2(f)
Plants	Native or naturalised, tolerant of waterlogging and long dry spells (Tables 8D.13 and 8D.14)	8.7.2(g)
VR	8.5 (rain garden / infiltration 9.0)	Table 8.3

Table 8D.3 — Vegetated swales

Item	Requirement	Clause
Section	Trapezoidal or parabolic; side slopes no steeper than 1V:3H	8.7.3(a)
Longitudinal slope	0.5% to 4%; check dams where steeper	8.7.3(b)
Flow at Q_wq	Depth not more than grass height; velocity ≤ 0.3 m/s	8.7.3(c)
1% AEP or bypass flow	No erosion (Chapter 9 permissible velocities)	8.7.3(d)
Underdrained ("dry") swale	Where slope < 1% or subsoil is poorly drained; filter media as Table 8D.12	8.7.3(e)
VR	4.0 (native vegetation swale)	Table 8.3

Table 8D.4 — Vegetated filter strips

Item	Requirement	Clause
Inflow	Sheet flow only; level spreader where flow arrives concentrated	8.7.4(a)
Length in flow direction	≥ 5 m	8.7.4(b)
Slope	1% to 8%	8.7.4(b)
Use	Mainly pretreatment for bioretention, infiltration and swales	8.7.4(c)
VR	5.0	Table 8.3

Table 8D.5 — Permeable pavement

Item	Requirement	Clause
Run-on	Impervious run-on area ≤ 2 × permeable area; none from unpaved or eroding areas	8.7.5(a)
Base reservoir	Clean open-graded aggregate holding its share of the WQv in its voids; void ratio confirmed by test	8.7.5(b)
Open (infiltrating) base	Only where Clause 8.8 is met; otherwise lined with underdrain and counted as filtration only	8.7.5(c)

Item	Requirement	Clause
Structure	Designed for traffic loading to the road authority's standards	8.7.5(d)
Exclusions	Not at hotspots; not where it will be resurfaced with impermeable material	8.7.5(e)
VR	7.5	Table 8.3

Table 8D.6 — Infiltration trenches and basins (Figure 8.3)

Item	Requirement	Clause
Pretreatment	Always (Table 8D.1)	8.7.6(a)
Fill	Clean washed aggregate, void ratio confirmed by test; geotextile on sides and top; sand base	8.7.6(b)
Observation well	To the base of each trench	8.7.6(c)
Storage	WQv (or Chapter 4 retention volume, if greater) held in the voids	8.7.6(d)
Drawdown	Empty within 72 h at the design infiltration rate	8.7.6(d), 8.8.7
Overflow	At grade, to the minor or major system	8.7.6(e)
Commissioning	Not in service until the contributing area is stabilised	8.7.6(f)
Criteria	All of Clause 8.8 (Table 8D.11)	8.8

Table 8D.7 — Wet ponds (extended wet detention)

Item	Requirement	Clause
Permanent pool	≥ WQv	8.7.7(a)
Extended detention	WQv above the pool, released over 24–48 h	8.7.7(a), 8.4.7
Forebay	At each inlet	8.7.7(b)
Flow path	Length-to-width ≥ 3:1; no short-circuiting	8.7.7(c)
Pool depth	1.0–2.5 m, with safety bench (Clause 7.9.4)	8.7.7(d)
Water balance	Monthly balance with IMD rainfall and pan evaporation: ≥ 50% of pool volume kept through the driest year in 10; no top-up with groundwater, potable water or sewage	8.7.8
Flood detention	Chapter 4 detention may sit above the extended detention level	8.7.7(f)
VR	5.0	Table 8.3

Table 8D.8 — Constructed wetlands

Item	Requirement	Clause
Layout	Inlet sedimentation pond (forebay), then shallow macrophyte zone	8.7.9(a)
Extended detention	WQv above normal water level, released over 24–48 h; depth ≤ 0.5 m	8.7.9(b)
Flow path	Length-to-width ≥ 3:1; deep zones across the flow path	8.7.9(c)
Bypass	High-flow bypass so that floods do not scour the macrophyte zone	8.7.9(d)
At a tank inlet	Within the tank's setback, outside its waterspread; not counted as tank storage	8.7.9(e)
Water balance	As wet ponds (minimum wetland depth kept)	8.7.8

Item	Requirement	Clause
VR	7.0 (extended detention wetland)	Table 8.3

Table 8D.9 — Gross pollutant traps and outfall screens

Item	Requirement	Clause
Treatment flow	Q_wq; all flows up to 1% AEP pass without washing out captured material	8.7.10(a)
Capture size	Down to about 5 mm	8.7.10(b)
Head loss	In the drain HGL (Chapter 9), screen 50% blocked	8.7.10(c), 8.10.3(a)
Storage	For the O&M cleaning interval; not less than 3-monthly cleaning	8.7.10(d)
Access	Vehicle access for suction or grab cleaning; safe cleaning platform	8.7.10(e), 8.10.3(d)
Blocked screen	Overflow or bypass path so that a fully blocked screen does not flood property	8.10.3(b)
Bar spacing	Chosen for the hazard: closer for litter capture, wider where blockage would cause flooding; to be confirmed in design	8.10.3(c)

Table 8D.10 — Oil-grit separators

Item	Requirement	Clause
Where	Fuel stations; vehicle workshops and washing bays; bus and truck depots; parking areas of more than 50 spaces	8.7.11(a)
Sizing	Q_wq, with high-flow bypass	8.7.11(b)
Not alone	Followed by a filtering BMP where the site needs a VR above 5; lined filtering BMP before any infiltration at hotspots	8.7.11(c), 8.8.8

Table 8D.11 — Infiltration and recharge criteria (all infiltrating BMPs)

Item	Requirement	Clause
Groundwater separation	Seasonal high groundwater ≥ 1.0 m below base (roof-only runoff in notified coastal sand: ≥ 0.5 m with mounding check, mound ≥ 0.3 m below base)	8.8.1, 8.8.9(d)
Bedrock or impermeable layer	Base ≥ 1.0 m above, unless designed for rock-aquifer recharge under Clause 8.8.10	8.8.2
Tests	Double-ring (ASTM D3385) for basins, bioretention, permeable pavement; falling-head borehole or trial-pit soakage (e.g. BRE Digest 365) for trenches, soakaways, pits, wells; ≥ 2 per facility and ≥ 1 per 100 m² of base or per 50 m of trench; Form 8-2	8.8.3
Design rate	Measured rate ÷ 2.0 (tests meet 8.8.3(c) and vary by no more than a factor of 2), otherwise ÷ 3.0	8.8.4
Minimum rates	≥ 13 mm/h full infiltration; 3–13 mm/h partial, with underdrain, credit only for volume infiltrated within 72 h; < 3 mm/h no credit	8.8.5
Setbacks (runoff from paved surfaces)	Foundations 3 m, basements 5 m; property boundary 1.5 m; drinking-water well or borewell 30 m; septic tank or soak pit 15 m; crest of slope steeper than 1V:3H 6 m	8.8.6
Expansive soils	Foundation setback doubled	8.8.11(b)

Item	Requirement	Clause
Drawdown	Empty within 72 h at design rate; tested after construction	8.8.7
Hotspots	No infiltration without treatment; none at all for waste sites and contaminated land	8.8.8
Road runoff	Never led directly into a well, borewell or recharge shaft	8.11.3(b)

8D.2 Bioretention drawdown check

The 48-hour ponding drawdown (Clause 8.7.2(b)) and the minimum rates of Clause 8.8.5 together decide whether a cell needs an underdrain.

- Rate needed to drain the maximum ponding depth in 48 h = 300 mm / 48 h = 6.25 mm/h.
- Design rate ≥ 13 mm/h: full infiltration, no underdrain, provided Clause 8.8 is met.
- Design rate from 6.25 mm/h to 13 mm/h: the ponding drains in time, but Clause 8.8.5(b) applies, so an underdrain is required.
- Design rate below 6.25 mm/h: the ponding cannot drain in 48 h by infiltration; an underdrain is required (Clause 8.7.2(e)), and below 3 mm/h no infiltration credit is given.

Illustrative — not design values. Two tests at the base of a proposed cell give 30 mm/h and 22 mm/h. The two results differ by a factor of 30 / 22 = 1.36, which is not more than 2, and the number of tests meets Clause 8.8.3(c), so the safety factor is 2.0. Taking the lower result, the design rate = 22 / 2.0 = 11 mm/h. This is above 6.25 mm/h, so the ponding drains in 300 / 11 = 27 h, within 48 h. It is below 13 mm/h, so the cell is a partial-infiltration design with an underdrain (Clause 8.8.5(b)).

8D.3 Filter media specification

Table 8D.12 — Bioretention filter media and layers

Layer	Requirement from Chapter 8	Values to be confirmed in design
Mulch	50–75 mm (Clause 8.7.2(c)); shredded hardwood or similar organic mulch that does not float away at the ponding depth	Material and source
Filter media	600–900 mm engineered sandy loam with low fines and low phosphorus content (Clause 8.7.2(c))	Particle size grading; fines (silt + clay) limit; organic matter content; phosphorus limit; saturated hydraulic conductivity of the compacted media; source and test certificate for each batch
Transition (choker) layer	Between media and gravel, to stop media washing into the gravel (Clause 8.7.2(d))	Thickness and grading, matched to the media and gravel
Gravel drainage layer	Below the transition layer, surrounding the underdrain (Clause 8.7.2(d))	Thickness, grading, washed
Underdrain	Perforated pipe where Clause 8.7.2(e) applies, with cleanouts	Diameter, slope, perforation, outlet level
Liner	Where Clause 8.7.2(f) applies (cannot meet Clause 8.8; hotspots)	Liner type and protection

Media shall be tested before placing, and placed in lifts without heavy compaction. The State Stormwater Technical Committee should set numeric limits for grading, fines, organic matter and phosphorus, with local sources of sand and compost, when it reviews this annex.

8D.4 Plant lists

The lists below are **indicative**. They name species that are native or long naturalised in central India and known to tolerate both wet and dry periods, or, where marked, dry upland only; no Chhattisgarh source on planting for stormwater was researched for this Manual. Hindi or Urdu names are given only where J. T. Platts, *A Dictionary of Urdu, Classical Hindi, and English* (1884, as served by the Digital South Asia Library), gives the name for the species or plant named, with its page; a dash means none was confirmed. Platts often uses older botanical names, which are given in the notes. No Chhattisgarhi name was checked. Final selection shall be confirmed with a horticulturist or the State Forest Department for the site's soil and water table. Sites on kanhar and dorsa lowland and at talab edges need species tolerant of heavy, cracking clay that stands in water in the monsoon and is hard and dry after it; sites on bhata and matasi uplands and on laterite species tolerant of dry, thin soils over murram or rock; and the toes of overburden dumps, the banks of sediment basins and the margins of ash ponds species that bind loose, poor spoil.

Table 8D.13 — Indicative plants for bioretention, swales and filter strips

Use	Botanical name	Hindi / Urdu name	Notes
Swales, filter strips, bioretention edges, dump toes	*Chrysopogon zizanioides* (vetiver)	ख़स / خس (khas)	Deep-rooted clump grass; tolerates flooding and drought; binds soil on side slopes, talab banks and the toes of overburden dumps. Platts: "A fragrant grass, *Andropogon muricatum* (from the roots of which *taṭṭīs* are made)" (p. 489), an older name of this species
Swales and filter strips (turf)	*Cynodon dactylon* (Bermuda grass)	दूब / دوب (dūb)	Low turf; tolerates drought, traffic and short flooding. Platts: "Bent grass, panic grass, *Panicum dactylon*, or *Cynodon dactylon*" (p. 531)
Swales and banks on floodplain sites	*Saccharum spontaneum*	कास / کاس (kās)	Native tall grass of river sands; binds banks; cut back where it would obstruct a channel. Platts: "A species of grass, of which ropes, &c. are made, *Saccharum spontaneum*" (p. 801)
Trees beside bioretention and swales	*Syzygium cumini*	जामुन, जामन / جامن (jāmun, jāman)	Native; tolerates wet ground. Platts: "The tree *Eugenia jambelana*, or *Syzygium jambolanum*, and its fruit" (p. 372)
Trees beside bioretention and swales	*Millettia pinnata* (syn. *Pongamia pinnata*)	करंज / کرنج (karanj)	Native; tolerates waterlogging and drought. Platts names *Galedupa arborea*, an older name of this species, but also the fever-nut *Caesalpinia bonducella* under the same word (p. 828); confirm the species with the supplier. Keep roots clear of underdrains
Trees beside bioretention, swales and channels	*Terminalia arjuna*	अर्जुन / ارجن (arjun)	Native riverbank tree; tolerates seasonal flooding. Platts: "a certain tree, *Terminalia alata glabra*" (p. 39); that the name refers to this species was not confirmed in Platts
Trees beside wet ponds and on lowland sites	*Neolamarckia cadamba* (syn. *Anthocephalus cadamba*)	कदम्ब / کدمب (kadamb)	Native; tolerates wet ground. Platts: "The tree *Nauclea cadamba*" (p. 821), an older name of this species; Platts gives the Sanskrit form, and the spoken form kadam was not checked
Trees on bhata and laterite margins, swale banks and dump toes	*Butea monosperma* (palash)	पलाश / پلاش (palāś); ढाक / ڈھاك (ḍhāk)	Native; tolerates poor, hard and seasonally wet ground. Platts: "the tree *Butea frondosa*" (p. 267) and "The *Butea frondosa* (syn. *palās*…)" (p. 569), an older name of this species
Trees on dry upland edges only (not in the wet zone)	*Shorea robusta* (sal)	साल / سال (sāl)	Native forest tree; does not tolerate waterlogging, so planted only on the upland edges of swales and filter strips and on banks above the 50% AEP level, never on kanhar. Platts: "The saul tree, *Shorea robusta*, and its wood" (p. 626)

Use	Botanical name	Hindi / Urdu name	Notes
Trees on dry upland edges only (not in the wet zone)	*Madhuca longifolia* var. *latifolia* (mahua)	महुआ / مہوا (mahu'ā)	Native; drought-tolerant; planted on dry banks and upland margins, not in a bioretention cell. Platts: "The tree *Bassia latifolia*, and its flower" (p. 1101), an older name of this species
Trees beside channels on floodplain sites	*Dalbergia sissoo*	शीशम / شیشم (śīśam)	Riverine tree, widely planted. Platts: "The tree *Dalbergia sisu*, and its wood" (p. 740)
Trees beside wet ponds and channels	*Ficus racemosa* (syn. *F. glomerata*)	गूलर / گولر (gūlar)	Native fig of stream banks. Platts: "The wild fig, *Ficus glomerata*" (p. 926). Keep roots clear of underdrains, pipes and walls
Bioretention (flowering, wet zones)	*Canna indica*	—	Naturalised; tolerates waterlogging; widely used in constructed wetlands in India

Table 8D.14 — Indicative plants for constructed wetlands and wet ponds

Zone	Botanical name	Hindi / Urdu name	Notes
Shallow macrophyte zone	*Typha* spp. (e.g. *T. domingensis*, syn. *T. angustata*)	—	Native; spreads quickly and needs periodic harvesting. No Hindi name was confirmed in Platts: पटेर (paṭer), sometimes used for it, is glossed by Platts as *Cyperus* (p. 229)
Shallow macrophyte zone	*Phragmites karka*	नल / نل (nal); नरसल / نرسل (narsal)	Native reed; vigorous; keep out of nalas and channels where it would obstruct flow. Platts: "A species of reed, *Amphidonax karka*, or *Arundo karka*" (p. 1152) and "A large species of reed, *Arundo karka*" (p. 1132)
Shallow macrophyte zone	*Canna indica*	—	As Table 8D.13
Open water and deep zones	*Nelumbo nucifera* (lotus)	कमल / کمل (kamal)	Native; for permanent pools only. Platts: "The lotus, *Nelumbium speciosum*" (p. 849)
Open water and deep zones	*Nymphaea nouchali* (blue water lily)	नीलोफ़र / نیلوفر (nīlofar)	Native; for permanent pools only. Platts: "the blue water-lily, *Nymphæa cyanea*" (p. 1168), an older name of this species

Table 8D.15 — Species not to be planted

Botanical name	Hindi / Urdu name	Reason
Eichhornia crassipes (water hyacinth)	—	Invasive; chokes talabs, lakes, nalas and screens; at Bhilai, water hyacinth from the township drains blocked the culverts at Junwani and Kosanala in 2025 (as reported)
Prosopis juliflora	—	Invasive on pond beds, embankments, drain banks and mine spoil
Ipomoea carnea	—	Invasive in ponds and channels; obstructs flow
Lantana camara	—	Invasive on forest edges and mine land; not for swales or banks

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Annex 8E — Erosion and sediment control standard details

Supports Clause 8.12 of Chapter 8.

The details below show the arrangement of each measure. Dimensions set by the Manual are given. Dimensions the Manual does not set are labelled "to be confirmed in design" and shall be shown on the ESC plan (Clause 8.12.1). Temporary measures are designed with the rainfall of Chapter 2 and a climate factor of 1.0 (Clause 8.12.4).

Table 8E.1 — Selection of measures (Clause 8.12.3)

Situation	Measure	Detail
Sheet flow leaving a disturbed area, along its downslope edge	Silt fence	Figure 8E.1
Concentrated flow from a disturbed area of up to 2 ha	Sediment trap	Figure 8E.2
Disturbed area of more than 2 ha	Sediment basin with surface dewatering outlet	Figure 8E.2
Existing drain inlet receiving site runoff	Inlet protection	Figure 8E.4
Vehicle exit to a public road	Stabilised construction exit, with wheel washing	Figure 8E.3
Stockpiles	Outside Zone 1 of any waterbody setback; ≥ 10 m from any drain; covered in the SW monsoon	Notes to Figure 8E.1
Dewatering	Through a sediment basin, settling tank or filter bag; never pumped directly to a drain or waterbody	Notes to Figure 8E.2
Concrete washout	Lined pit; never into a drain	Notes to Figure 8E.3

8E.1 Silt fence

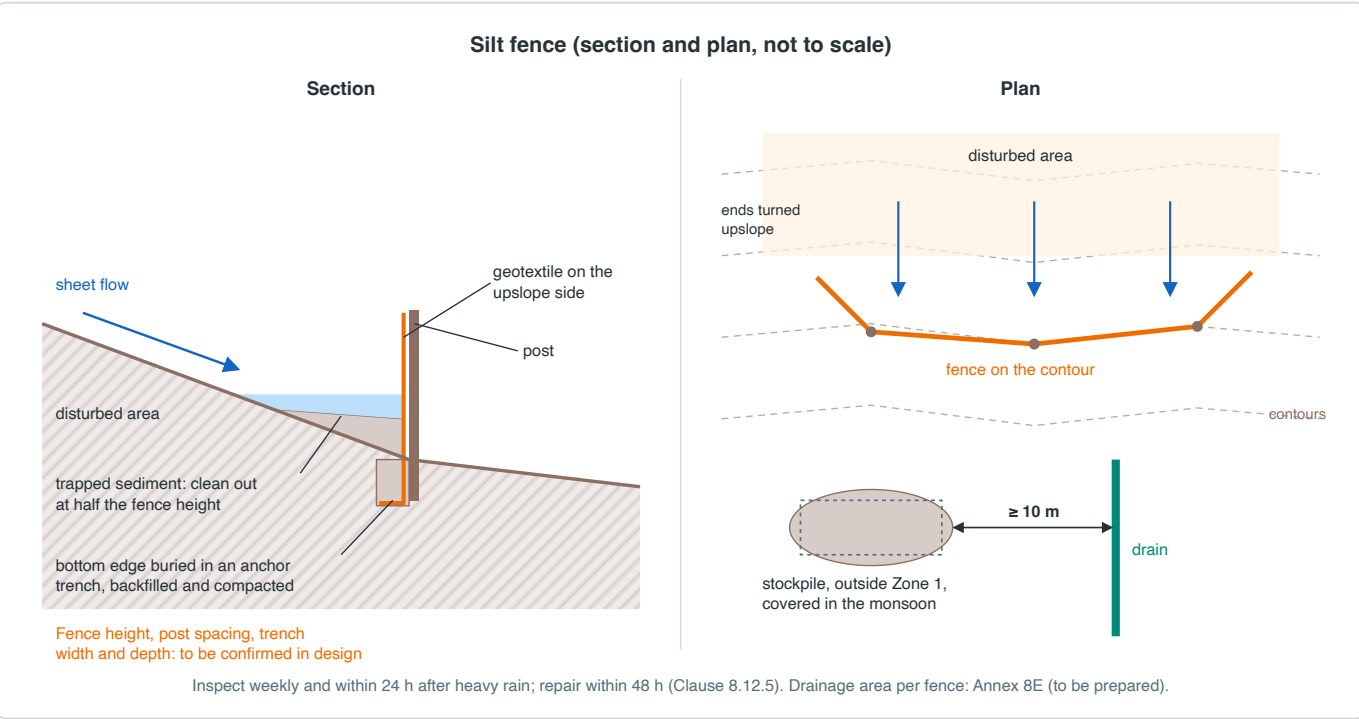


Figure 8E.1 — Silt fence: section showing geotextile on posts with the bottom edge buried in an anchor trench on the upslope side, and plan showing the fence on the contour with returns at the ends. Dimensions not set by the Manual are to be confirmed in design. Original drawing for this Manual, adapted from US EPA, NPDES Construction General Permit (2022), and CIRIA, The SuDS Manual (C753), 2015.

To be prepared. Clause 8.12.3(a) refers to this annex for the limit on the area draining to each silt fence. That limit, and the maximum slope length above a fence, need to be set by the State Stormwater Technical Committee for Chhattisgarh rainfall. Until they are published, the following interim limits apply, taken from common US practice (US EPA, Stormwater Best Management Practice fact sheet: Silt Fence):

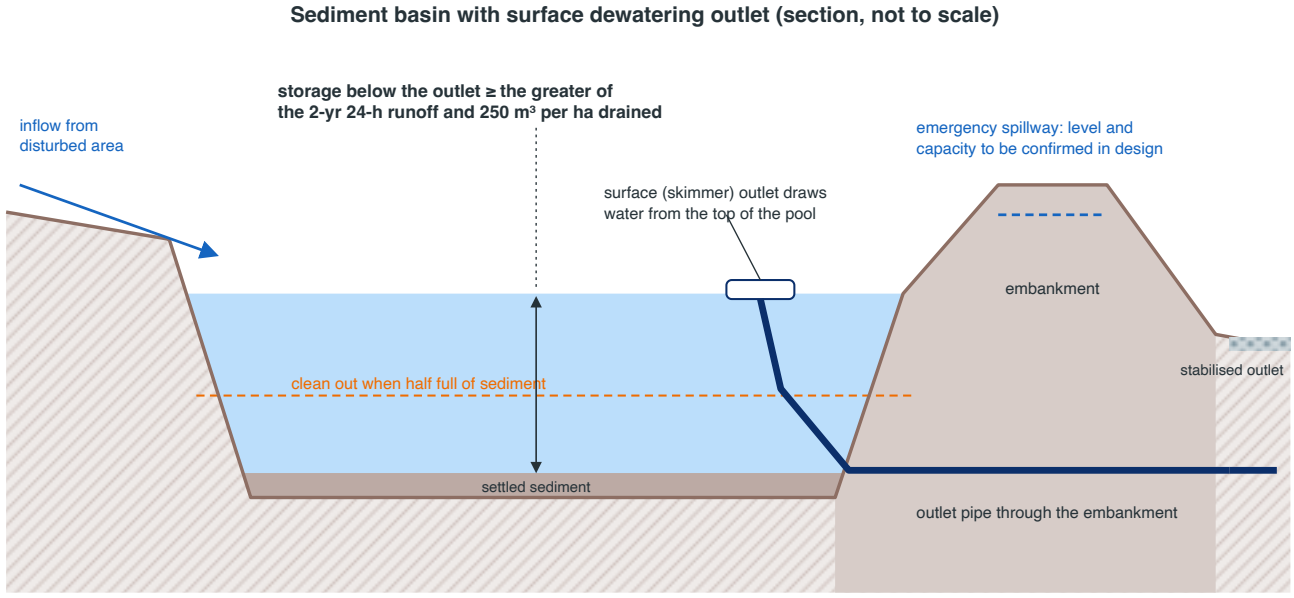
- drainage area: not more than **0.1 ha per 30 m** of fence;
- slope length above the fence: not more than 30 m on slopes under 2%, 23 m at 2–5%, 15 m at 5–10%, 8 m at 10–20% and 5 m steeper than 20%;
- wherever flow arrives concentrated, or a limit cannot be met, a sediment trap (Figure 8E.2) is used instead.

The designer shall show the drainage area and slope length served by each fence on the ESC plan.

Notes:

1. Place along the contour at the downslope edge of the disturbed area, never across a concentrated flow path.
2. Turn the ends upslope so that water cannot run round them.
3. Inspect weekly and within 24 hours after every heavy-rainfall event; repair within 48 hours; remove sediment when the deposit reaches half the fence height (Clause 8.12.5).
4. Remove only after the area draining to it is stabilised (Clause 8.12.6).
5. Stockpiles: outside Zone 1, at least 10 m from any drain, and covered during the SW monsoon (Clause 8.12.3(f)).

8E.2 Sediment trap and sediment basin



Sediment trap: disturbed area up to 2 ha. Sediment basin: more than 2 ha (Clause 8.12.3(b), (c)).
Design rainfall from Chapter 2 with climate factor 1.0 (Clause 8.12.4). Dewatering of excavations passes through the basin, a settling tank or a filter bag.

Figure 8E.2 — Sediment trap or basin in section: inflow, storage volume below the outlet, surface dewatering outlet (skimmer) drawing water from the top of the pool, emergency spillway, and sediment cleanout level at half the storage. Original drawing for this Manual, adapted from US EPA, NPDES Construction General Permit (2022).

Notes:

1. **Sediment trap:** for concentrated flow from disturbed areas of up to 2 ha (Clause 8.12.3(b)). Storage volume, depth and outlet: to be confirmed in design.

2. **Sediment basin:** for disturbed areas of more than 2 ha. Storage below the outlet ≥ the greater of (a) the runoff from the 50% AEP (2-yr) 24-hour storm and (b) 250 m³ per hectare of drainage area (Clause 8.12.3(c)).
3. The dewatering outlet draws water from the surface (Clause 8.12.3(c)).
4. An emergency spillway and its capacity: to be confirmed in design. Where the basin is also a permanent detention facility, the Chapter 4 spillway rules apply.
5. Clean out when half full of sediment (Clause 8.12.5). Remove sediment from any permanent facility before its as-built survey (Clause 8.12.6).
6. Dewatering of excavations shall pass through a sediment basin, settling tank or filter bag (Clause 8.12.3(g)).

Illustrative — not design values. A sediment basin serves 3.0 ha of disturbed land. Assume a 2-yr 24-hour rainfall of 100 mm (climate factor 1.0, Clause 8.12.4) and a curve number of 86 for bare compacted ground under the antecedent moisture condition of Chapter 2.

- $S = 25{,}400 / 86 - 254 = 41.35$ mm; $I_a = 0.2S = 8.27$ mm
- $Q = (P - I_a)^2 / (P - I_a + S) = (100 - 8.27)^2 / (100 - 8.27 + 41.35) = 8{,}414 / 133.08 = 63.2$ mm
- (a) Runoff volume = $10 \times Q \times A = 10 \times 63.2 \times 3.0 = 1{,}896$ m³
- (b) 250 m³/ha × 3.0 ha = 750 m³
- Required storage below the outlet = max (1,896, 750) = 1,896 m³. Here the 2-yr runoff governs.

8E.3 Stabilised construction exit and concrete washout

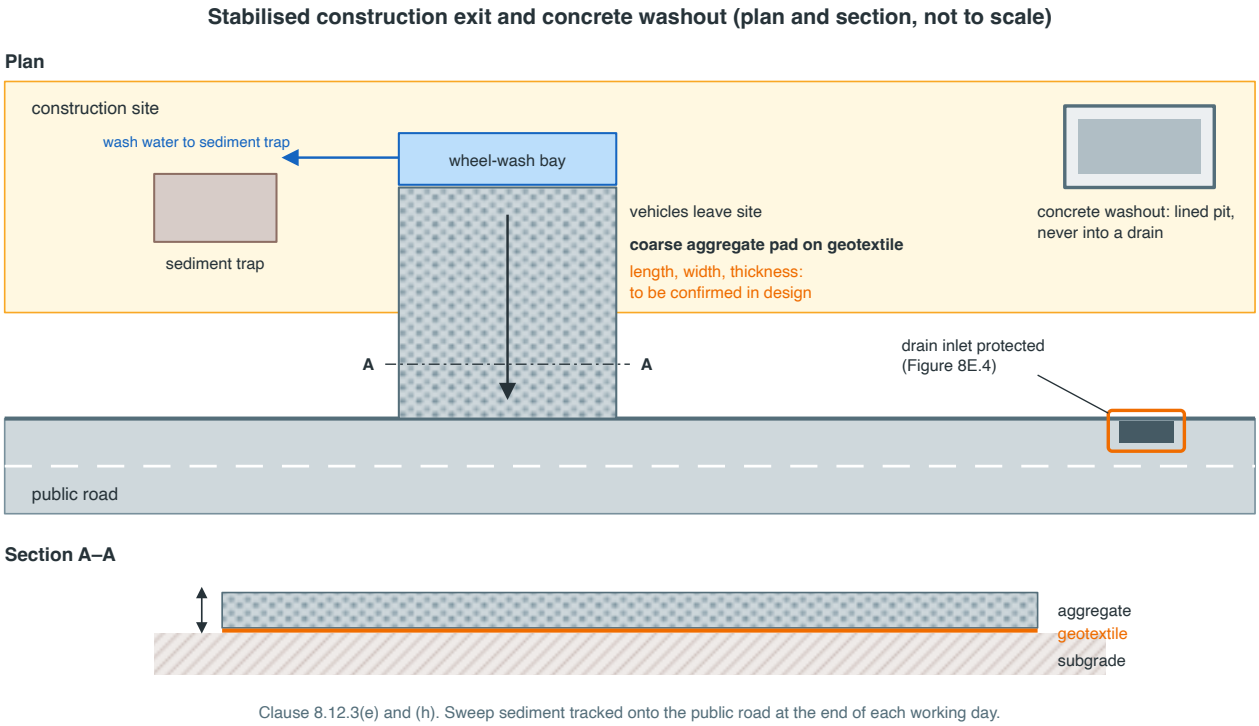


Figure 8E.3 — Stabilised construction exit: plan and section showing a coarse aggregate pad on geotextile between the site and the public road, a wheel-wash bay draining to a sediment trap, and a lined concrete washout pit. Dimensions not set by the Manual are to be confirmed in design. Original drawing for this Manual, adapted from US EPA, NPDES Construction General Permit (2022).

Notes:

1. Provide at every point where vehicles leave the site onto a public road, with wheel washing (Clause 8.12.3(e)).
2. Pad length, width, thickness and aggregate size: to be confirmed in design.
3. Wash water drains to a sediment trap or basin, never to a drain.
4. Concrete washout in a lined pit, never into a drain (Clause 8.12.3(h)).

5. Sweep any sediment tracked onto the public road at the end of each working day.

8E.4 Inlet protection

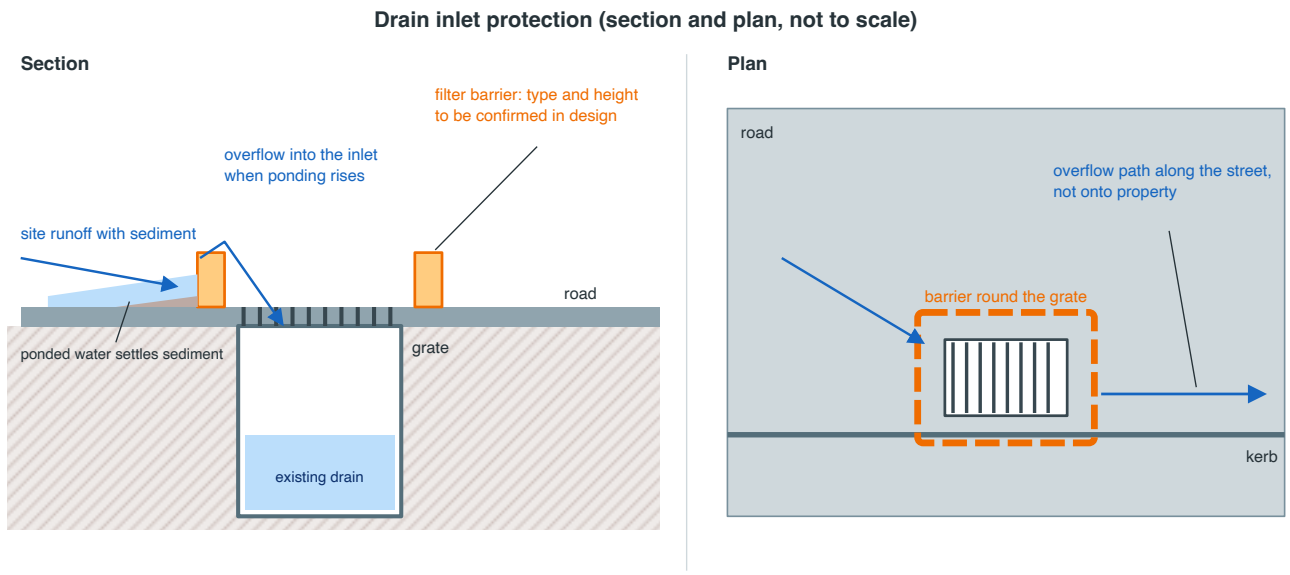


Figure 8E.4 — Inlet protection at an existing drain inlet: a filter barrier around a grate or kerb inlet, with an overflow path so that ponding does not flood the road or property. Original drawing for this Manual, adapted from US EPA, NPDES Construction General Permit (2022), and CIRIA, The SuDS Manual (C753), 2015.

Notes:

1. At every existing drain inlet that receives runoff from the site (Clause 8.12.3(d)).
2. The protection must not block the inlet: water ponding behind it must be able to overflow into the inlet or along the street.
3. Barrier type and height: to be confirmed in design.
4. Remove sediment after each heavy-rainfall event (Clause 8.12.5).

8E.5 Timing and inspection

Table 8E.2 — ESC timing and inspection (Clauses 8.12.2 and 8.12.5)

Item	Requirement
Completed earthworks	Stabilised by vegetation, mulch, matting or paving by 31 May
Area cleared between 10 June and 20 October	Sediment controls in place before clearing
Area exposed in the SW monsoon	As small as practicable
Inspection	Weekly, and within 24 h after every heavy-rainfall event (Chapter 10)
Repair	Within 48 h
Cleaning	When half full of sediment
Removal	Only after the area draining to the measure is stabilised

Form 8-1 — Water Quality Sheet

Supports Clauses 8.4, 8.6 and 8.14 of Chapter 8.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

Table 8F.1 — Form 8-1, Section A: Site and rainfall

No.	Field	Entry	Clause
A1	Project name and Authority file number	_____	—
A2	Tier (A / B) and site area, ha	_____	8.6.1
A3	Rainfall zone (Chapter 2)	_____	8.4.1
A4	P_ret, mm (Annex 2E)	_____	8.4.1
A5	P_3m, mm (Annex 8A, or the interim calculation: record, years, method)	_____	8.4.1
A6	P_wq = max (P_ret, P_3m), mm (no climate factor)	_____	8.4.1, 8.4.3
A7	WQv method: SCS-CN, or Rv (sites < 0.5 ha and Tier B simplified method only)	_____	8.4.2
A8	Redevelopment (existing impervious ≥ 20%)?	_____	8.6.2

Table 8F.2 — Form 8-1, Section B: Water quality volume and flow by drainage area

Area ID	Area (ha)	Impervious I (%)	Q_wq runoff depth (mm) or Rv	WQv (m^3)	Q_wq (m^3/s)	Volume retained, Chapter 4 (m^3)	WQv not retained (m^3)	BMP serving the area	Pretreatment
_____	_____	_____	_____	_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____	_____	_____	_____	_____
Total	_____			_____		_____	_____		

Table 8F.3 — Form 8-1, Section C: Level of Service and Value Rating (Tier A)

No.	Field	Entry	Clause
C1	CN pre-development; CN post-development; ΔCN	_____	8.6.2
C2	Required LoS (Table 8.2, or Annex 8B for redevelopment)	_____	8.6.2
C3	Composite VR (Annex 8B calculation attached)	_____	8.6.3
C4	Composite VR ≥ required LoS?	_____	8.6.3
C5	Tier B: route used under Clause 8.6.5 (a) retained on site / (b) bioretention, rain garden or infiltration	_____	8.6.5

Table 8F.4 — Form 8-1, Section D: Other submittals (Clause 8.14)

No.	Item	Attached (yes / no / not applicable)	Clause
D1	BMP designs to Clause 8.7 and Annex 8D, with pretreatment	_____	8.14(3)

No.	Item	Attached (yes / no / not applicable)	Clause
D2	Infiltration Test Record (Form 8-2), seasonal high groundwater level and source, design rate, drawdown time, setbacks	_____	8.14(4)
D3	Sewage and sullage connection plan; dye or smoke test record	_____	8.9.2, 8.14(5)
D4	GPT and first-flush design for any outfall to a waterbody	_____	8.10, 8.14(6)
D5	RWH layout separating harvesting, recharge and detention	_____	8.11, 8.14(7)
D6	ESC plan	_____	8.12, 8.14(8)
D7	Hotspot controls, where Clause 8.8.8 applies	_____	8.8.8

Table 8F.5 — Form 8-1, Section E: Prepared by

Field	Entry
Name, registration and empanelment number (Clause 10.6)	_____
Signature and date	_____

Form 8-2 — Infiltration Test Record

Supports Clauses 8.8.1 to 8.8.7 and 8.11.4 of Chapter 8.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. One record per facility. Attach a plan showing the test locations, and the readings for each test.

Table 8F.6 — Form 8-2, Section A: Facility

No.	Field	Entry	Clause
A1	Project and facility ID; facility type	_____	8.8.3
A2	Infiltration base area (m²) or trench length (m)	_____	8.8.3(c)
A3	Proposed base level of the facility, m (datum)	_____	8.8.3
A4	Runoff source: roof only / paved surfaces / mixed; hotspot (yes / no)	_____	8.8.6, 8.8.8
A5	Ground setting: sandy coastal / laterite or hard rock / delta clay or black cotton / other	_____	8.8.9–8.8.11

Table 8F.7 — Form 8-2, Section B: Tests

Test no.	Location (grid ref.)	Date	Method: double-ring (ASTM D3385) / falling-head borehole or trial-pit soakage (e.g. BRE Digest 365)	Test level, m	Groundwater level at time of test, m	Measured rate (mm/h)
1	_____	_____	_____	_____	_____	_____
2	_____	_____	_____	_____	_____	_____
3	_____	_____	_____	_____	_____	_____

Minimum number of tests: 2 per facility, and 1 per 100 m² of base area or per 50 m of trench (Clause 8.8.3(c)).

Table 8F.8 — Form 8-2, Section C: Design rate and drawdown

No.	Item	Entry	Clause
C1	Number of tests made; number required	_____	8.8.3(c)
C2	Highest and lowest measured rates; ratio (highest ÷ lowest)	_____	8.8.4
C3	Safety factor: 2.0 (tests meet 8.8.3(c) and ratio ≤ 2) or 3.0	_____	8.8.4
C4	Design rate = measured rate used ÷ safety factor, mm/h (state which measured rate was used)	_____	8.8.4
C5	Category: ≥ 13 mm/h full infiltration / 3–13 mm/h partial with underdrain / < 3 mm/h no credit	_____	8.8.5
C6	Depth of water to infiltrate, mm; drawdown time = depth ÷ design rate, h (≤ 72 h)	_____	8.8.7

Table 8F.9 — Form 8-2, Section D: Separation and setbacks

No.	Item	Entry	Clause
D1	Seasonal high groundwater level, m, and its source (Clause 3.10.2)	_____	8.8.1
D2	Separation from facility base to seasonal high groundwater, m (≥ 1.0; ≥ 0.5 for roof-only runoff in notified coastal sand with mounding check)	_____	8.8.1, 8.8.9(d)

No.	Item	Entry	Clause
D3	Depth to bedrock or impermeable layer below base, m (≥ 1.0 unless Clause 8.8.10)	_____	8.8.2
D4	Distances to: foundations / basement / property boundary / drinking-water well or borewell / septic tank or soak pit / crest of slope steeper than 1V:3H, m	_____	8.8.6
D5	Expansive soils: foundation setback doubled?	_____	8.8.11(b)

Table 8F.10 — Form 8-2, Section E: Declaration

Field	Entry
Tester: name, qualification in geotechnical testing, organisation	_____
I certify that the tests were made at the stated locations and levels, by the stated methods, and that the readings attached are complete. Signature and date	_____
Designer (Clause 10.6): name and signature	_____

ANNEXES TO CHAPTER 9

Hydraulic Design of Structures

These annexes support Chapter 9. Annex 9A gives the FHWA HDS-5 inlet-control equations in SI units, with published coefficients for common inlets, and a full worked culvert example. Annex 9B gives standard drawings that carry only the dimensions Chapter 9 and Chapter 6 already set; every other dimension is marked "to be confirmed in design". Annex 9C is a pump station design checklist with a standard control philosophy drawn from Clause 9.9. Form 9-1 is a draft form: it lists what Clause 9.14 requires and does not add to it.

Annex 9A — HDS-5 inlet-control coefficients and worked culvert example in SI units

Supports Clauses 9.4.1 to 9.4.7 of Chapter 9 and Clause 6.12 of Chapter 6.

9A.1 Inlet-control equations

FHWA *Hydraulic Design of Highway Culverts*, HDS-5, 3rd edition (2012), Appendix A, gives three inlet-control equations. In SI units:

> Unsubmerged, Form 1: HWi/D = Hc/D + K · [Ku · Q / (A · D^0.5)]^M + Ks · S

> Unsubmerged, Form 2: HWi/D = K · [Ku · Q / (A · D^0.5)]^M

> Submerged: HWi/D = c · [Ku · Q / (A · D^0.5)]² + Y + Ks · S

where HWi is the headwater depth above the inlet invert under inlet control (m), D is the interior height (rise) of the barrel (m), Hc is the specific head at critical depth, dc + Vc²/2g (m), Q is the discharge (m³/s), A is the full cross-sectional area of the barrel (m²), S is the barrel slope (m/m), K, M, c and Y are the constants of Table 9A.1, Ku = 1.811 in SI units (1.0 in US units), and Ks is the slope correction: −0.5, or +0.7 for mitred inlets.

Which equation applies depends on X = Ku · Q / (A · D^0.5):

- X ≤ 3.5: unsubmerged equation (Form 1 or Form 2 as listed in Table 9A.1);
- X ≥ 4.0: submerged equation;
- 3.5 < X < 4.0 (transition): use FHWA HY-8, which implements the HDS-5 transition. Where a hand calculation is used, take the higher of the unsubmerged and submerged results. This is a conservative simplification made by this annex, not an HDS-5 rule.

Table 9A.1 — HDS-5 inlet-control coefficients for common inlets

Barrel	Inlet edge	Form	K	M	c	Y
Circular concrete	Square edge with headwall	1	0.0098	2.0	0.0398	0.67
Circular concrete	Groove (socket) end with headwall	1	0.0018	2.0	0.0292	0.74
Circular concrete	Groove (socket) end projecting	1	0.0045	2.0	0.0317	0.69
Circular corrugated metal	Headwall	1	0.0078	2.0	0.0379	0.69
Circular corrugated metal	Mitred to slope	1	0.0210	1.33	0.0463	0.75
Circular corrugated metal	Projecting	1	0.0340	1.50	0.0553	0.54
Rectangular box	Wingwalls flared 30° to 75°	1	0.026	1.0	0.0347	0.81
Rectangular box	Wingwalls flared 90° and 15°	1	0.061	0.75	0.0400	0.80
Rectangular box	Wingwalls at 0° (extensions of the sides)	1	0.061	0.75	0.0423	0.82
Rectangular box	90° headwall with 19 mm (3/4 in) chamfers	2	0.515	0.667	0.0375	0.79
Rectangular box	90° headwall with 45° bevels	2	0.495	0.667	0.0314	0.82
Rectangular box	90° headwall with 33.7° bevels	2	0.486	0.667	0.0252	0.865

Source: FHWA HDS-5 (2012), Appendix A, Table A.1 (the coefficients are dimensionless and apply in SI units with Ku = 1.811). The values were checked against HIF-12-026, Table A.1 (chart and scale numbers 1/1–1/3, 2/1–2/3, 8/1–8/3 and 10/1–10/3). Where a later edition of HDS-5 differs, the later table governs. Entrance loss coefficients for outlet control are in Table 9.1 of Chapter 9.

9A.2 Worked example: pipe culvert under a collector road

Illustrative — not design values. The inputs below are invented to show the method; the results are not design values for any site.

Inputs.

Table 9A.2 — Inputs for the worked example

Item	Value	Source
Road class	Collector; design AEP 4% (25-yr), check event 2% (50-yr), and the 1% AEP check	Table 6.1, Clause 6.12.2
Design flow, 4% AEP	1.80 m³/s (including the climate factor)	Illustrative
Check flow, 2% AEP	2.20 m³/s	Illustrative
1% AEP flow	3.00 m³/s	Illustrative
Barrel	One 1,200 mm RCC pipe with collar joints, n = 0.015	Table 6.6
Inlet	Square edge with headwall: ke = 0.5; K = 0.0098, M = 2.0, c = 0.0398, Y = 0.67, Form 1	Tables 9.1 and 9A.1
Length and slope	L = 20.0 m; S = 0.005 (inlet invert 20.00 m, outlet invert 19.90 m, local datum)	Illustrative
Road levels at the inlet	Shoulder edge 22.00 m; lowest point of the road crest 22.10 m	Illustrative
Tailwater above the outlet invert, TW	0.60 m at 1.80 m³/s; 0.70 m at 2.20 m³/s; 0.85 m at 3.00 m³/s (normal depth in the downstream channel)	Illustrative; Clause 9.4.4
Receiving channel	Unlined sandy loam	Table 9.2

Minimum size: 1,200 mm ≥ 900 mm (Clause 6.12.5). The span is below 1.5 m, so the blockage check uses 50% of the inlet area blocked (Clause 6.12.4(a)).

Step 1 — Barrel properties. A = π · 1.2² / 4 = 1.131 m²; full-barrel hydraulic radius R = D/4 = 0.300 m; D^0.5 = 1.095.

Step 2 — Critical depth. For each flow, dc is found by trial from the critical-flow condition for a circular section, $Q^2 \cdot T / (g \cdot Ac^3) = 1$, where Ac and T are the flow area and top width at depth dc.

Table 9A.3 — Critical depth and specific head (D = 1.2 m)

Q (m³/s)	dc (m)	Ac (m²)	T (m)	Vc = Q/Ac (m/s)	Hc = dc + Vc²/2g (m)	Hc/D
1.80	0.737	0.728	1.168	2.471	1.048	0.873
2.20	0.817	0.820	1.119	2.683	1.184	0.987
3.00	0.952	0.962	0.972	3.118	1.447	1.206

Step 3 — Headwater under inlet control. X = 1.811 · Q / (1.131 × 1.095) = 1.462 · Q.

Table 9A.4 — Inlet-control headwater

Q (m³/s)	X	Equation	HWi/D	HWi (m)
1.80	2.631	Unsubmerged, Form 1: 0.873 + 0.0098 × 2.631² − 0.5 × 0.005 = 0.873 + 0.068 − 0.003	0.939	1.127
2.20	3.216	Unsubmerged, Form 1: 0.987 + 0.0098 × 3.216² − 0.003 = 0.987 + 0.101 − 0.003	1.085	1.302
3.00	4.385	Submerged: 0.0398 × 4.385² + 0.67 − 0.003 = 0.765 + 0.67 − 0.003	1.433	1.719

Step 4 — Headwater under outlet control (Clause 9.4.3). The friction term is 19.63 · n² · L / R^1.33 = 19.63 × 0.015² × 20.0 / 0.300^1.33 = 0.0883 / 0.2016 = 0.438, so H = (1 + 0.5 + 0.438) · V²/2g = 1.938 · V²/2g. L · S = 20.0 × 0.005 = 0.100 m.

Table 9A.5 — Outlet-control headwater

Q (m³/s)	V = Q/A (m/s)	V²/2g (m)	H (m)	(dc + D)/2 (m)	TW (m)	ho (m)	HWo = ho + H − L·S (m)
1.80	1.592	0.129	0.250	0.969	0.60	0.969	1.119
2.20	1.945	0.193	0.374	1.009	0.70	1.009	1.283
3.00	2.653	0.359	0.695	1.076	0.85	1.076	1.671

Step 5 — Governing headwater (Clause 9.4.1): the greater of HWi and HWo.

Table 9A.6 — Governing headwater and checks

Event	Q (m³/s)	HWi (m)	HWo (m)	Control	HW (m)	HW level (m)	HW/D
Design, 4% AEP	1.80	1.127	1.119	Inlet	1.127	21.13	0.94
Check, 2% AEP	2.20	1.302	1.283	Inlet	1.302	21.30	1.09
1% AEP	3.00	1.719	1.671	Inlet	1.719	21.72	1.43

Step 6 — Headwater limits at the design AEP (Clause 9.4.5):

- (a) HW/D = 0.94 ≤ 1.2. Complies.
- (b) HW level 21.13 m ≤ shoulder edge 22.00 m − 0.30 m = 21.70 m. Complies.

- (c) Water levels at upstream buildings and property boundaries are compared with the existing condition under Clause 6.12.2(d). This needs the upstream survey and is not shown here.

Step 7 — Road overtopping (Clauses 6.12.2, 6.12.3 and 9.4.6). The 1% AEP headwater level, 21.72 m, is below the lowest road crest, 22.10 m, so the road is not overtopped in the check or 1% AEP events. Had the headwater exceeded the crest, the overtopping flow Qr = Cd · L · h^1.5 (Cd ≤ 1.66) would be balanced with the culvert flow at a common headwater, and the overtopping depth checked against Clause 6.12.2(a).

Step 8 — Outlet velocity (Clause 9.4.7). Under inlet control, the outlet velocity is the velocity at normal depth in the barrel. The barrel's full-flow capacity is (1/n) · A · R^(2/3) · S^0.5 = (1/0.015) × 1.131 × 0.448 × 0.0707 = 2.39 m³/s.

- 1.80 m³/s: normal depth 0.778 m, outlet velocity 2.32 m/s.
- 2.20 m³/s: normal depth 0.908 m, outlet velocity 2.40 m/s.
- 3.00 m³/s: above the full-flow capacity, so the barrel flows full at the outlet; velocity = Q/A = 2.65 m/s.

The receiving channel is unlined sandy loam, with a permissible velocity of 0.53 m/s (Table 9.2), so outlet protection is required (Clause 9.6.1). All barrel velocities are below 3.0 m/s (Clause 6.7.3(b)).

Step 9 — Riprap apron (Clause 9.6.3; Figure 9.2). D50 = 0.2 · D · [Q / (g^0.5 · D^2.5)]^(4/3) · (D/TW), with g^0.5 · D^2.5 = 3.132 × 1.577 = 4.941.

Table 9A.7 — Riprap apron sizing

Q (m³/s)	TW (m)	Q / (g^0.5 · D^2.5)	[ ]^(4/3)	D50 (m)
1.80	0.60	0.364	0.260	0.2 × 1.2 × 0.260 × 2.00 = 0.125
2.20	0.70	0.445	0.340	0.2 × 1.2 × 0.340 × 1.71 = 0.140
3.00	0.85	0.607	0.514	0.2 × 1.2 × 0.514 × 1.41 = 0.174

Clause 9.4.7 requires the outlet to be protected for the design AEP and the 1% AEP, so the apron is sized here for the largest D50, 0.174 m. From Table 9.3, Class 2 (D50 = 150 mm) is too small, so **Class 3** (D50 = 250 mm) is used:

- apron length 5D = 5 × 1.2 = 6.0 m;
- apron thickness 2.4 × D50 = 2.4 × 0.25 = 0.60 m;
- apron width at the outlet at least 3D = 3.6 m, flaring to the downstream channel;
- stone placed on a granular filter or geotextile (Clause 9.6.4).

Step 10 — Blockage check (Clause 6.12.4(a)). Steps 3 to 7 are repeated for the 1% AEP flow with 50% of the inlet area blocked, and the results must meet Clause 6.12.2(a) and (d). This Manual does not yet set a standard way of representing a partly blocked inlet, so this check is carried out in HY-8 or by a method agreed with the Approving Authority. It is not worked here.

Table 9A.8 — Summary of the worked example (Illustrative — not design values)

Check	Result	Clause
Minimum size	1,200 mm ≥ 900 mm	6.12.5
Design HW/D	0.94 ≤ 1.2	9.4.5(a)
Design HW below shoulder edge	21.13 m ≤ 21.70 m	9.4.5(b)
Overtopping in the check and 1% events	None (21.72 m < 22.10 m)	6.12.2, 9.4.6
Outlet protection	Class 3 riprap apron, 6.0 m long, 0.60 m thick, 3.6 m wide at the outlet	9.6.1, 9.6.3
Blockage check	To be run in HY-8 or by an agreed method	6.12.4

Annex 9B — Standard drawings: headwalls, riprap aprons, drop structures, trash racks, detention risers

Supports Clauses 9.4, 9.6, 9.7 and 9.8 of Chapter 9.

The drawings below are schematic standard drawings. They show only the dimensions and rules that Chapter 9 and Chapter 6 set. Structural dimensions, reinforcement, foundations and fixings are to be confirmed in design (Clause 9.1.2). The riprap apron standard drawing is Figure 9.2 of Chapter 9, with dimensions from Table 9.3; it is not repeated here. Table 9B.1 lists the dimensions shown and their source clauses.

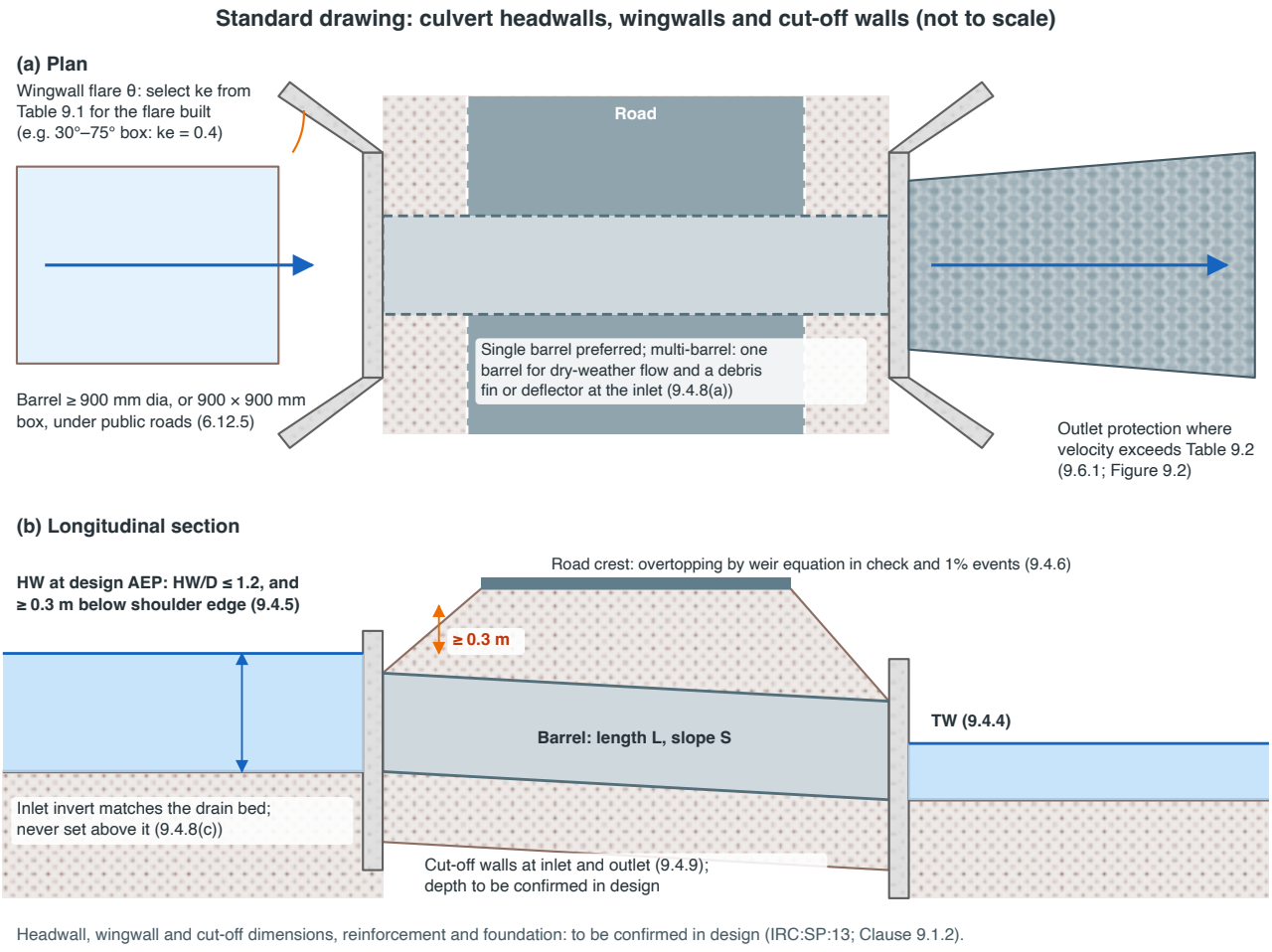


Figure 9B.1 — Standard drawing: culvert headwalls, wingwalls and cut-off walls, in plan and long section, with the headwater limits, invert rule and debris provisions. Original drawing for this Manual, adapted from FHWA, Hydraulic Design of Highway Culverts, HDS-5 (3rd ed., 2012).

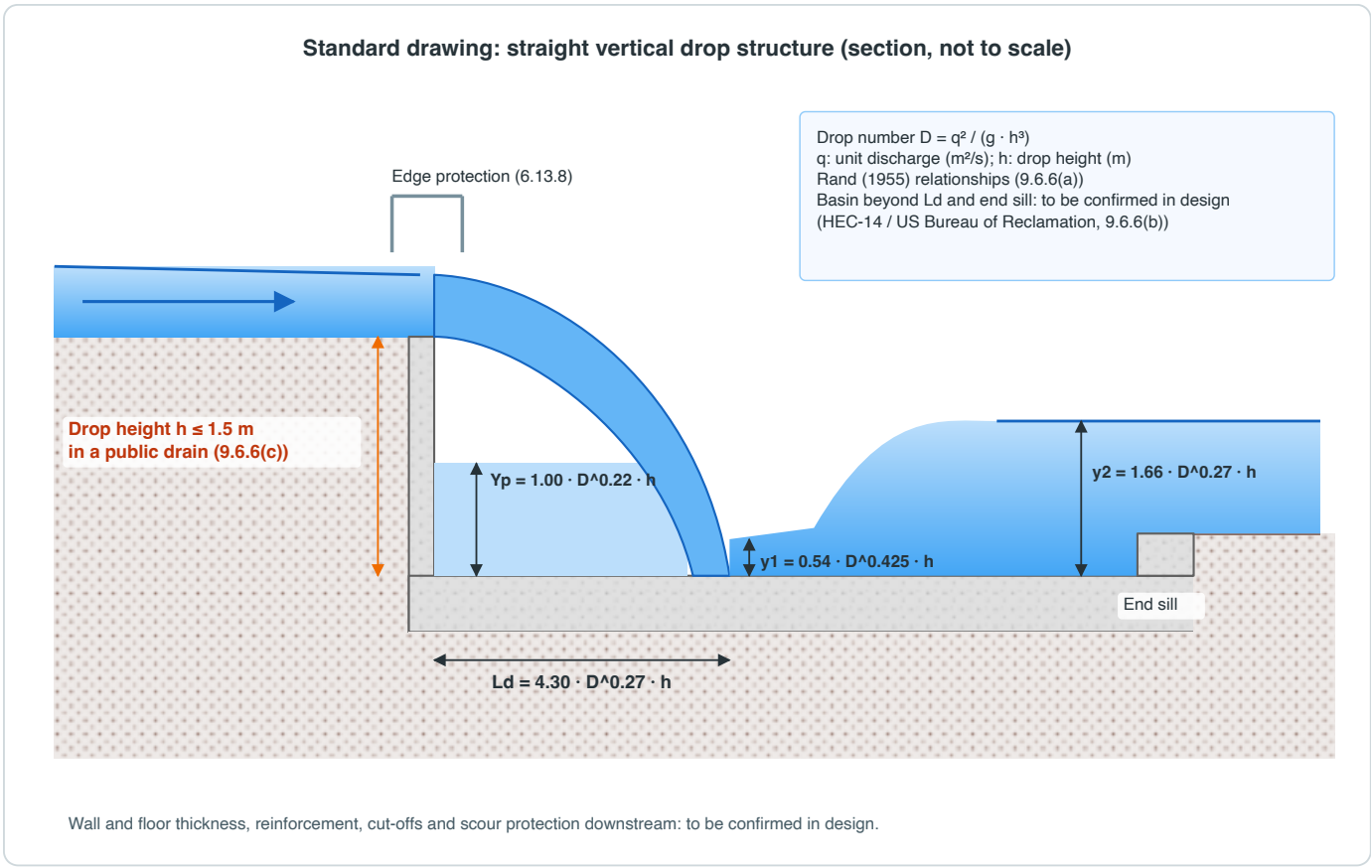


Figure 9B.2 — Standard drawing: straight vertical drop structure with the Rand (1955) pool geometry and the 1.5 m height limit. Original drawing for this Manual, adapted from Chow, Open-Channel Hydraulics (1959), and FHWA, HEC-14 (3rd ed., 2006).

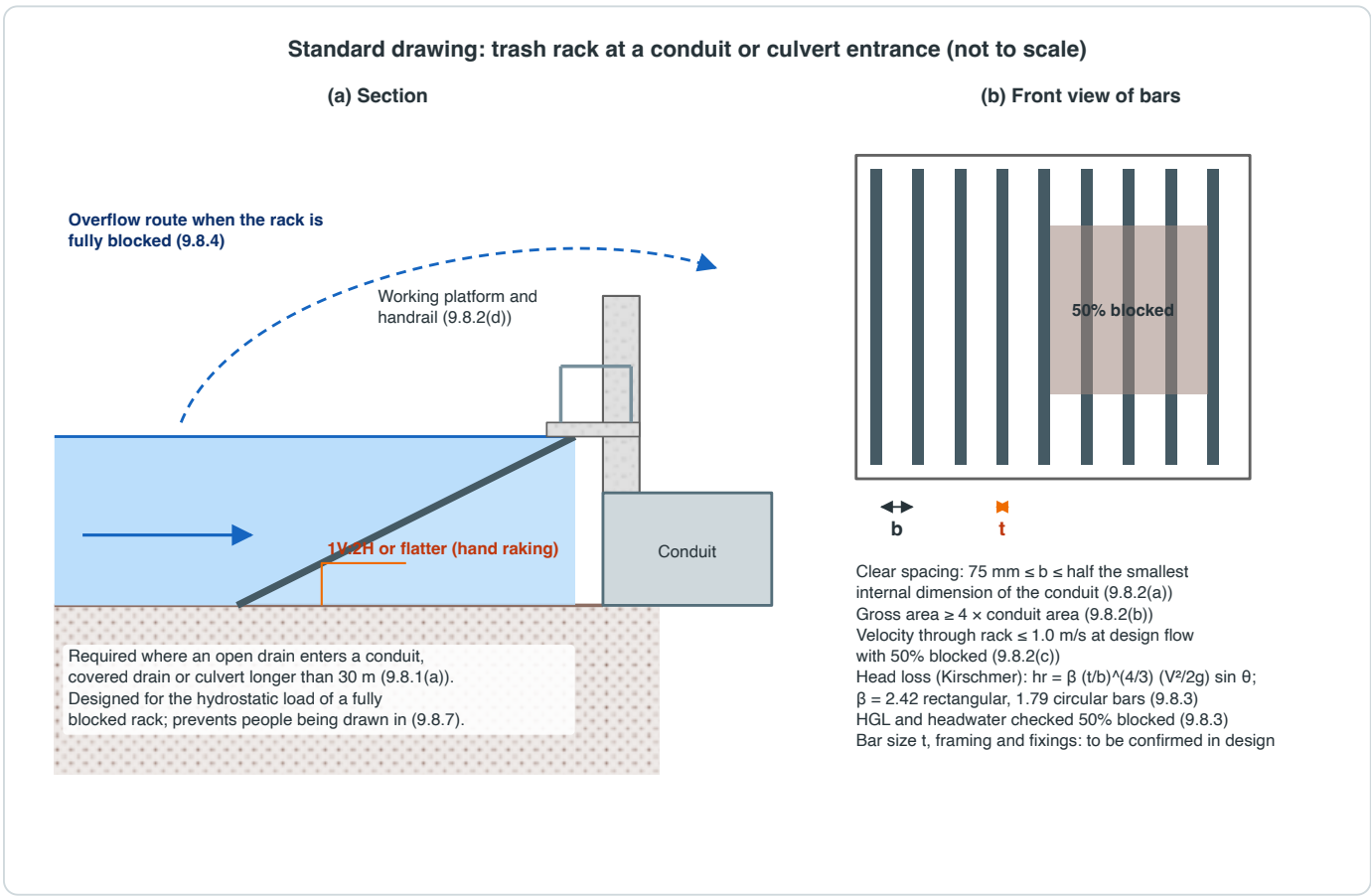


Figure 9B.3 — Standard drawing: trash rack at a conduit or culvert entrance, in section and front view, with bar spacing, area, velocity, slope and overflow requirements. Original drawing for this Manual, adapted from CIRIA, Culvert, screen and outfall manual, C786 (2019), and CPHEEO, Manual on Storm Water Drainage Systems (2019), §8.3.1.3.

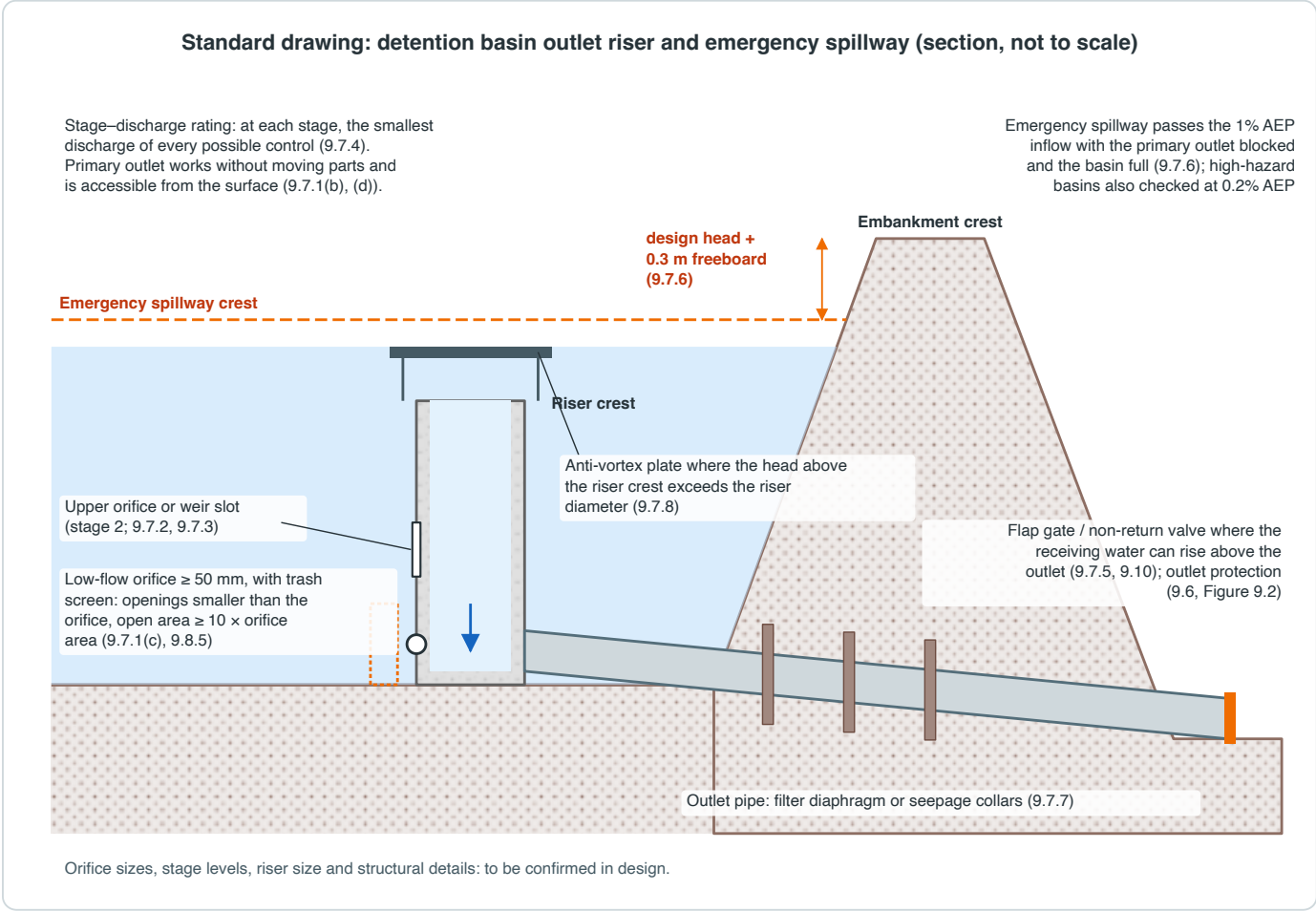


Figure 9B.4 — Standard drawing: detention basin outlet riser with low-flow orifice and screen, upper stage, anti-vortex plate, outlet pipe through the embankment, flap gate and emergency spillway. Original drawing for this Manual, adapted from Kansas City Metro APWA, Section 5600 (2006, 5608.4).

Table 9B.1 — Dimensions and rules on the standard drawings and their source clauses

Drawing	Item	Dimension or rule	Clause
9B.1	Minimum culvert size under public roads	900 mm diameter, or 900 mm × 900 mm box	6.12.5
9B.1	Headwater at the design AEP	HW/D ≤ 1.2; at least 0.3 m below the shoulder edge at the upstream face	9.4.5
9B.1	Entrance loss coefficient	From Table 9.1 for the inlet built (for example 0.4 for a box with wingwalls at 30°–75°)	9.4.3
9B.1	Inverts	Match the drain bed; never above it	9.4.8(c)
9B.1	Multi-barrel culverts	Avoided in debris-prone drains; where used, one barrel for dry-weather flow and a debris fin or deflector	9.4.8(a)
9B.1	Headwalls, wingwalls, cut-off walls	Required; dimensions to be confirmed in design	9.4.9
9B.1	Road overtopping	Qr = Cd · L · h^1.5, Cd ≤ 1.66	9.4.6
Figure 9.2	Riprap apron	Width at the outlet ≥ 3D, flaring to the channel; length and thickness from Table 9.3; filter or geotextile	9.6.3, 9.6.4

Drawing	Item	Dimension or rule	Clause
9B.2	Single vertical drop in a public drain	Height h ≤ 1.5 m; edge protection	9.6.6(c), 6.13.8
9B.2	Pool geometry	D = q²/(g h³); Ld/h = 4.30 D^0.27; Yp/h = 1.00 D^0.22; y1/h = 0.54 D^0.425; y2/h = 1.66 D^0.27	9.6.6(a)
9B.2	Basin beyond Ld, end sill	To be confirmed in design (HEC-14 or US Bureau of Reclamation)	9.6.6(b)
9B.3	Where required	Where an open drain enters a conduit, covered drain or culvert longer than 30 m	9.8.1(a)
9B.3	Clear bar spacing b	Not less than 75 mm and not more than half the smallest internal dimension of the conduit	9.8.2(a)
9B.3	Gross rack area	At least 4 times the conduit area	9.8.2(b)
9B.3	Velocity through the rack	Not more than 1.0 m/s at the design flow with 50% blocked	9.8.2(c)
9B.3	Inclination	No steeper than 1V:2H where raked by hand; working platform and handrail	9.8.2(d)
9B.3	Head loss and blocked check	Kirschmer equation; HGL and headwater checked with 50% blocked	9.8.3
9B.3	Overflow and safety	Overflow route for a fully blocked rack; designed for the full hydrostatic load	9.8.4, 9.8.7
9B.4	Low-flow orifice	Not smaller than 50 mm; trash screen with openings smaller than the orifice and open area at least 10 times the orifice area	9.7.1(c), 9.8.5
9B.4	Orifice and weir flow	Q = Cd · A · (2gh)^0.5, Cd = 0.60 sharp-edged; weir coefficients of Clause 9.7.3	9.7.2, 9.7.3
9B.4	Stage–discharge rating	Smallest discharge of every possible control at each stage	9.7.4
9B.4	Anti-vortex plate	Where the design head above the riser crest exceeds the riser diameter	9.7.8
9B.4	Outlet pipe through the embankment	Filter diaphragm or seepage collars; flared or headwalled inlet	9.7.7
9B.4	Flap gate or non-return valve	Where the receiving water can rise above the outlet	9.7.5, 9.10
9B.4	Emergency spillway	Passes the 1% AEP inflow with the primary outlet blocked and the basin full, with 0.3 m freeboard to the embankment crest; 0.2% AEP check for high-hazard basins	9.7.6
All	Structural dimensions, reinforcement, foundations, bar sizes, orifice sizes and stage levels	To be confirmed in design	9.1.2

Annex 9C — Pump station design checklist and standard control philosophy

Supports Clauses 9.8.6, 9.9 and 9.10 of Chapter 9, with Clause 6.10.5 of Chapter 6 and the outfall-closed check of Chapter 5.

9C.1 Design checklist

Submit this checklist with the pump station design report of Clause 9.14, item 6.

Table 9C.1 — Pump station design checklist

No.	Item	Requirement	Clause	Provided (Yes / No / NA)
1	Need for pumping	Gravity drainage cannot meet Chapters 5 and 6; reason stated	9.9.1	
2	Inflow	Inflow hydrograph routed through the catchment and any upstream storage	9.9.2	
3	Design standard	Design AEP of the element served; for underpasses, 2% AEP with a 1% AEP check	9.9.2(a), 6.10.5(a)	
4	Tidal outfall	Storage plus pumps hold the 48 h design volume with the outfall throttled for the tidal-lock duration (default two high tides of 6 h each)	9.9.2(b), Chapter 5	
5	Head range	Pumps selected from the lowest sump level against the lowest tailwater to the highest tailwater (HAT + surge + 0.3 m); pump and system curves submitted	9.9.2(c), 9.14	
6	Wet-well volume	Greatest of: 5 min of minimum inflow (variable speed and automatic control); 10 to 15 min of peak inflow, or the manufacturer's minimum cycle volume; $V = \Theta \cdot q / 4$ for each constant-speed pump	9.9.3	
7	Duty pumps	Same size where practicable	9.9.4(a)	
8	Standby	At least equal to the largest duty pump, and not less than 50% of the number of duty pumps	9.9.4(b)	
9	Alternation	Duty and standby roles alternate automatically	9.9.4(c)	
10	Pump type	Solids-handling (non-clog); axial-flow only behind a screen meeting Clause 9.8.6	9.9.5	
11	Sump and intake	Manufacturer's requirements and ANSI/HI 9.8; clear space 0.6 m (small and medium) or 1.0 m (large units)	9.9.6	
12	Model study	Physical model where any pump exceeds 1 m^3/s or the sump departs from ANSI/HI 9.8 geometry	9.9.6	
13	Silt trap and screen	Upstream of the wet well; screen spacing smaller than the pumps' solids size; removable modules; mechanical raking where inflow exceeds 1 m^3/s	9.9.7, 9.8.6	
14	Screen overflow	Overflow route when the screen is fully blocked	9.8.4	
15	Level control	Primary level sensor (ultrasonic, radar or pressure) with an independent float-switch back-up	9.9.8(a)	
16	Alarms	High-level and pump-fault alarms by telemetry to the local body's control room	9.9.8(b)	
17	Records	Sump levels, pump run hours and power status logged and kept at least 5 years	9.9.8(c)	
18	Power sources	Grid supply plus a standby generator or second independent feeder able to run all duty pumps at once	9.9.9(a)	
19	Fuel	At least 72 h of all duty pumps, stored above the flood level of Clause 9.9.10	9.9.9(b)	
20	Changeover	Automatic changeover to standby power	9.9.9(c)	
21	Flood protection	Electrical and control room floor, generator and transformer at least 0.5 m above the 1% AEP flood level; vehicle access usable in the 1% AEP event	9.9.10	
22	Discharge	Separate rising mains, or a non-return and an isolating valve where mains combine; delivery chamber or outfall with a flap gate or non-return valve; mains sized by a recognised friction formula	9.9.11	

No.	Item	Requirement	Clause	Provided (Yes / No / NA)
23	Gates	Flap or tide gate on outfalls below the 1% AEP still-water level; penstock or stop-log slot upstream for outfalls of 900 mm or more and every pumped outfall; head loss from tested data	9.10.1, 9.10.3, 9.10.4	
24	Sand and debris at the outfall	Gate in a chamber with surface access, or elastomeric check valves, at beach, estuary and creek outfalls	9.10.5	
25	Emergency connection	Hard-standing and quick-coupling connection for mobile pumps into the delivery chamber	9.9.12	
26	Materials	Exposure class of Clause 9.13; stainless steel grade 316, galvanised and coated steel, or non-metallic fixings within 5 km of the coast	9.13	
27	Design life	At least 50 years for civil works and 25 years for mechanical and electrical equipment	9.2.4	
28	Underpass stations	Depth markers and automatic warning or closure signals at 150 mm depth on the carriageway	6.10.5(d)	
29	Certification	Designed by an empanelled professional and checked by an independent empanelled professional	10.6.1(d), 10.6.4	

9C.2 Standard control philosophy

This is the standard control philosophy for a stormwater pump station designed under Clause 9.9. The designer adapts it to the station and submits it with the design report. Items marked "recommended" are good practice that Chapter 9 does not require; all others restate a clause.

Table 9C.2 — Control levels (values to be set in design)

Level	Rule for setting it	Clause
Pump stop	Not below the minimum submergence required by the pump manufacturer and ANSI/HI 9.8	9.9.6
Duty pump 1 start	Above the pump-stop level by at least the effective wet-well volume of Clause 9.9.3	9.9.3
Duty pumps 2, 3 … start	Staged above duty pump 1 start; spacing to be confirmed in design	9.9.2
High-level alarm	Above the highest duty-pump start level; to be confirmed in design	9.9.8(b)
Float back-up high	Independent float; starts the duty pumps	9.9.8(a)
Float back-up low	Independent float; stops the pumps	9.9.8(a)

1. **Normal (automatic) operation.** As the wet-well level rises to the duty pump 1 start level, the lead pump starts. If the level keeps rising, each further duty pump starts at its start level. The pumps stop when the level falls to the pump-stop level. The primary level sensor controls this sequence (Clause 9.9.8(a)).
2. **Duty rotation.** The lead pump changes automatically, after each pump-down cycle or by run hours as set in the design, so that duty and standby roles alternate and wear is shared (Clause 9.9.4(c)).
3. **Pump fault.** A pump that trips or fails to start is locked out. The next available pump, including the standby pump, takes its place, and a pump-fault alarm is sent by telemetry (Clauses 9.9.4(b) and 9.9.8(b)).
4. **Level-sensor failure.** If the primary sensor fails, or disagrees with the float switches, control passes to the independent float back-up: at the float-high level the duty pumps run, and at the float-low level they stop. An alarm is sent (Clause 9.9.8(a), (b)).
5. **Power failure.** Supply changes over automatically to the standby generator or second feeder (Clause 9.9.9(c)). Recommended: pumps restart one at a time after changeover, to limit the starting load on the generator.

6. **High level.** At the high-level alarm level an alarm is sent by telemetry to the local body's control room (Clause 9.9.8(b)). At an underpass, the warning or closure signals operate at 150 mm depth on the carriageway (Clause 6.10.5(d)).
7. **Outfall and gates.** Rising mains discharge to a delivery chamber or outfall fitted with a flap gate or non-return valve (Clause 9.9.11). Flap gates open and close under head difference without control action. The upstream penstock or stop logs are closed only for maintenance, or if the flap gate jams open (Clause 9.10.3). Recommended: penstock position is shown at the control panel and logged.
8. **Tide-locked operation.** Where the outfall is tide-locked, storage and pumping together hold the 48 h design volume for the tidal-lock duration (Clause 9.9.2(b) and Chapter 5). Where the station also has a gravity outfall, gravity discharge resumes through the flap gate when the tide falls.
9. **Screens.** Mechanically raked screens (required where inflow exceeds 1 m³/s, Clause 9.8.6) are run automatically. Recommended: raking is started by a timer and by the level difference across the screen, with an alarm when the difference stays high.
10. **Manual operation.** Recommended: each pump has a hand–off–auto selector at the panel, and the float back-up high-level start remains active in hand mode.
11. **Mobile pumps.** When fixed pumps are out of service or overwhelmed, mobile pumps discharge into the delivery chamber through the quick-coupling connection (Clause 9.9.12).
12. **Records.** Sump levels, pump run hours and power status are logged and kept for at least 5 years (Clause 9.9.8(c)). Recommended: alarms, pump starts and stops, and changeovers are logged as well.
13. **Testing.** The controls, floats, alarms, telemetry, changeover and generator are tested in the pre-monsoon inspection of Chapter 10 (Clause 10.12), and the results reported in the Annual Maintenance Declaration (Form 10-7; Clause 10.10.3).

Form 9-1 — Hydraulic Structures Report checklist

Supports Clause 9.14 of Chapter 9.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Not for statutory use.

Attach this checklist to the front of the Hydraulic Structures Report. For each item, enter Yes, No or Not applicable, and the drawing or page where it is shown.

Section A — Project

Table 9F.1 — Form 9-1, Section A: project

Field	Entry
Project name and location	_____
Local body, and Tier A or Tier B (Chapter 1)	_____
Applicant	_____
Designer (name and registration)	_____
Structures covered (culverts, channels, dissipators, outlets, screens, pumps, gates, inlets, manholes)	_____
Tailwater conditions used: MHWS, HAT + surge, sea-level allowance, joint case (Clause 9.2.1)	_____
Date and revision	_____

Section B — Contents required by Clause 9.14

Table 9F.2 — Form 9-1, Section B: contents required by Clause 9.14

No.	Item	Clause	Provided (Yes / No / NA)	Drawing or page
1	Culvert calculations (HDS-5 or HY-8 output) with HWi, HWo, outlet velocity, tailwater, overtopping and the blockage check	9.4, 6.12		
2	Channel water-surface profiles, velocities, Froude numbers and freeboard	9.5		
3	Outlet protection and dissipator design, with riprap sizing	9.6		
4	Detention outlet stage–discharge tables and the emergency spillway check	9.7		
5	Screen and rack design with the 50% blocked check	9.8		
6	Pump station design report: inflow hydrograph, storage, pump and system curves, wet-well volume, controls, power supply and the Chapter 5 outfall-closed check (with Annex 9C)	9.9		
7	Gate and valve selection with head-loss data	9.10		
8	Inlet, grate and cover load classes	9.11		
9	Exposure class and materials statement	9.13		

Section C — Compliance checks

Table 9F.3 — Form 9-1, Section C: compliance checks

No.	Check	Limit	Clause	Complies (Yes / No / NA)
C1	Climate factor included in design flows	Clause 2.8	9.2.2	
C2	Blocked condition checked where debris can block; safe overflow route	—	9.2.5	
C3	Pipes and box drains: design n (box drains ≥ 0.015); supercritical pipes: jump located in a structure, air venting	—	9.3.1, 9.3.4	
C4	Pipe size does not decrease downstream; joins only at manholes or junction chambers; no change of direction over 90°	—	9.3.5	
C5	Culvert design headwater = greater of HWi and HWo	—	9.4.1	
C6	Culvert headwater at the design AEP	HW/D ≤ 1.2; ≥ 0.3 m below shoulder edge	9.4.5	
C7	Road overtopping weir coefficient	Cd ≤ 1.66	9.4.6	
C8	Culvert inverts match the drain bed; headwalls, wingwalls and cut-off walls	—	9.4.8(c), 9.4.9	
C9	Channel Froude number at the design flow	≤ 0.8; none between 0.8 and 1.2	9.5.2	
C10	Channel velocity	Table 9.2	9.5.3	
C11	Channel slope in engineered channels	≤ 0.4% natural, ≤ 0.2% concrete, or drops / chute	9.5.4	
C12	Side slopes; low-flow channel where bed width ≥ 3 m; uplift or weep holes	—	9.5.6, 9.5.7, 9.5.9	
C13	Outlet protection where outlet velocity exceeds Table 9.2	Table 9.3 apron, or basin	9.6.1, 9.6.3	
C14	Single vertical drop height	≤ 1.5 m	9.6.6(c)	
C15	Detention primary outlet: no moving parts; minimum orifice	≥ 50 mm with screen	9.7.1	
C16	Emergency spillway	1% AEP, outlet blocked, basin full, 0.3 m freeboard; 0.2% AEP for high-hazard basins	9.7.6	
C17	Outlet pipe through embankment; anti-vortex plate where needed	—	9.7.7, 9.7.8	
C18	Conduit-entrance racks	Spacing 75 mm to half the smallest conduit dimension; area ≥ 4×; ≤ 1.0 m/s at 50% blocked	9.8.2	
C19	Detention orifice screens	Openings smaller than the orifice; open area ≥ 10×	9.8.5	
C20	Pump station	Annex 9C checklist complete	9.9	
C21	Gates on outfalls below the 1% AEP still-water level; secondary closure; corrosion-resistant fittings	—	9.10	
C22	Grate load classes, parallel openings ≤ 25 mm, kerb openings ≥ 100 mm × 1.0 m, locked covers	—	9.11	
C23	Manhole sizes (Table 9.4), 600 mm access openings, drops, benching, surcharge covers	—	9.12	

No.	Check	Limit	Clause	Complies (Yes / No / NA)
C24	Exposure class (Table 9.5); chloride and sulphate tests in coastal and delta areas; sulphide protection; metalwork within 5 km of the coast	—	9.13	

Section D — Variances

Table 9F.4 — Form 9-1, Section D: variances

Clause	Requirement not met	Reason and proposed alternative	Approved by and date
_____	_____	_____	_____

Section E — Signatures

The report shall be prepared and signed by a registered professional, and the Professional Design Certificate (Form 10-3) attached (Clauses 10.6.1 and 10.6.3 of Chapter 10). For a facility with an embankment, a high-hazard basin, a pumped facility, a tide gate or a regional facility, the signatory shall be an empanelled Stormwater Design Professional, and a second, independent empanelled professional shall sign as checker (Clauses 10.6.1(c), (d) and 10.6.4).

Table 9F.5 — Form 9-1, Section E: signatures

Role	Name	Registration or empanelment no.	Signature	Date
Designer	_____	_____	_____	_____
Independent checker (Clause 10.6.4, where applicable)	_____	_____	_____	_____

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0.

ANNEXES TO CHAPTER 10

Submittals, Review, Operation & Maintenance and Enforcement

These annexes support Chapter 10. They give the specification and user-guide outline of the compliance calculator, the GIS data specification, the maintenance access dimensions that the Manual sets, model O&M plans for each facility type, model empanelment criteria, and the eight forms that Chapter 10 requires. The Design Rainfall Sheet (Form 2-1) is in the annexes to Chapter 2. The forms are draft forms. The model texts are for consideration by a competent authority and are not in force.

Annex 10A — Compliance calculator and user guide

Supports Clauses 10.4.1(g), 10.5 and 10.9.1(b) of Chapter 10.

> **Status.** The compliance calculator does not exist yet. Clause 10.5.1 requires the State Stormwater Technical Committee to issue and maintain it. This annex is the specification the calculator shall meet and the outline of its user guide. Until the calculator is issued, the designer completes Form 10-2 by hand from the calculations of Chapters 4, 5 and 8, enters "not issued" as the calculator version, and attaches the calculations.

10A.1 General requirements

The calculator shall:

- (a) be a spreadsheet or web tool, published free of charge with its user guide and the source of every table in it (Clause 10.5.4);
- (b) print its version number and release date on every output page and every exported file (Clause 10.5.3);
- (c) keep a public version log. A new version applies to new submittals 6 months after release; a submittal keeps one version from Stage 1 to Stage 4 unless the Applicant elects a later one (Clause 10.5.3);
- (d) work for one site at a time, with one or more drainage areas, each draining to its own outfall (Clause 4.5.1.3);
- (e) never apply a value that the Manual has not published. Where a table it needs has not been issued (for example Table 4.3 of Annex 4B, or the S-2 discharge tables of Annex 1C), the calculator shall show that route as "not available" rather than estimate the value; and
- (f) export its inputs and results as a machine-readable file (CSV or JSON) as well as Form 10-2, so that the Authority can load them into the facility register (Clause 10.17.1(b)).

10A.2 Inputs

Table 10A.1 — Calculator inputs

No.	Input	Unit	Source
I1	Authority, tier (A or B) and file number	—	Clause 1.6

No.	Input	Unit	Source
I2	Site area; land disturbance area	ha	Clause 1.5.2
I3	New impervious area; replaced impervious area; existing impervious area before demolition	m²	Clauses 1.5.2, 1.5.6, 4.5.6.1
I4	Development type: new, redevelopment, linear public work, single dwelling	—	Clauses 1.5.6–1.5.8
I5	Flood-sensitive catchment (yes / no), and basin with an approved BSMP (yes / no)	—	Clauses 4.5.3, 4.13, 11.12
I6	Rainfall zone	—	Annex 2A
I7	Retention depth P_R; 3-month ARI 24-hour depth P_3m	mm	Annexes 2E, 8A
I8	Design rainfall depths by duration (1–72 h) and AEP, with the climate factor, as recorded on Form 2-1	mm	Clauses 2.5, 2.8, 2.13
I9	15-minute intensities i_10 and i_2, with the climate factor (fixed-coefficient route only)	mm/h	Clause 4.14.4
I10	Drainage areas: for each, area, land cover, impervious percentage, HSG and curve number, pre- and post-development	ha, %, —	Clauses 3.3, 3.7, 3.9
I11	Areas preserved under a registered covenant	ha	Clause 4.4.7
I12	Seasonal high groundwater depth below the base of each retention practice	m	Clauses 4.4.3(c), 8.8
I13	Measured infiltration rate at each practice	mm/h	Clause 4.4.3
I14	Retention volume provided, by practice; reuse demand where retention is by reuse	m³, m³/d	Clauses 4.4.3, 4.4.5
I15	Route: full hydrograph method (Clause 4.6); design-aid S-1 (Clause 4.14.2); PSD S-2 (Clause 1.7); fixed-coefficient (Clause 4.14.3); constrained redevelopment (Clause 4.5.6)	—	Clauses 1.7, 4.5.6, 4.6, 4.14
I16	For the full method: model results for each AEP and duration — pre- and post-development peaks, storage used, retention credit found by routing	m³/s, m³	Clauses 4.4.6.2, 4.6
I17	Outlet: low-flow orifice diameter or least slot dimension; drawdown times from routing	mm, h	Clauses 4.7, 4.8.2
I18	Outfall: tidal or non-tidal; outfall-closed check result	—	Chapter 5
I19	Fee-in-lieu unit rate notified for the basin, where the route applies	₹/m³	Clause 4.13.3
I20	Stage of submittal (1, 2 or 4) and, at Stage 4, the as-built values of I14, I16 and I17	—	Clauses 10.5.2, 10.9.1(b)

10A.3 Calculations, by clause

Table 10A.2 — Calculations the calculator shall perform

Step	Calculation	Clause
C1	**Applicability.** Chapters 4 and 5 apply where land disturbance ≥ 0.1 ha or new plus replaced impervious area ≥ 500 m², over the common plan of development. Report exemptions. Flag that the universal provisions apply at every size, and that RWH applies to buildings on plots above the threshold of rule 78(4) of the CGBVN 1984, reported as 500 m² (Rajya Sabha answer of 2019; the amendment of 17 February 2009 substitutes the figures "500" and "300" and the threshold is [TBC]), by the systems of Appendix ङ (N), to government buildings with roofs of more than 1,500 sq ft (new) or 5,000 sq ft (existing) (General Administration Department order of 31 May 2016), to integrated townships (Appendix ण) and to every development in Nava Raipur (Naya Raipur Development Plan-2031, §10.3.2); recharge is credited, with pre-treatment, only after a hydrogeologist's check in the critical and semi-critical urban blocks (Dharsiwa in Raipur district, Durg and Dhamdha), with a cavity check in the Chandi and Charmuria limestone of the Raipur–Durg–Rajnandgaon belt and a fluoride and arsenic check where the groundwater carries them, and not at all on kanhar, on floodplains, where the seasonal high water table is within the separation of Chapter 8, on mine-disturbed ground, overburden dumps or ash ponds, or within a mine's dewatering zone.	1.5.2–1.5.8
C2	**Process flags.** Stage 0 mandatory (≥ 2 ha, flood-sensitive, variance or regional route, tidal outfall or waterbody); Stages 1 and 2 may be combined (< 1 ha, not flood-sensitive); empanelled signatory required; independent check required.	10.3.2, 10.3.3, 10.6.1, 10.6.4
C3	**Route eligibility.** Tier B simplified method: ≤ 2 ha, or ≤ 0.5 ha in a flood-sensitive catchment, and only once Table 4.3 is published. Fixed-coefficient route: ≤ 1 ha and none of the exclusions of Clause 4.14.3. Constrained redevelopment: existing ≥ 60% impervious and ≤ 1 ha. Regional detention or fee-in-lieu: only in a basin with an approved BSMP.	4.5.6.1, 4.13.1, 4.14.1, 4.14.3, 11.12.1(b)
C4	**Retention volume.** RRV = 10 × Q_R × A, with Q_R the SCS-CN runoff from P_R for each sub-area at AMC II, summed over sub-areas. The larger of the Ia = 0.2S and Ia = 0.05S results governs. The Rv form (RRV = 10 × P_R × Rv × A, Rv = 0.05 + 0.009 × I) is offered only for sites < 0.5 ha and the Tier B method. No climate factor.	4.4.2, 3.8.5(c), 2.8.3
C5	**Preservation credit.** Preserved areas are modelled at their pre-development CN in the post-development condition.	4.4.7
C6	**Retention checks.** Design infiltration rate ≤ half the measured rate; groundwater ≥ 1.0 m below the base; drawdown of the retained volume within 72 h at the design rate (or by the stated reuse demand). A practice that fails any check gets no credit; any shortfall is reported and treated as WQv.	4.4.3–4.4.5, 4.4.6.4
C7	**Water quality volume.** P_wq = max(P_ret, P_3m); WQv = 10 × Q_wq × A by SCS-CN, or (P_wq / 1000) × Rv × A (A in m²) for sites < 0.5 ha and the Tier B method. The retained volume counts toward the WQv in full. No climate factor.	8.4.1–8.4.4, 3.8.5(c)
C8	**Retention credit.** Tier A: the credit found by routing is entered (I16); the calculator does not compute it. Tier B: V_credit = C_R × RRV provided, with C_R = 0 until Annex 4B is published.	4.4.6
C9	**Design-aid route (S-1).** V = SSR × A − V_credit; Q_PSD,T = PSD_T × A for the 2-yr and 100-yr stages, from Table 4.3. Shown as "not available" until Table 4.3 is published.	1.7.2, 4.14.2
C10	**Permissible site discharge (S-2).** Q = PSD (m³/s/ha) × A for each AEP, from the S-2 tables. Shown as "not available" until they are issued.	1.7.2, 1.7.3(e)
C11	**Fixed-coefficient route.** Q_cap,10 = 2.78 × C_10 × i_10 × A and Q_cap,2 = 2.78 × C_2 × i_2 × A (L/s); V_100 and V_2 = max over d of [10 × C_v × P(d) × A − 1.8 × Q_cap × d] for d = 1 to 72 h, with pervious C ≥ 0.50 for d ≥ 12 h; V = max(V_100, V_2); retention added without credit. Coefficients from Table 4.3A (provisional).	4.14.4–4.14.7
C12	**Release-rate screen (full method).** For each AEP and duration: Q_post ≤ 0.9 × Q_pre (standard), or the two mixed-frequency caps (flood-sensitive). Report the governing duration and margin.	4.5.2, 4.5.3
C13	**Constrained redevelopment.** Q_post ≤ 0.5 × Q_existing for 2-, 10- and 100-yr at each duration; shortfall volume = greenfield storage − storage provided; fee = shortfall × unit rate.	4.5.6.2, 4.5.6.3
C14	**Minimum orifice.** Flag any orifice < 50 mm. Where a cap is below the discharge of a 50 mm orifice at the corresponding stage, report "50 mm orifice deemed to comply".	4.5.5.1, 4.8.2
C15	**Drawdown.** Report the detention drawdown (90% of the 10-yr stored volume within 48 h), the WQv release time (24–48 h) and the retention emptying time (≤ 72 h).	4.7
C16	**Fee-in-lieu.** Fee = detention volume required on site × notified unit rate.	4.13.3

Step	Calculation	Clause
C17	**Model screen.** Where the full method is used, compare the model results (I16) with the calculator's screen values (C9 to C11, where available) and flag a difference above the tolerance stated in the user guide.	10.5.2(c)
C18	**As-built re-run.** At Stage 4, repeat C4 to C17 with the as-built values and report each change from Stage 2.	10.9.1(b), 10.9.2

10A.4 Outputs

The calculator produces:

- (a) **Form 10-2**, filled in, with the calculator version and release date on every page;
- (b) a machine-readable export of every input and result of Tables 10A.1 and 10A.2; and
- (c) a list of the flags raised (C1 to C3, C6, C14, C17), each with the clause on which it is based (Clause 10.7.7).

10A.5 Testing before release

Before each release the State Stormwater Technical Committee shall check that the calculator reproduces, within rounding, the worked examples of the Manual that use the same method, including the illustrative examples of Clauses 4.6.7 and 4.14, and the benchmark problems of Annex 3C where they apply. The test results shall be published with the version log.

10A.6 Outline of the user guide

The user guide shall contain at least:

1. Purpose, status and limits of the calculator: a summary and screening check outside the design-aid route (Clause 10.5.2(c)).
2. How to obtain the current version, and the version rule of Clause 10.5.3.
3. Each input of Table 10A.1, with where to find it in the Manual and its annexes.
4. Each calculation of Table 10A.2, with its equation and clause.
5. The tolerance used in step C17, and what the drainage report must explain where it is exceeded (Clause 10.5.2(c)).
6. How to read and sign Form 10-2, and what to attach.
7. At least one worked example for each route, labelled "Illustrative — not design values".
8. The sources of every table in the calculator (Clause 10.5.4), and the version log.

Annex 10B — GIS data specification and attribute schema

Supports Clauses 10.4.4, 10.14.1(d), 10.14.3 and 10.17 of Chapter 10.

10B.1 Format and reference systems

- (a) **Format.** One OGC GeoPackage (.gpkg) per submittal is preferred. Shapefiles are accepted, one per layer, zipped with all component files. Field names shall be as in Tables 10B.2 to 10B.6; shapefile names are limited to 10 characters, and the names below meet that limit.
- (b) **Horizontal reference.** WGS 84 geographic (EPSG:4326), or WGS 84 / UTM zone 44N (EPSG:32644), statewide, including for Jashpur Nagar and the eastern part of Jashpur district, which lie in zone 45N, whichever contains the site (Clause 10.4.4(c)). One system shall be used for all layers.

- (c) **Vertical reference.** All levels shall be in metres relative to a named vertical datum, stated in the metadata. The metadata shall also state the relationship between that datum and the river datums of the CWC gauges on the receiving rivers, of WRD's dam, barrage, reservoir and tank levels and of the levels of the lakes and reservoirs of Nava Raipur, and, for an outfall to a mine void, of the lessee's mine survey used in Chapter 5 (Clause 10.4.4(d)).
- (d) **Accuracy.** Positions shall come from the site survey or the approved drawings. At Stage 4 and at handover, positions and levels shall come from the as-built survey (Clause 10.9.1(a)).
- (e) **File name.** `<Authority file number>_<stage>_<yyyymmdd>.gpkg`, for example `RMC-BP-2026-0123_S2_20261015.gpkg`.
- (f) **Identifiers.** Every feature has an identifier unique within the submittal (FAC_ID, OUT_ID, CAT_ID, FLP_ID). The Authority replaces them with register identifiers on acceptance and returns the mapping to the Applicant.

10B.2 Layers

Table 10B.1 — Layers

Layer	Geometry	Content	Required at
SW_SITE	Polygon	Site boundary	Stages 1, 2, 4
SW_CATCH	Polygon	Catchment boundaries draining to and through the site, pre- and post-development (Clause 10.4.1(b))	Stages 1, 2, 4
SW_FAC	Polygon (point for buried items smaller than 5 m across)	Every stormwater facility: retention, detention, water quality, pumping, gate, conveyance element designed under this Manual	Stages 2, 4; handover
SW_OUTF	Point	Every outfall from the site, and every public outfall	Stages 1, 2, 4; handover
SW_FLOW	Line	Major-system flow paths through and from the site, and natural drainage paths (Clause 10.15.1(g))	Stages 1, 2, 4
SW_CONV	Line	Drains and pipes designed under Chapter 6	Stage 4 for public works; handover
SW_META	Table (no geometry)	Metadata of Table 10B.6	Every stage

10B.3 Attribute schema

Types: T = text, I = integer, R = real, D = date (ISO 8601, yyyy-mm-dd). "Req." marks fields that shall not be empty; other fields are filled where they apply.

Table 10B.2 — Attributes of SW_FAC (facilities)

Field	Type	Unit	Req.	Content	Source
FAC_ID	T	—	Yes	Facility identifier	10B.1(f)
FAC_TYPE	T	—	Yes	Code from Table 10B.7	Clause 4.10, Table 10.3
FAC_NAME	T	—		Name on the drawings	—
OWNER_TYP	T	—	Yes	PRIVATE / ASSOC / DEVELOPER / ULB / DEPT	Clauses 10.10, 10.14
OWNER_NM	T	—	Yes	Name of a corporate owner or association only; for an individual, "private individual" (Clause 10.17.3)	Clause 10.17.3
CTRL_AREA	R	ha	Yes	Area draining to the facility	Clause 4.5.1.3

Field	Type	Unit	Req.	Content	Source
V_RET	R	m³		Retention volume provided	Clause 4.4
V_WQ	R	m³		Water quality volume provided	Clause 8.4
V_DET	R	m³		Detention volume at the top design level	Clause 4.6
Q_2	R	L/s		Release at the 50% AEP (2-yr) design level	Clause 4.5
Q_10	R	L/s		Release at the 10% AEP (10-yr) design level	Clause 4.5
Q_100	R	L/s		Release at the 1% AEP (100-yr) design level	Clause 4.5
ORIF_MM	R	mm		Low-flow orifice diameter or least slot dimension	Clause 4.8.2
INV_OUT	R	m		Outlet invert level	Clause 10.9.1(a)
TWL_DES	R	m		Top design water level	Clause 10.9.2(a)
SPILL_LV	R	m		Spillway or overflow crest level	Clause 4.9
PUMPED	T	—	Yes	Y / N	Clause 10.10.3
REGIONAL	T	—	Yes	Y / N	Clause 4.13
EMPANEL	T	—	Yes	Y / N: facility under Clause 10.6.1(c) or (d)	Clause 10.6.1
STATUS	T	—	Yes	Code from Table 10B.8	Clause 10.17.1(a)
APPR_DATE	D	—		Date of Stage 2 approval	Clause 10.3
ASB_DATE	D	—		Date the as-built certificate was accepted	Clause 10.9
DECL_REF	T	—		Registration reference of the Maintenance Declaration (Form 10-6)	Clause 10.10.1(e)
HAND_DATE	D	—		Date of the Asset Handover Certificate (Form 10-8)	Clause 10.14
LAST_INSP	D	—		Date of the latest inspection	Clause 10.12
INSP_RES	T	—		Code from Table 10B.9	Clause 10.17.1(a)
OUT_ID	T	—	Yes	Outfall to which the facility discharges	—

Table 10B.3 — Attributes of SW_OUTF (outfalls)

Field	Type	Unit	Req.	Content	Source
OUT_ID	T	—	Yes	Outfall identifier	10B.1(f)
RECEIVER	T	—	Yes	Receiving drain, channel, waterbody or sea, by name	Clause 4.5.1.3
RECV_TYPE	T	—	Yes	PUBLIC_DRAIN / CANAL / RIVER / TANK / SEA / OTHER	Chapter 5
INV_LEVEL	R	m	Yes	Outfall invert level	Clause 10.9.1(a)
SIZE_MM	R	mm	Yes	Diameter, or width × height written in the drawing	Chapter 6
TIDAL	T	—	Yes	Y / N	Chapter 5
GATE_TYPE	T	—		NONE / FLAP / TIDE / DUCKBILL / PENSTOCK / NRV	Clauses 5.11, 9.10
GATE_REQ	T	—	Yes	Y / N: invert below the 1% AEP still-water level	Clause 5.11.1
GPT	T	—		Y / N: gross pollutant trap fitted	Clause 8.10.1
Q_100	R	L/s		Design 1% AEP discharge	Chapter 5

Table 10B.4 — Attributes of SW_CATCH (catchments)

Field	Type	Unit	Req.	Content	Source
CAT_ID	T	—	Yes	Catchment identifier	10B.1(f)
CONDITION	T	—	Yes	PRE / POST / EXTERNAL (land outside the site draining through it)	Clause 10.4.1(b)
AREA_HA	R	ha	Yes	Area	—
IMPERV_PC	R	%	Yes	Impervious percentage	Clause 3.9
HSG	T	—	Yes	A / B / C / D, or mixed with area shares in NOTES	Clause 3.7
CN	R	—		Curve number, AMC II	Chapter 3
OUT_ID	T	—	Yes	Outfall to which it drains	Clause 4.5.1.3
FLOODSENS	T	—	Yes	Y / N	Clause 4.5.3
NOTES	T	—		Free text	—

Table 10B.5 — Attributes of SW_FLOW and SW_CONV (flow paths and conveyance)

Field	Type	Unit	Req.	Content	Source
FLP_ID	T	—	Yes	Line identifier	10B.1(f)
LINE_TYPE	T	—	Yes	OVERLAND / NATURAL / OPEN_DRAIN / COVERED_DRAIN / PIPE / CULVERT	Chapter 6
US_LEVEL	R	m		Upstream invert or ground level	Chapter 6
DS_LEVEL	R	m		Downstream invert or ground level	Chapter 6
SIZE_MM	R	mm		Pipe diameter, or drain width (depth in NOTES)	Clause 6.7.6
Q_DES	R	L/s		Design discharge	Chapter 6
AEP_DES	T	—		Design AEP, for example "10%" or "1%"	Clause 6.4
RESERVE_M	R	m		Drainage reserve width, where one applies	Clause 6.9.7
NOTES	T	—		Free text	—

Table 10B.6 — Metadata (SW_META)

Field	Content
FILE_NO	Authority file number
STAGE	1, 2, 4 or HANDOVER
H_CRS	EPSG code of the horizontal reference system
V_DATUM	Name of the vertical datum
TIDE_REL	Relationship of the vertical datum to the Chapter 5 river datums of the CWC gauges on the receiving rivers, of WRD's dam, barrage, reservoir and tank levels and of the levels of the lakes and reservoirs of Nava Raipur, and, for an outfall to a mine void, of the lessee's mine survey (for example, MSL = x.xx m on the site datum)
SURVEYOR	Name and licence number of the surveyor
SURV_DATE	Date of the survey
PREP_BY	Name and registration number of the certifying professional

Field	Content
CALC_VER	Compliance calculator version (Annex 10A), or "not issued"
DATE	Date the file was prepared

10B.4 Code lists

Table 10B.7 — Facility type codes (FAC_TYPE)

Code	Facility	Inspection row of Table 10.3
RET_SOAK	Soakaway, recharge pit or shaft	Retention / infiltration
RET_TRENCH	Infiltration trench or basin	Retention / infiltration
RET_BIO	Bioretention or rain garden	Retention / infiltration
RET_PERM	Permeable pavement	Retention / infiltration
RET_REUSE	Retention by reuse (tank with demand)	Underground or podium detention tank, gravity outlet
DET_BASIN	Surface detention basin	Surface detention basin or multi-use open space area
DET_OSR	Multi-use detention in open space, park or playfield	Surface detention basin or multi-use open space area
DET_ROOF	Rooftop detention	Underground or podium detention tank, gravity outlet
DET_PODIUM	Podium or landscape-deck detention	Underground or podium detention tank, gravity outlet
DET_PARK	Parking-lot detention	Surface detention basin or multi-use open space area
DET_TANK	Underground tank or vault, gravity outlet	Underground or podium detention tank, gravity outlet
DET_PUMP	Pumped detention or pump station	Pumped detention, pumps and controls
WQ_POND	Wet pond or constructed wetland	Surface detention basin or multi-use open space area
WQ_SWALE	Swale or filter strip	Private drains and inlets
WQ_GPT	Gross pollutant trap, first-flush device or oil-grit separator	Gross pollutant trap / first-flush device
GATE	Flap or tide gate, non-return valve	Flap / tide gate, non-return valve
CONV	Private drain, inlet or culvert	Private drains and inlets

Table 10B.8 — Status codes (STATUS)

Code	Meaning
PROPOSED	In a Stage 1 or Stage 2 submittal
APPROVED	Stage 2 approved; not yet built
ASBUILT	As-built certificate accepted (Clause 10.9)
HANDEDOVER	Accepted by the ULB (Clause 10.14)
ALTERED	Changed with the Authority's approval (Clause 10.3.5)
REMOVED	Removed with the Authority's approval

Table 10B.9 — Inspection result codes (INSP_RES)

Code	Meaning
OK	Working as designed; no defect
MINOR	Defect found and corrected; date in the inspection record
MAJOR	Defect affecting storage, outlet or safety; notice issued (Clause 10.15.2, step 1)
NOACCESS	Inspection could not be made
NOTFOUND	Facility missing, filled, covered or changed in use (Clause 10.15.1(c))

10B.5 Facility register extract

The public facility register of Clause 10.17.1(a) is an extract of SW_FAC: FAC_ID, FAC_TYPE, location (centroid), V_DET (and V_RET, V_WQ), Q_100 (and Q_2, Q_10), OWNER_NM, ASB_DATE, LAST_INSP and INSP_RES. It is published as CSV and GeoPackage under the licence of Clause 10.17.4. Personal data are not published (Clause 10.17.3).

Annex 10C — Maintenance access dimensions

Supports Clause 10.13 of Chapter 10.

Clause 10.13 sets access by function. This annex collects the access dimensions that the Manual already sets in other chapters, and lists the dimensions that it does not set. Where a dimension is not set by the Manual, the designer shall state it on the drawings, show that it suits the plant and equipment named in the O&M plan (Clause 10.11.1(h)), and mark it "to be confirmed in design".

Table 10C.1 — Access dimensions set by the Manual

Element	Requirement	Clause
Open drains with a top width of 2 m or more	Maintenance access strip at least 3.5 m wide along one bank, connected to a public road at intervals of not more than 500 m	6.9.4
Drainage reserves	Width not less than Table 6.9, based on the required drain size; where the access strip of Clause 6.9.4 is wider, the wider governs	6.9.7
Covered drains, minimum section	0.6 m × 0.6 m clear	6.7.6
Covered drains, removable covers	Panels at not more than 10 m centres, each at least 0.6 m long and spanning the full clear width, liftable by two workers or a hand-operated device	6.13.2(a)
Covered drains, access chambers	Clear opening at least 0.6 m × 0.6 m at every junction, bend and change of section, and at not more than 30 m centres on straight runs	6.13.2(b)
Manholes and access chambers on public drains	Clear access opening at least 600 mm	9.12.3
Inlet chambers on public roads	Silt sump at least 300 mm deep below the outlet pipe invert, cleanable from the surface	6.6.7
Underground tanks and vaults	Access openings over the outlet chamber and over every inlet; in tanks over 500 m², not more than 30 m apart; floor falling at least 1% to a sump at the outlet	4.10.2(b), (c)
Detention outlets serving more than 1 ha	A means of isolating the outlet (stop-log slot or penstock)	4.8.6

Element	Requirement	Clause
Outfall gates	Stop-log slot or penstock upstream on outfalls larger than 600 mm (Chapter 5) and on outfalls of 900 mm or more or serving a pumped or storage system (Chapter 9); the gate chamber accessible at all tides, with its access above HAT + surge + sea-level allowance	5.11.3(d), (e); 9.10.3
Pump stations	Maintenance vehicle access usable in the 1% AEP event; hard standing and a quick-coupling connection for mobile pumps; clear space between adjacent pumps or motors of 0.6 m (small and medium units) or 1.0 m (large units)	9.9.6, 9.9.10, 9.9.12
Pretreatment devices	Cleanable without entering or disturbing the main BMP	8.5.4
Screens at outfalls, culvert inlets and pump sumps	Cleanable from a safe platform, with vehicle access	8.10.3(d)
Infiltration trenches	Observation well to the base of each trench	8.7.6(c)
Publicly accessible storage	Side slopes no steeper than 4H:1V, with at least a quarter of the perimeter at 6H:1V or flatter	4.15.1(c)
Wet basins	Safety bench at least 3 m wide, no more than 0.3 m below normal water level	4.15.2
Fencing	At least 1.2 m high, non-climbable, with a lockable gate, where Clause 4.15.3 requires it	4.15.3

Table 10C.2 — Access dimensions not set by the Manual (to be confirmed in design)

Element	What the designer shall state	Clause setting the function
Vehicle access route to the inlet, outlet and base of a basin, pond or open channel	Width, gradient, turning space and design load for the plant in the O&M plan	10.13.2
Maintenance strip beside a basin or pond	Width along at least one side	10.13.2
Ramp into a basin	Width, gradient and surface for the sediment-removal plant	10.13.2
Access openings of private tanks	Clear size of each opening over the inlet, outlet and each chamber	10.13.3(a)
Ladders and steps	Type and spacing; ventilation for confined-space entry	10.13.3(b)
Headroom in underground tanks	Clear height for manual cleaning	10.13.3(d)
Reach to orifices and screens	How the outlet is cleaned without entering a full facility	10.13.4

> **Commentary.** APWA 5600 (2006, §5608) required a maintenance access strip of 15 ft (about 4.5 m) for detention facilities. The Manual did not adopt a fixed width for private sites, because plot sizes in Chhattisgarh cities vary widely (Clause 10.13, Commentary). The State Stormwater Technical Committee may add standard values to Table 10C.2 when it issues the standard drawings of Annexes 6C and 9B.

Annex 10D — Model O&M plans by facility type

Supports Clauses 10.11, 10.12 and 10.13 of Chapter 10.

The model plans below are starting points. The designer adapts them to each facility and completes the items marked "_____". Inspection frequencies are the minimum frequencies of Table 10.3; a plan may set more frequent inspection, never less. Where this annex leaves a value to the designer, the Manual does not set it.

10D.1 Contents of every O&M plan

Every plan shall contain the items of Clause 10.11.1:

Table 10D.1 — Contents of an O&M plan

Item	Content	Clause
1	Inventory and plan drawing of each facility, with design levels, volumes and outlet details (from SW_FAC, Annex 10B)	10.11.1(a)
2	Routine tasks and their frequency (Tables 10D.2 to 10D.8)	10.11.1(b)
3	Trigger for sediment removal, as a depth (mm) or a percentage of design volume: _____	10.11.1(c)
4	Inspection checklist matching Form 10-7	10.11.1(d)
5	For pumps, gates and valves: operating rules, test schedule, standby power, and actions on a cyclone or heavy-rainfall warning	10.11.1(e)
6	Actions against standing water and mosquito breeding, consistent with the drawdown times of Clause 4.7	10.11.1(f)
7	Safety: confined spaces, deep water, public access	10.11.1(g)
8	Access routes and equipment (Annex 10C)	10.11.1(h)
9	Annual cost estimate (₹ _____ per year) and who pays it	10.11.1(i)
10	Records to be kept, and for how long: _____ years (pump records at least 5 years, Clause 9.9.8(c))	10.11.1(j)

10D.2 Model plans

Table 10D.2 — Retention and infiltration (soakaways, trenches, bioretention, permeable paving)

Task	Frequency	Standard or trigger	Clause
Inspect surface, inlets, pretreatment and overflow	Every 6 months; before 31 May; after each heavy-rainfall day	No blockage; overflow clear	Table 10.3
Check drawdown at the observation well or surface	At each inspection after rain	Empties within 72 h of the end of rain; if not, investigate clogging	4.4.5, 8.7.6(c)
Clean pretreatment (forebay, filter strip, silt trap)	When the sediment trigger (item 3) is reached	Pretreatment cleaned without disturbing the main BMP	8.5.3, 8.5.4
Bioretention: mulch, plants, ponding	Every 6 months	Ponding drains within 48 h; dead plants replaced; mulch 50–75 mm	8.7.2
Permeable paving: surface	Every 6 months	Sweep or vacuum; no resurfacing with impermeable material	8.7.5(e)
Keep the contributing area stable	Each inspection	Bare soil stabilised; no sediment washing in	10.8.3(b)
Keep the facility in place and in use	Each inspection	Not filled, covered, paved or built over without approval	10.10.1(d)

Table 10D.3 — Surface detention basin or multi-use open space area

Task	Frequency	Standard or trigger	Clause
Inspect inlets, outlet, orifice, trash rack and spillway	Every 6 months; before 31 May; after each heavy-rainfall day	Orifice and rack clear; spillway unobstructed	Table 10.3, 4.8, 4.9
Remove debris and sediment	When the sediment trigger is reached, and after events	Design volume restored	10.11.1(c)
Check that the basin drains	After rain	No low point ponds for more than 72 h	4.15.7
Vegetation	As set in the plan	Grass cut; no woody growth on embankments or in the outlet zone	10.11.1(b)
Safety features	Every 6 months	Signs in Hindi and English and a depth marker in place; fences and gates sound where required	4.15.3, 4.15.5
Embankment (if any)	Every 6 months and after events	No cracks, slips, seepage or animal burrows	4.9.6

Table 10D.4 — Underground or podium detention tank, gravity outlet

Task	Frequency	Standard or trigger	Clause
Inspect from the access openings: inlets, outlet, orifice, screen, sump, vents	Every 6 months; before 31 May; after each heavy-rainfall day	Clear; vents open	Table 10.3, 4.10.2
Clean the sump and floor	When the sediment trigger is reached	Floor falls freely to the sump	4.10.2(c)
Inlet silt trap or GPT	Every 3 months (Table 10.3 row for GPTs)	Cleaned	4.10.2(e)
Overflow route	Each inspection	Operates when the outlet is blocked; route to the overland path unobstructed	4.9.5
Confined-space entry	Each entry	Entry procedure stated in the maintenance declaration followed; covers locked after	4.15.6
Use of the space	Each inspection	Not used for parking or storage; volume not reduced	10.10.1(d)

Table 10D.5 — Pumped detention, pumps and controls

Task	Frequency	Standard or trigger	Clause
Test run of each pump, duty and standby	Monthly	Each pump starts on level control; roles alternate	Table 10.3, 9.9.4(c)
Full duty test	Before 31 May	Station meets its design duty point	Table 10.3, 10.9.3
Standby power	Monthly with the test run; full test before 31 May	Automatic changeover works; fuel for at least 72 h of all duty pumps on site	9.9.9
Level sensors, float back-up, alarms, telemetry	Monthly	High-level and fault alarms reach the control room	9.9.8
Screens and silt trap	Every 3 months and after events	Clean	9.9.7
Records	Continuous	Sump levels, run hours and power status logged; kept at least 5 years	9.9.8(c)

Task	Frequency	Standard or trigger	Clause
Warning actions	On an IMD heavy-rainfall or cyclone warning	Actions written in the plan: _____ (for example staff on site, fuel topped up, mobile pump connection checked)	10.11.1(e), 9.9.12

Table 10D.6 — Flap or tide gate, non-return valve

Task	Frequency	Standard or trigger	Clause
Inspect and operate	Every 3 months; after each heavy-rainfall day	Opens at a small head; no debris, sand or growth holding it	Table 10.3, 9.10.4
Seal test	Before 31 May	Seals against the receiving water	Table 10.3, 10.9.3
Beach, estuary and creek outfalls	After every high-tide or surge event in the monsoon season	Sand and debris cleared	9.10.5(b)
Isolation	Before 31 May	Stop-log slot or penstock operable	9.10.3
Hinges, pins and fixings	Before 31 May	No corrosion that would stop the gate working	9.10.6
Property non-return valves	Every 3 months	Cleaning eye accessible; valve seals	5.11.4

Table 10D.7 — Gross pollutant trap or first-flush device

Task	Frequency	Standard or trigger	Clause
Inspect and empty	Every 3 months; before 31 May; after each heavy-rainfall day	Emptied before the capture chamber is full; bypass clear	Table 10.3, 8.4.6
Screens	Each inspection	Clean; overflow path clear	8.10.3(b)
Disposal	Each cleaning	Material taken to an authorised solid-waste facility	—

Table 10D.8 — Private drains and inlets

Task	Frequency	Standard or trigger	Clause
Inspect and clean drains, inlets and silt sumps	Every 6 months; before 31 May	Free of silt and debris	Table 10.3
Check for sewage connections	Each inspection	None; any cross-connection removed	1.5.4, 8.9
Overland flow paths	Each inspection	Not obstructed by walls, fill or stored material	6.9.2, 10.15.1(g)

Annex 10E — Empanelment criteria for Stormwater Design Professionals and reviewers

Supports Clauses 10.6, 10.7.4 and 11.4.3 of Chapter 10.

Model text for consideration by a competent authority. Not in force. Clause 10.6.2 assigns the issue of empanelment criteria to T&CP. The text below is a model. Values shown as "[ ]" are for the competent authority to set; the Manual does not set them.

10E.1 Model criteria

1. **Lists.** The competent authority shall keep three public lists:
    - (a) **Stormwater Design Professionals**, who may sign designs under Clause 10.6.1 and act as independent checkers under Clause 10.6.4;
    - (b) **Empanelled Reviewers**, who may review submittals for an Authority under Clause 10.7.4; and
    - (c) a **basin modelling endorsement**, held by a Stormwater Design Professional who may prepare or direct a BSMP under Clause 11.4.3.
2. **Eligibility for list (a).** An applicant shall:
    - (a) hold a degree in civil engineering, or an equivalent qualification recognised by the competent authority (Clause 10.6.2);
    - (b) be registered as a professional under the Chhattisgarh Bhumi Vikas Niyam 1984 (Clause 1.4);
    - (c) have at least [ ] years of experience in drainage design after graduation, shown by a list of at least [ ] projects with the applicant's role in each (Clause 10.6.2); and
    - (d) have completed a training course on this Manual, with an assessment, within the [ ] years before application (Clause 10.6.2).
3. **Eligibility for list (b).** As for list (a), with at least [ ] years of experience, of which at least [ ] years in the design or review of drainage under this Manual or an equivalent standard.
4. **Basin modelling endorsement (c).** A Stormwater Design Professional who shows experience in basin-scale 1D–2D modelling, including calibration, on at least [ ] studies (Clause 11.4.3).
5. **Independence of a checker.** A checker under Clause 10.6.4 shall not be employed by, or a partner of, the designer's firm or the Applicant, and shall not have worked on the design.
6. **Conflict of interest of a reviewer.** An Empanelled Reviewer shall not review a site on which the reviewer, or the reviewer's firm, has worked in any capacity (Clause 10.7.4), and shall declare any other interest before accepting a review. The Authority remains the decision-maker (Clause 10.7.4).
7. **Application.** The application shall include proof of degree and registration, the project list with references, the training certificate, and a signed declaration that the information is true.
8. **Term and renewal.** Empanelment is valid for [ ] years. Renewal requires evidence of continuing practice and of any refresher training issued after a revision of the Manual (Clause 1.12).
9. **Suspension and removal.** The competent authority may suspend or remove an empanelled person after a false or negligent certificate (Clause 10.6.5), on a report under step 6 of Table 10.4, or on loss of registration. The person shall be given notice and an opportunity to be heard. Every suspension and removal shall be published on the list.
10. **Publication.** The lists shall be published and kept current, with each person's name, registration number, list, endorsement and validity date.

Form 10-1 — Drainage Report Checklist

Supports Clauses 10.4.1 to 10.4.4 and 10.7.1 of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Lodge with every Stage 1 and Stage 2 submittal. For each item give the page or sheet where it is addressed, or the reason it does not apply. The Authority does not start technical review without a completed Form 10-1 (Clause 10.4.3).

Table 10F.1 — Form 10-1, Section A: Submittal

No.	Field	Entry
A1	Authority and file number	_____
A2	Site name and address; survey numbers	_____
A3	Stage: 1 / 2 / combined 1 and 2 (Clause 10.3.3) / public work DPR or tender (Clause 10.3.4)	_____
A4	Applicant	_____
A5	Certifying professional (name, registration and empanelment number)	_____

Table 10F.2 — Form 10-1, Section B: Stage 1 contents (Clause 10.4.1)

No.	Item	Clause	Page / sheet	Not applicable: reason
B1	Form 1-1; Form 1-2 where a variance or waiver is sought	10.4.1(a)	_____	_____
B2	Site plan: existing and proposed levels, impervious areas, catchment boundaries to and through the site	10.4.1(b)	_____	_____
B3	Soils (HSG), seasonal high groundwater, infiltration tests or test plan	10.4.1(c)	_____	_____
B4	Design Rainfall Sheet (Form 2-1)	10.4.1(d)	_____	_____
B5	Pre-development condition and hydrologic method	10.4.1(e)	_____	_____
B6	Order of measures: areas preserved, retention method, detention concept, conveyance	10.4.1(f)	_____	_____
B7	Preliminary retention, WQ and detention volumes and release rates, with Form 10-2	10.4.1(g)	_____	_____
B8	Downstream extent and tailwater case	10.4.1(h)	_____	_____
B9	Waterbodies, setbacks and the 1% AEP level	10.4.1(i)	_____	_____
B10	Land reserved for facilities and maintenance access	10.4.1(j)	_____	_____
B11	Professional Design Certificate (Form 10-3)	10.4.5	_____	_____

Table 10F.3 — Form 10-1, Section C: Stage 2 contents (Clause 10.4.2)

No.	Item	Clause	Page / sheet	Not applicable: reason
C1	Updated Stage 1 contents (Section B)	10.4.2	_____	_____
C2	Final calculations for every storage duration and AEP, with the outfall-closed check	10.4.2(a)	_____	_____

No.	Item	Clause	Page / sheet	Not applicable: reason
C3	Stage–storage–discharge tables; emergency spillway design	10.4.2(b)	_____	_____
C4	Hydraulic design of conveyance, inlets and structures	10.4.2(c)	_____	_____
C5	Water quality design; first-flush device or GPT where required	10.4.2(d)	_____	_____
C6	Construction drawings, details and specifications of every facility	10.4.2(e)	_____	_____
C7	Construction sequence, with protection of retention and infiltration areas	10.4.2(f)	_____	_____
C8	Hold-point schedule (Form 10-4, Section A)	10.4.2(g)	_____	_____
C9	Draft O&M plan and draft Maintenance Declaration (Form 10-6)	10.4.2(h)	_____	_____
C10	Model input files and GIS data	10.4.2(i)	_____	_____
C11	Certificates (Form 10-3, signed by designer and, where required, checker)	10.4.2(j)	_____	_____

Table 10F.4 — Form 10-1, Section D: Digital submittal (Clause 10.4.4)

No.	Item	Clause	Provided (yes / no)	Detail
D1	Reports and drawings as searchable PDF	10.4.4(a)	_____	_____
D2	Models in native, runnable format; software name and version	10.4.4(b)	_____	_____
D3	GIS data to Annex 10B; horizontal reference system (EPSG code)	10.4.4(c)	_____	_____
D4	Vertical datum named; relationship to the Chapter 5 river datums of the CWC gauges on the receiving rivers, of WRD's dam, barrage, reservoir and tank levels and of the levels of the lakes and reservoirs of Nava Raipur, and, for an outfall to a mine void, of the lessee's mine survey	10.4.4(d)	_____	_____

Table 10F.5 — Form 10-1, Section E: Declaration and completeness check

Field	Entry
I confirm that this checklist is complete and correct. Certifying professional: name, signature, date	_____
For the Authority: date received	_____
Date of completeness check (within 5 working days, Clause 10.7.1)	_____
Result: accepted for review / returned	_____
Missing items, if returned	_____
Officer: name, designation, signature	_____

Form 10-2 — Compliance Calculator Summary

Supports Clauses 10.4.1(g), 10.5 and 10.9.1(b) of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Produced by the compliance calculator (Annex 10A) with Stage 1, 2 and 4 submittals. Until the calculator is issued, complete it by hand and enter "not issued" in A3. One column per drainage area where the site has more than one outfall.

Table 10F.6 — Form 10-2, Section A: Site and route

No.	Field	Entry	Clause
A1	Authority, file number, stage (1 / 2 / 4)	_____	10.5.2(a)
A2	Site area, ha; land disturbance, ha; new + replaced impervious area, m²	_____	1.5.2
A3	Calculator version and release date	_____	10.5.3
A4	Tier (A / B); flood-sensitive (yes / no); approved BSMP for the basin (yes / no)	_____	1.6, 4.5.3, 11.12
A5	Route: full method / S-1 / S-2 / fixed-coefficient / constrained redevelopment / regional or fee-in-lieu	_____	1.7, 4.5.6, 4.13, 4.14
A6	Flags raised by the calculator (list)	_____	Annex 10A.4

Table 10F.7 — Form 10-2, Section B: Rainfall and soils

No.	Field	Entry	Clause
B1	Rainfall zone; Form 2-1 reference	_____	2.13
B2	Retention depth P_R, mm; 3-month ARI depth P_3m, mm	_____	2.10, 8.4.1
B3	Climate factor applied (≤ 6 h; > 6–72 h)	_____	2.8
B4	HSG by sub-area; seasonal high groundwater, m below base	_____	3.7, 4.4.3
B5	Measured and design infiltration rates, mm/h	_____	4.4.3

Table 10F.8 — Form 10-2, Section C: Volumes

No.	Quantity	Required (m³)	Provided (m³)	Clause
C1	Retention volume (RRV)	_____	_____	4.4.2
C2	Water quality volume (WQv)	_____	_____	8.4.2
C3	Retention credit against detention	_____	—	4.4.6
C4	Detention volume	_____	_____	4.6, 4.14
C5	Fee-in-lieu or regional volume, and fee (₹)	_____	_____	4.5.6.3, 4.13.3

Table 10F.9 — Form 10-2, Section D: Release rates (each outfall)

AEP	Q_pre or cap (L/s)	Q_post (L/s)	Governing duration (h)	Meets criterion (yes / no)	Clause
50% (2-yr)	_____	_____	_____	_____	4.5
10% (10-yr)	_____	_____	_____	_____	4.5

AEP	Q_pre or cap (L/s)	Q_post (L/s)	Governing duration (h)	Meets criterion (yes / no)	Clause
1% (100-yr)	_____	_____	_____	_____	4.5

Table 10F.10 — Form 10-2, Section E: Outlets, drawdown and stage comparison

No.	Field	Entry	Clause
E1	Low-flow orifice, mm (minimum 50 mm)	_____	4.8.2
E2	Detention: time to release 90% of the 10-yr stored volume, h (≤ 48 h)	_____	4.7.1
E3	WQv release time, h (24–48 h)	_____	4.7.2
E4	Retention emptying time, h (≤ 72 h)	_____	4.7.3
E5	Model result differs from the calculator screen by more than the guide tolerance? If yes, report section that explains it	_____	10.5.2(c)
E6	Stage 4 only: changes from the Stage 2 values (list), and whether every criterion is still met	_____	10.9.2
E7	Prepared by (name, registration number), signature, date	_____	10.6.3(b)

Form 10-3 — Professional Design Certificate

Supports Clauses 10.6.1 to 10.6.5 of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Signed by the designer with every Stage 1 and Stage 2 submittal, and by an independent checker for facilities under Clause 10.6.1(c) and (d).

Table 10F.11 — Form 10-3, Section A: Project and signatory

No.	Field	Entry	Clause
A1	Authority, file number, stage	_____	—
A2	Site and facilities covered by this certificate	_____	—
A3	Designer: name, qualification, CGBVN 1984 registration number	_____	10.6.1
A4	Empanelment number, where required, and the reason: Tier A ≥ 1 ha / flood-sensitive / embankment or high-hazard basin / pumped, tide gate or regional facility	_____	10.6.1(a)–(d)
A5	Variances or waivers approved (Form 1-2 references), or "none"	_____	10.6.3(a)

Table 10F.12 — Form 10-3, Section B: Certificate of the designer

No.	I certify that:	Initials
B1	the design complies with the Chhattisgarh Stormwater Design Manual, except for the variances listed at A5, which have been approved	_____
B2	the rainfall, soils and tailwater inputs are as recorded on Form 2-1 and Form 10-2	_____
B3	the models submitted reproduce the results reported	_____
B4	I understand that a false or negligent certificate is a contravention under Clause 10.15	_____

Table 10F.13 — Form 10-3, Section C: Certificate of the independent checker (Clause 10.6.4)

No.	Field or statement	Entry
C1	Checker: name, registration and empanelment number	_____
C2	I am independent of the designer and the Applicant and have not worked on this design	_____
C3	I have checked the facilities listed: _____, and agree with the certificate in Section B	_____
C4	Matters on which I do not agree, if any	_____

Table 10F.14 — Form 10-3, Section D: Signatures

Role	Name	Signature	Date
Designer	_____	_____	_____
Independent checker (where required)	_____	_____	_____

Form 10-4 — Hold-Point Inspection Record

Supports Clauses 10.4.2(g), 10.8 and 10.9.3 of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Section A is the hold-point schedule, approved at Stage 2. Section B is completed at each hold point, with dated, geotagged photographs (Clause 10.8.2). Work shall not proceed past a hold point until Section B is signed. See Figure 10.2.

Table 10F.15 — Form 10-4, Section A: Hold-point schedule

Hold point	Facility (FAC_ID)	Stage of work	Planned date	Notice to Authority sent (date)*	Authority attended (yes / no)
(a) Excavated base, before media, liner or structure	_____	_____	_____	_____	_____
(b) Outlet structure, orifice and trash screen, before backfill or first filling	_____	_____	_____	_____	_____
(c) Infiltration and verification tests (Clause 10.9.3)	_____	_____	_____	–	_____
(d) Outfall, flap or tide gate and non-return valve, before downstream connection	_____	_____	_____	–	_____
(e) Pumps and controls, at commissioning	_____	_____	_____	–	_____
Other hold points added by the Authority	_____	_____	_____	_____	_____

* For sites under Clause 10.6.1, at least 3 working days' notice of hold points (a) and (b) (Clause 10.8.2).

Table 10F.16 — Form 10-4, Section B: Inspection record (one per hold point)

No.	Field	Entry
B1	Hold point and facility	_____
B2	Date and time of inspection; weather	_____
B3	Inspected by (certifying professional); others present	_____
B4	Approved drawing and revision checked against	_____
B5	Levels, dimensions and materials found, against approved values	_____
B6	For (a): retention and infiltration areas protected from traffic and stockpiles (Clause 10.8.3(a))	_____
B7	For (b): orifice size and invert; screen in place	_____
B8	For (c): infiltration test result and emptying time (≤ 72 h); pump duty point; gate opening and sealing	_____
B9	For (d) and (e): gate type, seating and test; pump commissioning record	_____
B10	Photograph references (dated, geotagged)	_____
B11	Result: released / not released; corrective work required	_____
B12	Corrective work checked (date, by)	_____
B13	Signature of the certifying professional, date	_____

Form 10-5 — As-built Certificate

Supports Clauses 10.9 and 10.3.5 of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Lodged before the completion certificate. The Authority shall not issue the completion certificate until this form is accepted (Clause 10.9.4).

Table 10F.17 — Form 10-5, Section A: Project and survey

No.	Field	Entry	Clause
A1	Authority, file number; Stage 2 approval reference and date	_____	—
A2	Licensed surveyor: name, licence number, date of survey	_____	10.9.1(a)
A3	Vertical datum	_____	10.4.4(d)
A4	GIS data to Annex 10B, as built (file name)	_____	10.4.4(c)

Table 10F.18 — Form 10-5, Section B: Approved against as built (one block per facility)

Item	Approved	As built	Accepted (yes / no)	Clause
Storage volume at each design level, m³	_____	_____	_____	10.9.2(a)
Outlet dimensions (orifice, weir), mm	_____	_____	_____	10.9.2(b)
Outlet and outfall inverts, m	_____	_____	_____	10.9.2(b)
Spillway or overflow level, m	_____	_____	_____	4.9
Retention volume, m³	_____	_____	_____	4.4

Table 10F.19 — Form 10-5, Section C: Re-run and tests

No.	Field	Entry	Clause
C1	Compliance calculator re-run with as-built values: version, and Form 10-2 (Stage 4) attached	_____	10.9.1(b)
C2	Model re-run with as-built values (where a model was used): file name	_____	10.9.1(b)
C3	Every criterion of Chapters 4 and 5 met on re-run (yes / no; list any not met)	_____	10.9.2(c)
C4	Infiltration retention: measured emptying time, h (≤ 72 h)	_____	10.9.3
C5	Pumps: duty point achieved (flow at head)	_____	10.9.3
C6	Tide gates and non-return valves: shown to open and seal	_____	10.9.3
C7	Deviations, and how corrected or re-certified under Clause 10.3.5	_____	10.9.2

Table 10F.20 — Form 10-5, Section D: Attachments and certificate

No.	Field	Entry
D1	Completed hold-point records (Form 10-4)	_____
D2	Final O&M plan	_____
D3	Executed Maintenance Declaration (Form 10-6) or handover documents (Form 10-8)	_____

No.	Field	Entry
D4	I certify that the facilities listed were built as approved, except as stated at C7, and perform as designed. Certifying professional: name, registration number, signature, date	_____
D5	For the Authority: accepted / returned; officer, date	_____

Form 10-6 — Maintenance Declaration

Supports Clauses 10.10.1, 10.10.2, 10.10.4 and 10.13.1 of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Model text for consideration by a competent authority. Not in force. Executed by the owner of every private stormwater facility before the completion certificate is issued, and registered so that it runs with the land.

> **Drafting note D10A.1.** The instrument under which this declaration is registered, and the Act that gives the Authority its right of entry and its power to carry out works in default, are to be confirmed by the competent authority. This form does not cite statute sections.

Table 10F.21 — Form 10-6, Section A: Parties and land

No.	Field	Entry
A1	Owner (the declarant): name and address; for a company or association, its registration number	_____
A2	Authority	_____
A3	Land: survey numbers, village, tehsil, district; extent	_____
A4	Approval reference (file number, Stage 2 approval date)	_____
A5	For multi-owner developments: the association or body that takes over after the developer (Clause 10.10.2)	_____

Table 10F.22 — Form 10-6, Section B: Schedule of facilities

Facility (FAC_ID)	Type	Location on the attached plan	Design volume (m³)	Outlet	O&M plan reference
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____

Model declaration. I/We, the owner named in Section A, declare and undertake, for myself/ourselves and for every successor in title to the land:

1. to operate, inspect and maintain each facility in Section B under the approved O&M plan, and to make the inspections of Clause 10.12 (Clause 10.10.1(b));
2. to give the Authority, its officers and its contractors a right of entry to inspect each facility and the access route shown on the plan, and to carry out works in default under Clause 10.15, with the cost recoverable from the owner (Clause 10.10.1(c));
3. not to remove, fill, cover, reduce or change the use of any facility, or obstruct its access route, without the Authority's written approval (Clauses 10.10.1(d), 10.13.5);
4. where Clause 10.10.3 applies, to lodge an Annual Maintenance Declaration and Inspection Report (Form 10-7) by 31 May each year;
5. for underground tanks, to follow the confined-space entry procedure stated here: _____ (Clause 4.15.6);
6. on sale or transfer of the land or any part of it, to disclose this declaration and the latest Form 10-7 to the purchaser, who takes on these obligations from the date of transfer (Clause 10.10.4); and
7. for a multi-owner development, that the developer remains responsible until the body named at A5 is legally constituted and has accepted this declaration in writing (Clause 10.10.2).

Table 10F.23 — Form 10-6, Section C: Execution and registration

Field	Entry
Owner: signature, name, date	_____
Witness 1: signature, name, address	_____
Witness 2: signature, name, address	_____
Acceptance by the association or body at A5 (when constituted): signature, date	_____
Registration: office, document number, date	_____
For the Authority: received and recorded in the facility register; officer, date	_____

Form 10-7 — Annual Maintenance Declaration and Inspection Report

Supports Clauses 10.10.3, 10.11.1(d) and 10.12 of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Due by **31 May** each year from every owner of a facility in a Tier A area, and from every owner of a pumped facility, tide gate or regional facility anywhere (Clause 10.10.3). See Figure 10.3.

Table 10F.24 — Form 10-7, Section A: Owner and facilities

No.	Field	Entry
A1	Reporting year (1 June to 31 May)	_____
A2	Owner or association; Maintenance Declaration registration reference	_____
A3	Facilities covered (FAC_ID and type)	_____
A4	Any facility under Clause 10.6.1(c) or (d) (yes / no). If yes, Section E is required	_____

Table 10F.25 — Form 10-7, Section B: Inspections made during the year (Table 10.3)

Facility	Routine inspections (dates)	Pre-monsoon inspection (date)	After heavy-rainfall events (dates)	Monthly pump test runs (number)
_____	_____	_____	_____	_____
_____	_____	_____	_____	_____

Table 10F.26 — Form 10-7, Section C: Pre-monsoon checklist (one per facility)

No.	Item	Condition (OK / defect)	Defect and date corrected
C1	Inlets, pretreatment and silt traps clear	_____	_____
C2	Orifice, trash screen and outlet clear; orifice size unchanged	_____	_____
C3	Sediment below the trigger in the O&M plan	_____	_____
C4	Storage volume not reduced; no filling, covering or change of use	_____	_____
C5	Overflow and spillway route clear	_____	_____
C6	Retention drains within 72 h (observation well or surface)	_____	_____
C7	Pumps: full duty test; standby pump and standby power tested; fuel for 72 h on site	_____	_____
C8	Alarms and telemetry tested	_____	_____
C9	Flap or tide gates and non-return valves: operated and seal tested	_____	_____
C10	Safety: covers locked, fences, signs and depth markers in place	_____	_____
C11	Access routes clear and not used for parking	_____	_____

Table 10F.27 — Form 10-7, Section D: Declaration of the owner

Field	Entry
I/We declare that the pre-monsoon inspection of Clause 10.12 has been made, that the defects found have been corrected as listed in Section C, and that the facilities are maintained under the approved O&M plan. Owner: name, signature, date	_____

Table 10F.28 — Form 10-7, Section E: Empanelled professional (facilities under Clause 10.6.1(c) and (d))

Field	Entry
I have inspected the facilities listed and confirm Section C. Name, empanelment number, signature, date	_____

Table 10F.29 — Form 10-7, Section F: For the Authority

Field	Entry
Date received; on time (by 31 May) yes / no	_____
Entered in the facility register (Clause 10.17), date	_____
Follow-up action, if any (Clause 10.15)	_____

Form 10-8 — Asset Handover Certificate

Supports Clause 10.14 of Chapter 10.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Signed by the developer and the ULB when drains, open space facilities, regional facilities or other stormwater assets built by a developer are handed over.

Table 10F.30 — Form 10-8, Section A: Asset and parties

No.	Field	Entry
A1	Developer: name and address	_____
A2	ULB	_____
A3	Layout or project, approval reference	_____
A4	Assets handed over (FAC_ID, type, length or volume), with a plan	_____

Table 10F.31 — Form 10-8, Section B: Conditions of handover (Clause 10.14.1)

No.	Condition	Met (yes / no)	Reference or date
B1	As-built certificate (Form 10-5) accepted	_____	_____
B2	Facility cleaned; pipes and culverts inspected internally (CCTV or manual entry), record handed over	_____	_____
B3	Land, or an easement over it, transferred to the ULB	_____	_____
B4	O&M plan, drawings, GIS data (Annex 10B) and warranties handed over	_____	_____

Table 10F.32 — Form 10-8, Section C: Defects liability and register

No.	Field	Entry	Clause
C1	Defects liability period: from _____ to _____ (includes at least one full monsoon season, from 10 June to 20 October (31 October in the Bastar division), after handover)	_____	10.14.2
C2	Developer responsible during the period for defects and routine maintenance	_____	10.14.2
C3	Security deposit held by the ULB, ₹, and release date	_____	10.14.2
C4	Entered in the ULB GIS drainage asset register and the facility register: date	_____	10.14.3

Table 10F.33 — Form 10-8, Section D: Signatures

Party	Name and designation	Signature	Date
Developer	_____	_____	_____
Certifying professional	_____	_____	_____
ULB	_____	_____	_____

ANNEXES TO CHAPTER 11

Basin and City Stormwater Master Plans

These annexes support Chapter 11. They give a terms of reference template for a Basin Stormwater Master Plan (BSMP), the layout of the basin list and boundary layer, the layout and method for depth–damage curves, the standard for publishing BSMP data and models, and the annual implementation report form. Annexes 11B and 11C need data that are not yet available; they say so and give the rule to use in the meantime. The template and the form are a model and a draft form for consideration by a competent authority, and are not in force.

Annex 11A — BSMP terms of reference template

Supports Clauses 11.1 to 11.12 of Chapter 11.

> *Model text for consideration by a competent authority. Not in force.* The Lead Authority adapts this template for each basin. Items in "[ ]" are for the Lead Authority to complete. Clause references are to this Manual, which the consultant shall follow in full.

11A.1 Background and purpose

1. The [Lead Authority] intends to prepare a Basin Stormwater Master Plan for the [name] basin under Chapter 11 of the Chhattisgarh Stormwater Design Manual.
2. The BSMP shall (Clause 11.1.2):
    - (a) map flood hazard for the basin under existing and future conditions;
    - (b) set the level of service for each part of the drainage system;
    - (c) identify, cost and rank the works, storage and land needed to reach that level of service;
    - (d) identify land to be reserved in the statutory land-use plan for regional storage, talab and tank restoration and major flow paths;
    - (e) set the basin inputs for site design: regional facilities, the fee-in-lieu unit rate, and any basin release rate stricter than Chapter 4; and
    - (f) recommend catchments for notification as flood-sensitive.
3. The BSMP may make the site criteria of Chapters 4 to 8 stricter for the basin. It shall not relax them, except through the regional detention and fee-in-lieu routes of Clauses 4.13 and 4.5.6 (Clause 11.1.3).

11A.2 Client, partners and study area

1. **Client:** [Lead Authority] (Clause 11.4.1).
2. **Partners:** WRD, wherever the basin includes a river, canal or tank under its control, and [other local bodies, RDA or T&CP, CGSDMA] (Clause 11.4.1).
3. **Study area:** the whole of the [name] basin, draining to [outfall], as delineated under Clause 11.3, including the parts outside the client's limits (Clause 11.3.1). The provisional boundary is in [Annex 11B / attached map]. The consultant shall confirm it in Task 2.

4. **Links to other basins:** [for example the receiving river, reservoir or talab corridor]. Diversions between basins by canal, drain or surplus course shall be mapped and represented in the model (Clause 11.3.2).

11A.3 Team

1. The study shall be led by an engineer empanelled as a Stormwater Design Professional with the basin modelling endorsement of Annex 10E (Clause 11.4.3).
2. The team shall include at least: [a hydrologist; a hydraulic modeller; a coastal or tidal specialist where the basin has a tidal outfall; a GIS and survey lead; a planner; a cost engineer; a community engagement lead]. The client sets the minimum qualifications and experience of each.

11A.4 Tasks

Task 1 — Inception. Review existing studies, data and projects in the basin. Prepare a data inventory against Table 11.1, a gap list, a survey plan, a consultation plan, and the work programme. Deliver the inception report.

Task 2 — Data and surveys. Assemble, survey where missing, and quality-check every item of Table 11.1 (Clause 11.5.1):

- (a) terrain: DEM from LiDAR or photogrammetry with vertical accuracy of 0.15 m RMSE or better in urban areas, tied to the Survey of India datum, with the accuracy test reported;
- (b) drain network: every public drain, culvert and outfall, with invert levels, sizes, condition and silt depth; private drains above 600 mm;
- (c) waterbodies: every tank, talab, pond and marsh under Chapter 7: FTL, MWL, bund level, stage–storage, sluices, surplus weirs, supply channels and encroachment, in the fields of Annex 7A;
- (d) rivers and canals: cross-sections at no more than 200 m spacing and at every structure;
- (e) outfalls, rivers, reservoirs, barrages and mine voids: outfall invert levels, on a named datum, against the warning, danger and high flood levels of the receiving river at the nearest CWC gauge (the Indravati at Jagdalpur, the Kharun at Pathardihi, the Hasdeo, the Mahanadi or another river) and its recorded peaks, with any local gauge kept as a separate series; for outfalls to reservoirs, barrage ponds and talabs, the full reservoir level, maximum water level and spillway and sluice data from WRD or the custodian, with any conflict between published values; the release records of the dams and barrages that set the receiving levels, and the extent of the Hirakud backwater; for any mine void proposed as storage, the lessee's survey of its level, spill and stability, with the consent of DGMS; the breach flow paths of ash dykes; the waterway lines of the Chhattisgarh Bhumi Vikas Niyam 1984 and the wetland lines where they bind; and sluices, flap gates, interception weirs, anicuts, check dams, lake outlets and pumping stations on the outfalls, with their capacities and operating rules;
- (f) rainfall and water levels: all available records (IMD, including its observatories at Raipur (since 1901) and Raipur (Mana) airport, its stations at Jagdalpur, Pendra Road, Ambikapur, Raigarh, Durg, Champa, Jashpurnagar, Kanker and Rajnandgaon, and the automatic weather stations and rain gauges reported by the Meteorological Centre Raipur; the district and tehsil rain gauges of the State, whose records IMD tabulates in the *Climate of Chhattisgarh*; the rain and river gauges of CWC and of WRD, including those at its dams and barrages; the agrometeorological observatory of Indira Gandhi Krishi Vishwavidyalaya at Raipur; and any gauges kept by the Municipal Corporations, NRDA and the Smart City companies), including the 4 August 1910 (Raipur's record day of 370.3 mm, rainfall only), August 2020 (the Kharun at Raipur and the upper Mahanadi), August 2025 (the Bastar division) and September 2026 (statewide, with the Indravati at Jagdalpur and the Kharun at Raipur) events;
- (g) flood evidence: surveyed flood marks and mapped extents of past events, including public reports, photographs and residents' records;
- (h) land use: existing land use and imperviousness; future land use from the statutory master plan;
- (i) soils and groundwater: HSG map (Annex 3A or Clause 3.7.2), seasonal high groundwater, aquifer and recharge zones; and
- (j) sewer interaction: known cross-connections, dry-weather flow in drains, solid-waste hotspots.

Confirm the basin boundary and the inter-basin diversions (Clause 11.3.2). Deliver the data report and the data in the formats of Annex 11D.

Task 3 — Model build. Build a coupled 1D–2D hydraulic model of the minor system, the major (overland) system, rivers, canals and tanks, and the outfalls (Clause 11.7.1; Figure 11.2). Use the methods of Chapter 3 and the tidal and river boundaries of Chapter 5. Run the design storms of Chapter 2, including the 24-, 48- and 72-hour storms, for at least the 50%, 10%, 1% and 0.2% AEPs, with areal reduction under Clause 2.7.3 (Clause 11.7.2). Where the basin has tank cascades, run continuous simulation under Clause 3.13 with the full IMD gridded daily record where available, and at least 30 years, disaggregated by the temporal patterns of Annex 2D. The two-step approach of Clause 3.13.5 may be used: starting tank levels and antecedent conditions from the continuous run are passed to the design-storm runs (Clause 11.7.3).

Task 4 — Calibration and validation. Calibrate and validate the model under Clause 3.15, using at least two of the 4 August 1910 (Raipur's record day of 370.3 mm, rainfall only), August 2020 (the Kharun at Raipur and the upper Mahanadi), August 2025 (the Bastar division) and September 2026 (statewide, with the Indravati at Jagdalpur and the Kharun at Raipur) events where data exist (Clause 11.7.5). Report the performance measures of Clause 3.15.4 and the reasonableness check of Clause 3.15.6.

Task 5 — Scenarios and hazard maps. Model the scenarios of Clause 11.7.4: (a) existing; (b) future, uncontrolled; (c) future, controlled; (d) future, with plan; and (e) climate, applied to (a) and (d). Produce flood hazard maps of extent, depth, velocity and hazard class (H1–H6, Chapter 6) for the 10%, 1% and 0.2% AEPs, existing and future-with-plan (Clause 11.8.4(a)).

Task 6 — Level-of-service assessment. Compare scenarios (a), (c) and (d) with the levels of service of Table 11.2 and any stricter levels the client adopts (Clause 11.6.1). Map every shortfall.

Task 7 — Options. Develop options in the order of Clause 11.8.1: protect and restore storage; regional retention and detention, sited by modelling; conveyance; outfall works; property-level protection; non-structural measures. Assess each option for the criteria of Clause 11.8.2: reduction in flood risk (buildings and people flooded, and expected annual damage using Annex 11C); capital and 30-year maintenance cost; land required; effect on groundwater recharge in the critical and semi-critical blocks of the Seonath plain, Dharsiwa (Raipur), Durg and Dhamdha among them, where the Chandi and Charmuria limestone that supplies Raipur is karstified and cavernous, the shallow post-monsoon water table, within 5 m in about three wells in four, kanhar clays that take no recharge once wet, mine dewatering in the Korba coalfield, and the quality of the groundwater, with fluoride and nitrate in parts of the State and arsenic at Ambagarh Chowki, and of the rivers, reservoirs and talabs that the runoff reaches; effect on downstream basins and the receiving river, reservoir or talab; and who benefits.

Task 8 — Basin inputs for site design. Where scenario (c) does not meet the level of service, propose basin release rates (m³/s/ha) by AEP and sub-catchment (Clause 11.8.3). Recommend catchments for notification as flood-sensitive, with the evidence of Clause 4.5.4.1. Identify regional facility sites, with storage volumes, land needs and the sites they serve, and compute the fee-in-lieu unit rate, including land, construction and 10 years of maintenance (Clauses 4.13.3, 11.8.4(c), (d)).

Task 9 — Capital Improvement Programme. Rank the works by risk reduction per rupee, with cost, phasing and responsible agency (Clause 11.8.4(e)).

Task 10 — Maintenance and monitoring. Prepare the pre-monsoon maintenance programme: desilting, gate and pump testing, and tank operating rules (Clause 11.8.4(f); Chapters 7 and 10). Specify monitoring needs: new rain gauges, level sensors and flood-mark survey points (Clause 11.8.4(g); Clause 10.16.1).

Task 11 — Land-use integration. Prepare the material for the statutory land-use plan required by Clause 11.9.1: sites to be reserved; the 1% AEP hazard map with the Chapter 7 setbacks; land in hazard classes H5–H6 in the 1% AEP event; and open space locations that can form part of flow paths or regional storage.

Task 12 — Consultation. Publish the draft BSMP for public comment for at least 60 days, with public meetings in each affected local body (Clause 11.10.1). Keep a log of comments and responses.

Task 13 — Final BSMP and publication. Revise the BSMP after consultation. Prepare the package for approval by the State Stormwater Technical Committee, including the model for review (Clause 11.11.1). Publish the data, models, maps and CIP under Annex 11D (Clause 11.10.2).

11A.5 Deliverables

Table 11A.1 — Deliverables

No.	Deliverable	Task	Due (months from start)
D1	Inception report, data inventory and survey plan	1	[ ]
D2	Data report and data sets (Annex 11D)	2	[ ]
D3	Model build report and model files	3	[ ]
D4	Calibration and validation report	4	[ ]
D5	Scenario results, hazard maps and level-of-service assessment	5, 6	[ ]
D6	Options report, with basin inputs for site design	7, 8	[ ]
D7	Draft BSMP with draft CIP, for consultation	9–12	[ ]
D8	Final BSMP, consultation log, approval package and open-data release	13	[ ]

11A.6 Ownership and publication

All data, models, maps and reports produced under these terms are owned by the Lead Authority and shall be published under Annex 11D. The consultant shall not use any software licence, data licence or format that prevents publication, or that prevents a third party from re-running the model with the stated software version.

Annex 11B — Basin lists and boundaries for Tier A bodies

Supports Clauses 11.3 and 11.4 of Chapter 11.

To be prepared. This annex will contain the list of basins for every Tier A body, with a GIS layer of basin boundaries delineated under Clause 11.3.2. It needs the list of Tier A bodies (Annex 1A), a DEM of the accuracy of Clause 11.5.1, re-flown over the coalfields where mining changes the ground, the drain network of every drain-owning body (the Municipal Corporations, NRDA, the Bhilai Steel Plant township, RDA and the Chhattisgarh Housing Board, with the sewage interceptions on the nalas), the rivers, dams, barrages and reservoirs of WRD with their full reservoir levels, spillways, gates and operating rules on a stated datum, the release records of Gangrel, Minimata Bango, the Hasdeo barrage, Kharang and the Mongra, Arpa Bhaisajhar and Kelo works, the extent of the Hirakud backwater inside the State, the mine voids, pit lakes, overburden dumps and garland drains on the coal leases with their surveyed levels, the ash ponds and the breach flow paths of their dykes, the talabs and the lakes of Nava Raipur with their levels and the 50 m lines of the State Wetland Authority, and the tanks recorded in the land records (other likely sources: Survey of India, CWC, CGWB, DGMS, the Chhattisgarh Council of Science and Technology and the district land records). No basin map of any Chhattisgarh city was found. Until the annex is published, the basins of Raipur, Nava Raipur, Durg and Bhilai, Bilaspur, Korba, Raigarh and Jagdalpur named in Clause 11.3.3 are at least those named there; other Tier A bodies define their basins in their first BSMP (Clause 11.3.3); and the interim arrangements of Clause 11.12 apply in every basin without an approved BSMP.

Table 11B.1 — Basins named in Clause 11.3.3 (provisional)

Tier A body or area	Basin	Note from Chapter 11
Raipur and Birgaon (RMC, Birgaon Municipal Corporation)	Catchments draining to the Kharun through the city's nalas	Talabs as storage; diversion weirs of the Chandanidih, Kara and Nimora plants and the Mahadev Ghat anicut in the 1D network; Kharun at Pathardihi as boundary
Nava Raipur (NRDA)	The four catchments of the four main streams of the Naya Raipur Development Plan-2031	Lakes and reservoirs (Sendh, Jhanjh, Khandawa and others) as storage and outfall; 50 m wetland line shown
Durg, Bhilai, Risali and Bhilai-Charoda (Municipal Corporations, Bhilai Steel Plant)	Catchments draining to the Seonath, including the Shankar and Pulgaon nalas	Township drains and sewage interceptions mapped; Seonath below the Mongra barrage as boundary
Bilaspur (Municipal Corporation)	Catchments draining to the Arpa	Arpa Bhaisajhar barrage level and releases as boundary; Kopra Jalashay shown
Korba (Municipal Corporation, coal and power companies)	Catchments draining to the Hasdeo below the Darri barrage	Bango and Darri releases as inflows; Bangodam and Katghora levels as boundary; outfall-closed case a design case; mine voids only with lessee and DGMS consent; ash-dyke breach paths shown
Raigarh (Municipal Corporation)	Catchments draining to the Kelo below the Kelo dam	Mahanadi, Mand and Hirakud backwater levels as boundary for low outfalls
Jagdalpur (Municipal Corporation)	Catchments draining to the Indravati	CWC Jagdalpur levels as boundary; outfall-closed case a design case
Other Tier A bodies, including Rajnandgaon, Dhamtari, Ambikapur and Chirmiri	To be defined in the first BSMP	Clause 11.3.3

Table 11B.2 — Attributes of the basin boundary layer (layout)

Field	Type	Content
BASIN_ID	Text	Basin identifier
NAME_EN, NAME_HI	Text	Basin name in English and Hindi

Field	Type	Content
TIER_A	Text	Tier A body or bodies in the basin
OUTFALL	Text	Outfall to the sea, river or major waterbody (Clause 11.2)
AREA_KM2	Real	Basin area, km²
LEAD_AUTH	Text	Lead Authority (Clause 11.4.1)
PARTNERS	Text	Partners, including WRD where it applies
LINKS	Text	Diversions to and from other basins (Clause 11.3.2)
BSMP_STAT	Text	None / in preparation / draft published / approved
BSMP_DATE	Date	Date of approval of the current BSMP
REVIEW_DUE	Date	Date of the next review (Clause 11.11.2)

Annex 11C — Depth–damage curves for Chhattisgarh building types

Supports Clause 11.8.2 of Chapter 11.

To be prepared. This annex will contain depth–damage curves for the main building types of Chhattisgarh cities, for structure and contents, for use in computing expected annual damage under Clause 11.8.2. It needs a building-type classification and building stock data (Census housing tables, property-tax records, Lead Authority GIS), damage surveys and claims from the 4 August 1910 (Raipur's record day of 370.3 mm, rainfall only), August 2020 (the Kharun at Raipur and the upper Mahanadi), August 2025 (the Bastar division) and September 2026 (statewide, with the Indravati at Jagdalpur and the Kharun at Raipur) floods (Revenue and Disaster Management Department and CGSDMA damage assessments, insurers), and replacement costs (Public Works Department schedules of rates). Until it is published, each BSMP shall state the depth–damage relations it uses and their source, and shall report the reduction in buildings and people flooded for every option (Clause 11.8.2), which does not depend on the curves.

Table 11C.1 — Depth–damage curve (layout, one table per building type)

Depth of water above floor level (m)	0	0.25	0.5	1.0	1.5	2.0	3.0
Structure damage (% of replacement cost)	_____	_____	_____	_____	_____	_____	_____
Contents damage (% of contents value)	_____	_____	_____	_____	_____	_____	_____
Building type, source and year of costs	_____						

11C.1 Use in option assessment

Expected annual damage (EAD) is computed for each scenario from the damages at the modelled AEPs. With damages D_1, D_2, … at annual exceedance probabilities P_1 > P_2 > …, using at least the 50%, 10%, 1% and 0.2% AEP results of Clause 11.7.2:

$$EAD \approx \Sigma \tfrac{1}{2} \times (D_i + D_{(i+1)}) \times (P_i - P_{(i+1)})$$

The reduction in EAD between scenario (c) and scenario (d), or between options, is the risk-reduction benefit used to rank works per rupee (Clause 11.8.4(e)). The BSMP shall state how damages beyond the 0.2% AEP and below the 50% AEP were treated.

Annex 11D — BSMP data and model publication standard

Supports Clauses 11.5.1 and 11.10 of Chapter 11, and Clause 10.17 of Chapter 10.

11D.1 What is published, and when

Table 11D.1 — Items to be published

Item	Format	At draft (Clause 11.10.1)	At approval (Clause 11.10.2)
BSMP report, with the level-of-service assessment and options	Searchable PDF	Yes	Yes, final
Flood hazard maps: extent, depth, velocity, hazard class, for each AEP and scenario of Clause 11.8.4(a)	GeoTIFF rasters; extents also as GeoPackage polygons	Yes	Yes
Capital Improvement Programme	CSV and PDF	Draft	Yes
Basin release rates, recommended flood-sensitive catchments, regional facility sites, fee-in-lieu unit rate	GeoPackage and CSV	Yes	Yes
Data of Table 11.1 (terrain, drains, waterbodies, cross-sections, river, reservoir, barrage and mine-void levels and outfall structures, rainfall and levels, flood evidence, land use, soils and groundwater, sewer interaction)	GeoTIFF (terrain); GeoPackage (features); CSV (tables and time series)	Summary	Yes, except security-sensitive asset details
Model files: native, runnable, with all boundary-condition, rainfall and control files, the software name and version, and a run log	Native format, zipped	No	Yes
Calibration and validation report	Searchable PDF	Yes	Yes
Consultation log of comments and responses	PDF or CSV	—	Yes

11D.2 Formats and references

- (a) Raster data shall be GeoTIFF, preferably cloud-optimised, with the cell size stated. Vector data shall be GeoPackage. Tables and time series shall be CSV in UTF-8, with a header row and units in the column names.
- (b) Horizontal and vertical references shall be as in Annex 10B.1(b) and (c). The vertical datum and its relationship to the Chapter 5 river datums of the CWC gauges on the receiving rivers, of WRD's dam, barrage, reservoir and tank levels and of the levels of the lakes and reservoirs of Nava Raipur, and, for an outfall to a mine void, of the lessee's mine survey shall be stated in every data set that carries levels.
- (c) Times shall be in Indian Standard Time, written as ISO 8601 (yyyy-mm-ddThh:mm+05:30). Units shall be SI.
- (d) Hazard map files shall be named `<basin>_<scenario>_<AEP>_<variable>.tif`, where scenario is a to e of Clause 11.7.4, AEP is written as 50, 10, 1 or 0p2, and variable is depth, velocity, hazard or extent; for example `kharun_d_1_depth.tif`.
- (e) Waterbody data shall use the fields of Annex 7A. Facility and outfall data shall use the fields of Annex 10B.

11D.3 Metadata

Every published data set shall carry: title; description; Lead Authority; source and method; date of capture; accuracy (for the DEM, the measured vertical RMSE); horizontal and vertical reference; scenario and AEP, where relevant; BSMP version; licence; and a contact.

11D.4 Withholding and personal data

- (a) Security-sensitive details of critical assets may be withheld, with the reason recorded and published (Clauses 10.17.3, 11.10.2). The flood hazard maps and the model shall still be published; only the sensitive attributes are removed.
- (b) Personal data shall not be published (Clause 10.17.3). Flood evidence collected from residents shall be published without names or contact details unless the person consents.

11D.5 Licence, viewer and versions

- (a) Published data and models shall be released under the Government Open Data Licence – India or an equivalent open licence (Clause 10.17.4).
- (b) The hazard maps, regional facility sites and CIP shall also be shown on a public map viewer (Clause 11.10.2).
- (c) Each approved BSMP shall carry a version number and date. Superseded versions shall stay available, marked as superseded, so that designs approved under them can be checked.
- (d) The published model package shall allow a third party with the stated software version to reproduce the published results. Designers may use its boundary conditions for site designs in the basin (Clause 11.10, Commentary).

Form 11-1 — Annual implementation report

Supports Clause 11.11.3 of Chapter 11, with Clauses 4.5.6.3 and 4.13.4 of Chapter 4.

Draft form — Chhattisgarh Stormwater Design Manual, Draft v1.0. Published by the Lead Authority for each basin with an approved BSMP before each south-west monsoon, by **31 May** (Clause 11.11.3).

Table 11F.1 — Form 11-1, Section A: Basin and plan

No.	Field	Entry
A1	Basin; Lead Authority; partners	_____
A2	BSMP version and date of approval	_____
A3	Reporting year (1 June to 31 May)	_____
A4	Date of publication (by 31 May) and web address	_____

Table 11F.2 — Form 11-1, Section B: CIP progress

CIP rank	Work	Agency	Cost in CIP (₹)	Spent this year (₹)	Spent to date (₹)	Status	Expected completion
_____	_____	_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____	_____	_____

Table 11F.3 — Form 11-1, Section C: Fee-in-lieu ledger (Clauses 4.5.6.3(c) and 4.13.4)

Regional facility or tank	Volume credited to sites (m³)	Volume delivered (m³)	Fees received (₹)	Fees spent (₹)	Fees held, with year received
_____	_____	_____	_____	_____	_____
Basin total: volume owed and delivered	_____	_____	_____	_____	_____

Table 11F.4 — Form 11-1, Section D: Maintenance completed (Clause 11.8.4(f))

No.	Item	Planned	Done	Date completed
D1	Desilting of drains and canals (length, km; volume, m³)	_____	_____	_____
D2	Tank and talab desilting and restoration (tanks, number; storage restored, m³)	_____	_____	_____
D3	Gate tests (number of gates)	_____	_____	_____
D4	Pump station duty tests (number of stations)	_____	_____	_____
D5	Tank operating rules reviewed with the custodian (number)	_____	_____	_____
D6	Monitoring added: rain gauges, level sensors, flood-mark points (Clause 11.8.4(g))	_____	_____	_____

Table 11F.5 — Form 11-1, Section E: Flooding against the levels of service

Event (dates)	Rainfall compared with the design depths (Clause 10.16.2(a))	Level-of-service element (Table 11.2)	Location	Met (yes / no)	Event report reference (Clause 10.16.2)
_____	_____	_____	_____	_____	_____

Table 11F.6 — Form 11-1, Section F: Review triggers and sign-off

No.	Field	Entry
F1	Event exceeding the 1% AEP design depth in the basin this year (yes / no). If yes, review due within 2 years (Clause 11.11.2)	_____
F2	Statutory land-use plan revised this year (yes / no) (Clause 11.11.2)	_____
F3	Next 10-year review due (date)	_____
F4	Officer of the Lead Authority: name, designation, signature, date	_____